2024 Summary of Revisions

City of Bismarck
Construction Specifications for Municipal Public Works Improvements

The City of Bismarck has revised the Construction Specifications for Municipal Public Works Improvements for 2024.

The following is a list of specification sections and standard plates that have significant revisions. Specification numbers listed below are the numbers in the 2024 revision, which may not correspond to the previous version. Other sections have been revised for clarity, formatting, or other minor revisions. If further explanation of any revisions is required, please contact the City of Bismarck Engineering Department. Specifications with revisions highlighted are available on the City website at:
www.bismarcknd.gov/Engineering

DIVISION 100 - GENERAL PROVISIONS

Section 102 - Bidding and Contract Documents
- 5 - Removed Builder's Risk Insurance requirement

Section 105 - Legal Relationships and Responsibilities
- 3 - Added lawn maintenance requirement

Section 107 - Estimates and Payments
- 2.1 - Added requirements for payment for Materials Stored

DIVISION 200 - EARTHWORK

Section 205 - Erosion and Sediment Control
- 1 - Added additional requirements
- 3.11 - New section for Transition Mat

DIVISION 500 - RIGID PAVEMENT

Section 501 - Portland Cement Concrete Pavement
- 2.12 - Added approved Curing Compound

DIVISION 600 - CONCRETE SIDEWALKS, DRIVEWAYS, AND CURB AND GUTTER

Section 601 - Concrete Sidewalks
- 2.7 - Added Macro-Fiber Reinforcement materials requirements
- 3.1 - Added Macro-Fiber Reinforcement construction requirements

Section 602 - Concrete Driveways

- 3.11 - Added Macro-Fiber Reinforcement construction requirements

DIVISION 800 - SEWERS

Section 801 - Sanitary Sewer

- 3.5 - Revised Backfill requirements
- 3.6 - Revised Backfill Classifications

Section 801 - Storm Sewer

- Backfill requirements revised as per Section 801-3.5 and 3.6

DIVISION 900 - Water Distribution

Section 901 - Watermain

- Backfill requirements revised as per Section 801-3.5 and 3.6
- 3.11B - Revised Hydrant Extension construction requirements

Section 910 - Water Main Pipe Bursting

- 2.2 - Added data logger requirements
- 3.4 - Added data logger requirements

DIVISION 1000 - ELECTRICAL

Section 1003 - Streetlighting Feedpoint

- 2.4 - Revised material requirements
- 3.1 - Revised requirements for pad mounted feedpoints

Sections 1004 - Streetlight Units

- 2.1 - Revised material requirements for luminaires
- 2.2 through 2.7 - Revised streetlight materials requirements
- 3.1 – Added mast arm orientation requirements

DIVISION 1200 - MISCELLANEOUS CONSTRUCTION

Section 1205 - Manholes and Inlets

- 2.5 - Moved to 1206 for clarity
- 2.6 - Moved to 1206 for clarity
- 2.9 - Revised Air Release Valve requirements
- 3.5 - Moved to 1206 for clarity

Section 1206 - Casting and Adjustment

- 2.2 - Moved from 1205 for clarity
  - Added requirement for utility labels on manhole lids
- 2.3 - Moved from 1205 for clarity

- 3.4 - Moved from 1205 for clarity

Section 1209 - Sanitary Sewer and Water Service Connections
- 2.4 - Added additional allowable PE pipe material

Section 1211 - Traffic Control
- 4.1 - Revised to include payment schedule for projects with units

Section 1212 - Highway Posts and Signs
- Completely revised Section

STANDARD DETAILS - The following Standard Details have been revised (drawings with revisions highlighted are available on the City of Bismarck website):
- 600 - 2
- 1003 – 1,2,4
- 1205 - 1-4,6,11
- 1209 - 1

DIVISION 100

SECTION 101 – ABBREVIATIONS AND DEFINITIONS

101-1 REFERENCES

The specifications rely on many cross-references, both to internal sources in the specifications and external sources in other contract documents, City of Bismarck manuals, and other industry resources. If the contract documents reference an external publication, the City of Bismarck intends that the reference be to the most recent issue, including interim publications before the date of the advertisement, unless the contract specifies otherwise.

Each contract item listed in the contract references a section number from the specifications; therefore, all of the provisions of that referenced section that are relevant to the proper completion of the contract item are binding upon the CONTRACTOR. This includes the requirements found in the "General" subsections as well as those specific requirements listed thereafter.

Within the specifications, references to other sections of the specifications apply the same as if they were a part of the section from which they were referenced. A cross-reference to a specific section of these specifications includes all general requirements of the referenced section.

101-2 ABBREVIATIONS

Wherever the following abbreviations are used in the contract documents, their meaning shall be as follows:

AASHTO	American Association of State Highway and Transportation Officials
AC	Asphaltic Cement
ACI	American Concrete Institute
ACPA	American Concrete Pipe Association
ADA	Americans with Disabilities Act
AGC	Associated General Contractors of America
AIA	American Institute of Architects
AISI	American Iron and Steel Institute
ANLA	American Nursery and Landscaping Association
ANSI	American National Standards Institute
ARTBA	American Road and Transportation Builders Association
ASCE	American Society of Civil Engineers
ASLA	American Society of Landscape Architects
ASTM	ASTM International
ATSSA	American Traffic Safety Services Association
AWPA	American Wood Protection Association
AWWA	American Water Works Association

AWS	American Welding Society
BMP	Best Management Practices
CADD	Computer-Aided Drafting Design
CRSI	Concrete Reinforcing Steel Institute
DBE	Disadvantaged Business Enterprise
EEO	Equal Employment Opportunity
ESAL	Equivalent Single Axle Load
EPA	U.S. Environmental Protection Agency
FAA	Federal Aviation Administration
FHWA	Federal Highway Administration, U.S. Department of Transportation
HDPE	High-Density Polyethylene
IES	Illuminating Engineering Society
ISSA	International Slurry Surfacing Association
ITE	Institute of Transportation Engineers
ITS	Intelligent Transportation System
IMSA	International Municipal Signal Association
LED	Light Emitting Diodes
MUTCD	Manual on Uniform Traffic Control Devices
NCHRP	National Cooperative Highway Research Project
NDCC	North Dakota Century Code
NDDEQ	North Dakota Department of Environmental Quality
NDDOT	North Dakota Department of Transportation
NDPDES	North Dakota Pollutant Discharge Elimination System
NEC	National Electrical Code
NEMA	National Electric Manufacturers Association
NEPA	National Environmental Policy Act
NHTSA	National Highway Traffic Safety Administration
NPCA	National Precast Concrete Association
NRMCA	National Ready-Mix Concrete Association
NTCIP	National Transportation Communications for ITS Protocol
NTPEP	National Transportation Product Evaluation Program
OSHA	Occupational Safety and Health Administration, U.S. Department of Labor
PCA	Portland Cement Association
PCC	Portland Cement Concrete
PCI	Precast/Prestressed Concrete Institute
PVC	Polyvinylchloride - PVC Pipe
QA	Quality Assurance
QC	Quality Control
SAE	SAE International
SG	Specific Gravity
SSPC	The Society for Protective Coatings
SWPPP	Storm Water Pollution Prevention Plan
UL	Underwriters Laboratory, Inc.
USACE	United States Army Corps of Engineers
USFWS	U.S. Fish and Wildlife Service
VMA	Voids in Mineral Aggregate

101-3 DEFINITIONS

Wherever the following terms are used in the contract documents, their meaning shall be as follows:

Act of God. An unforeseeable act, event, or happening resulting from natural causes such as earthquake, tornado, or other cataclysmic phenomena.

Actual Quantity. The quantity of a contract item measured by the ENGINEER.

Addendum. A document issued by the CITY after the advertisement and before the bid opening that modifies or supplements the proposal package and will become part of the contract.

Advertisement for Bids. A public announcement inviting proposals. The advertisement will identify how to get a copy of the proposal package, how to submit a proposal, and the date, time, and place of the bid opening.

Award. The CITY's acceptance of a proposal.

Base Course. The layer or layers of specified or selected material placed on a subbase or subgrade to support a surface course.

Bid Bond. The security furnished to guarantee the Bidder will enter into the contract if the Proposal is accepted.

Bidder. An individual or legal entity submitting a proposal.

Bid Opening. The public opening of proposals at the date, time, and location identified in the Advertisement.

Bid Item List. A list of the bid items and estimated quantities in the proposal forms. The bid item list becomes the list of contract items after execution of the contract.

Bid Schedule. Bid document which includes all bid items, unit of measurement and estimated quantities.

Bid Unit Price. The price per unit for a contract item submitted by the Bidder on the proposal forms. After award the bid unit price becomes the contract unit price.

Business Day. Any calendar day, except Saturdays and holidays.

Calendar Day. Every day shown on the calendar. A day begins and ends at midnight.

Certificate of Compliance. A certificate provided by the CONTRACTOR to the Engineer.

Change Order. A written order from the CITY to the CONTRACTOR, and signed by both parties, detailing contract revisions for work within the scope of the original contract.

City. The City of Bismarck or its representatives.

City Engineer. The Director of the City of Bismarck Engineering Department acting directly or through authorized representatives.
Engineer. An authorized representative of the City of Bismarck Engineering Department, who is responsible for engineering supervision of construction.

Construction Limits. The area from the beginning station to the ending station of the project and between the slope stakes or as shown on the Plans.

Contract. The written agreement between the CITY and the CONTRACTOR setting forth the obligations of the parties for the performance of the prescribed work.

The contract includes the following:

1. Proposal for bids, instructions to bidders, and requirements for bidders
2. General provisions
3. Special provisions
4. Specifications
5. Contract drawings
6. All addenda issued by the City prior to receipt of bids
7. Bid of the Contractor
8. Bid bond, performance, and payment bonds and maintenance bond, if any
9. Certificate of insurance
10. Notice of award
11. Resolution of City awarding the bid
12. Agreement
13. All provisions required by law to be inserted in the contract whether actually inserted or not.

Contract Amount. The total amount of the contract, including all contract revisions to date.

Contract Bond. The security, executed by the CONTRACTOR and the surety or sureties, furnished to the CITY to guarantee complete execution of the contract and all supplemental agreements and the payment of all legal debts pertaining to project construction.

Contract Item (Pay Item). A specific unit of the work for which the contract provides a price. During the bidding process, the term “bid item” may be used to describe these items.

Contract Unit Price. The price included in the contract for a contract item.

Contract Time. The amount of time allowed for completion of the contract, including authorized time extensions. The contract time will be a number of working days, a number of calendar days, a completion date, or a completion date with a minimum number of working days. The contract time may include milestones.

Contractor. The individual or legal entity contracting with the CITY for performance of prescribed work.

> **Employee.** Any person working on the project covered by the contract that is under the direction or control of, or receives compensation from, the CONTRACTOR or a subcontractor.
> **Subcontractor.** An individual or legal entity with whom the CONTRACTOR sublets part of the contract.
> **Superintendent.** The CONTRACTOR's authorized representative in responsible charge of the work.
> **Surety.** The legal entity or individual, other than the CONTRACTOR, executing a proposal guaranty or contract bond.

Employee. See CONTRACTOR.

Engineer. See CITY.

Equipment. All machinery, tools, apparatus, and supplies necessary for maintenance, construction, and completion of the work.

Extra Work. Work not provided for in the contract but considered essential by the ENGINEER for satisfactory completion of the contract within its intended scope.

Final Completion. A project is considered as final complete when all construction, including all punch list items, is fully complete and accepted by the ENGINEER.

Force Account. Payment for contract revisions per approved costs and additives.

Haul Road. Highways, streets, or roads designated by the CITY for use by the CONTRACTOR to haul material to or from the project.

Highway, Street, or Road. A general term indicating a public way used by vehicles and pedestrians. Includes entire area within the right of way.

Holidays. City of Bismarck holidays are as follows:

1. Every Sunday;
2. New Year's Day, January 1;
3. Martin Luther King Jr. Day, the third Monday of January;
4. Presidents' Day, the third Monday of February;
5. Good Friday, the Friday before Easter Sunday;

6. Memorial Day, the last Monday in May;
7. Independence Day, July 4;
8. Labor Day, the first Monday in September;
9. Veterans Day, November 11;
10. Thanksgiving Day, the fourth Thursday in November;
11. Christmas Day, December 25; and
12. Every day appointed as a public holiday by the president of the United States or the Governor of the State

If January 1, July 4, November 11, or December 25 fall on a Sunday, the following Monday is a holiday.

If January 1, July 4, November 11, or December 25 fall on a Saturday, the previous Friday is a holiday.

Materials. Any substances, products, supplies, assemblies, or raw materials specified for use in the performance of the work.

Median. The portion of a divided street separating the traveled ways.

Notice to Bidders. A notice issued by the CITY of projects available for proposals in an upcoming bid opening.

Notice to Proceed. The CITY's notice to the CONTRACTOR to begin the work.

Pavement Structure. The combination of subbase, base course, and surface course placed on a subgrade to support and distribute the traffic load to the roadbed.

Plan Quantity. The quantity of a contract item shown on the bid item list and the Plans.

Plans. The Project Plans and Standard Drawings that show the location, character, and dimensions of the prescribed work, including layouts, profiles, cross sections, and other details.

Project. The specific section of infrastructure on which construction is to be performed under the contract.

Project Number. A number generated by the CITY containing coded project data. Found on the cover sheet of the Plans.

Project Site. All areas used by the CONTRACTOR in the performance of the work.

Proposal. (Commonly referred to as Bid) A Bidder's offer on CITY forms, to perform the work at the prices quoted.

Proposal Forms. The CITY-provided forms on which a Bidder must prepare and submit its Proposal for the work. That portion of the Proposal package containing certifications,

affidavits, acknowledgements, and the bid item list. The Proposal form is prepared and submitted by the Bidder and is then considered a Proposal.

Proposal Package. All documents made available to prospective Bidders by the CITY before the opening of Proposals. These documents will become part of the contract.

Request for Proposals. A publication addressing the work required for the project. The request for proposals may include the Special Provisions and the Proposal forms.

Responsive Proposal. A Proposal that meets all requirements of the Proposal package.

Responsible Bidder. A Bidder who has met all of the CITY's prequalification requirements and submits a qualifying bid on a project.

Retainage. Withholding of a portion of the final payment to assure the CONTRACTOR has finished a project completely and satisfactorily.

Right-of-Way. A general term denoting land, property, or interest therein, acquired for or devoted to a roadway.

Roadbed. The graded portion of a highway, street or road, within top and side slopes, prepared as a
foundation for the pavement structure.

Shop Drawings. Supplemental design sheets or similar data, such as drawings, diagrams, illustrations, samples, schedules, or calculations, that the contract requires the CONTRACTOR to submit to the ENGINEER. Once the CITY has reviewed a work drawing, it becomes part of the contract.

Sieve. U.S.A. Standard Sieve, as defined in ASTM E11. The specified percent passing for each sieve is measured by weight.

Special Provisions. See Specifications.

Specifications. The compilation of written requirements for performance of the work, including the following:

> **Standard Specifications.** A book of specifications approved for general application and repetitive use.
>
> **Special Provisions.** Revisions or additions to the standard specifications that cover special conditions for the project.

Stabilization. The modification of soils or aggregates by incorporating materials that increase load-bearing capacity, firmness, or resistance to weathering or displacement.

Standard Drawings. An approved set of drawings showing Standard Details of construction and materials for the work on a project.

Standard Specifications. See Specifications.

Station. When used as a definition or term of measurement, a station is 100 linear feet.

Structures. Bridges, culverts, catch basins, drop inlets, retaining walls, cribbing, manholes, endwalls, buildings, sewers, service pipes, underdrains, foundation drains, and similar features that may be encountered in the work.

Subcontractor. See CONTRACTOR.

Subbase. The layers of specified or selected materials of designated thickness placed on a subgrade to support a base course.

Subgrade. The top surface of an embankment or cut section on a graded roadway. It is the foundation for the subbase, base course, and surface course.

Substantial Completion. A project is substantially complete when it is operational and ready for use by the CITY.

Superintendent. See CONTRACTOR.

Surety. See CONTRACTOR.

Surface Course. One or more layers of a pavement structure designed to accommodate the traffic load, the top layer of which resists skidding and traffic abrasion. The top layer is sometimes called “Wearing Course.”

Traffic. Vehicles, pedestrians, and other modes of transportation.

Total Sum Bid. The total amount of a Proposal; the sum of the price extensions for all bid items.

Work. The providing of all labor, materials, equipment, and incidentals necessary to complete the project in accordance with the contract.

Work Order. A written directive from the ENGINEER to the CONTRACTOR to perform changed work, extra work, or other additional work.

SECTION 102 – BIDDING AND CONTRACT DOCUMENTS

102-1 PROPOSALS

No bids received after the time set for the receipt of the proposals will be considered. The right is reserved to hold all bids for a period of 30 days and to reject any or all bids. Bidders are invited to be present at the opening of Proposals.

102-2 FORM OF PROPOSAL AND SIGNATURE

The Proposal must be made on forms provided for that purpose, or forms provided by the Bidder which follow the same format, enclosed in a sealed envelope, and marked and addressed as required in the Advertisement. It must state the unit prices and the sum of money for which the Bidder proposes to supply the materials and perform the work called for in the Proposal and Schedule of Work. Bidders shall submit a bid on a unit price basis for each item of work so listed in the Proposal. The total of all estimated prices will be determined as the sum of the products of the estimated quantity of each item and the unit price bid for the item. If the bid is made by an individual, it must be signed with the full name of the Bidder whose address must be given. If made by a firm, it must be signed in the co-partnership name by a member of the firm, and the name and address of each member of the firm must be given. If made by a corporation, it must be signed by an officer of the corporation in the corporate name, and the corporate seal must be attached to such signature.

The Bidder may substitute a computer-printed spreadsheet of the Bid Schedule for the CITY-furnished Bid Schedule found in the Proposal. The substitute Schedule shall be attached to the last page of the CITY-furnished Bid Schedule in the Bidder's Proposal.

The following information shall appear on top of each page of the computer-printed Bid Schedule:

1. Improvement District Number or Project Number.
2. Date of Bid Opening.
3. Type and Description of Work (i.e., Sanitary Sewer, Water Main, Storm Sewer, and Incidentals).
4. Page Number.
5. Bidder's Name and Address.
6. Acknowledgement of Addenda.

The substitute Bid Schedule shall be printed on sheets of approximately the same size as the Bid Schedule in the Proposal, and the words and numerals shall be clear and legible. Each page shall be arranged, numbered, and contain the same bid items as the corresponding Bid Schedule in the Proposal. Column headings shall be the same as those in the CITY-furnished Bid Schedule.

Each bid item shall be separated from the bid items above and below by one or more blank spaces. Solid lines for separating columns and items are not required, but

dashed lines may be placed either vertically or horizontally.

The total sum of the bid shall be at the bottom of the last page of the computer-printed Schedule. CONTRACTOR shall initial in ink next to the total sum.

The bidder, or authorized representative, shall sign the substitute Bid Schedule in ink on the last page of the computer printout. The signer's name and title shall be printed below or beside the signature. The person signing the substitute Bid Schedule above shall also sign and complete the Affidavit in the Bidder's Proposal, as regularly required.

In case of discrepancies between item descriptions or quantities in the CITY-furnished Bid Schedule in the Proposal and those in the computer-printed Bid Schedule, the CITY-furnished Bid Schedule in the Proposal will govern. Any omitted items or missed items will be considered as "zero," and no payment will be considered for that item.

102-3 BID BOND

Unless otherwise specified, each bid shall be accompanied by a bid bond in the amount of 5 percent of the amount of the bid, meeting the requirements of Section 48 of the North Dakota Century Code, as amended.

The bid bond shall be conditioned such that the principal's bid is valid for a minimum of 30 calendar days, or such time as detailed in the project documents, and if accepted and a contract is awarded, the principal shall return the executed contract within ten days of receipt of the notice of award.

102-4 CONTRACT BOND

The Performance and Payment Bond required in Section 102 shall not be included as a separate item, but shall be incidental to the project.

102-5 CONTRACTOR'S INSURANCE

The CONTRACTOR shall not commence work under the contract until a "Certificate of Insurance" has been obtained and submitted to the CITY for all insurance required under this section and proof of such insurance has been delivered to the CITY, nor shall the CONTRACTOR allow any Subcontractor to commence on any subcontract until all similar insurance required of the Subcontractor has been obtained and proof has been delivered to the CITY.

(a) Compensation Insurance. The CONTRACTOR shall take out, and maintain during the life of the contract, Workers Compensation Insurance for all of CONTRACTOR's employees employed at the site of the project. In case any work is sublet, the CONTRACTOR shall require the Subcontractor similarly to provide Workers Compensation Insurance for all of the latter's employees unless such employees are covered by the protection afforded by the CONTRACTOR. In the case of employees engaged in hazardous work under the contract, at the site of the project, who are not

protected under the Workers Compensation statute, the CONTRACTOR shall provide and shall cause each subcontractor to provide Employer's Liability Insurance for the protection of its employees not otherwise protected.

(b) Public Liability and Property Damage Insurance. The CONTRACTOR shall take out, and maintain during the life of the contract, such Public Liability and Property Damage Insurance as shall protect it, the CITY, and any Subcontractor performing work covered by the contract, for claims and damages for personal injury including accidental death and including the coverage for "Assault and Battery" as well as from claims for property damage (including damage to CITY's property), which may arise from operations under the contract, whether such operations by itself or any Subcontractor or by anyone directly employed by either of them to, from, or on the site and the amounts of such insurance shall be as follows:

Public Liability Insurance in an amount not less than $500,000 per person and in an amount not less than $2,000,000 per occurrence for personal injuries, etc., including accidental death to any person, and Property Damage Insurance not less than $2,000,000. Where excavation, trenching, or tunneling is involved, the Property Damage Liability Coverage under the Comprehensive General Liability Policy shall specifically provide coverage for damage to underground property. The CITY OF BISMARCK shall be named as an additional insured on all the policies required under this section, with a waiver of subrogation in favor of the CITY OF BISMARCK.

(c) Satisfactory Coverage. In the event the form of any policy or certificates or the amount of the insurance or the companies writing same are not satisfactory to the CITY, the CONTRACTOR shall obtain new policies or certificates in compliance with these Specifications. The CONTRACTOR shall not cause any policies to be canceled or to permit them to lapse, and all insurance policies shall include a clause to the effect that the policy shall not be canceled or changed until 30 days after the CITY has received written notice as evidenced by the return receipt of the registered letter.

(d) Proof of Insurance. "Certificates of Insurance" shall contain true transcripts from the policy, authenticated by the proper officer of the insurer, evidencing in particular those insured, the extent of the insurance, the locations and operations to which the insurance applies, the effective date and expiration date, and the notice of cancellation clause mentioned herein above.

~~**(e) Builder's Risk Insurance.** The CONTRACTOR will maintain Builder's Risk Insurance or like insurance coverage (fire and extended coverage) on a 100 percent completed value basis on the insurable portion of the project for the benefit of the CITY, the CONTRACTOR, and all Subcontractors, as their interest may appear.~~

102-6 DEBARMENT CERTIFICATION

As required by Bismarck City Ordinance, all suppliers, CONTRACTORs, and service providers doing business with the CITY must certify that they are in compliance with all federal, state, and local laws, regulations, and orders including, but not limited to, those

regarding non-discrimination, wages and hours, Workers Compensation, and immigration. Failure of compliance may result in the cancellation of any CITY contract and exclusion from consideration for future contracts.

By submission of a bid or proposal, the bidder or proposer certifies, to the best of its knowledge and belief, that it and its principals:

(a) Are not presently debarred, suspended, proposed for debarment, declared ineligible, or voluntarily excluded from covered transactions by any federal department or agency;

(b) Have not, within a three year period preceding this certification, been convicted of or had a civil judgment rendered against them for commission of fraud or a criminal offense including, but not limited to, a violation of federal or state antitrust statutes, or commission of embezzlement, theft, forgery, bribery, falsification or destruction of records, making false statements, immigration violations, or receiving stolen property in connection with obtaining, attempting to obtain, or performing a public (federal, state, or local) contract;

(c) Are not presently indicted for or otherwise criminally or civilly charge by a governmental entity (federal, state, or local) with commission of any of the offenses listed in subparagraph (b) of this certification; and

(d) Have not within a three year period preceding this certification had one or more public contracts (federal, state, or local) terminated for cause or default.

The bidder or proposer also certifies that if it later becomes aware of any information contradicting the statements above, it will provide that information to the CITY.

If the bidder or proposer is unable to certify to all statements in this certification, it shall indicate so in its bid or proposal or in a transmittal letter or message accompanying its bid or proposal and provide a written explanation to the CITY.

102-7 LOCAL CONDITIONS

Bidders shall satisfy themselves as to the nature of the material to be handled and the local site conditions affecting the work, and if conditions are found to be different than anticipated by the CONTRACTOR subsequent to the signing of the contract, it shall not in any way relieve the CONTRACTOR from its obligation or any risks from the fulfillment of all the work and terms of the contract.

102-8 INDEMNITY AGREEMENT FOR CONTRACTORS

The CONTRACTOR agrees to indemnify and hold harmless the CITY OF BISMARCK, its appointed and elective officers and employees, from and against any and all loss or expense, including attorney's fees and costs by reason of liability imposed by law upon the CITY, its elected or appointed officials or employees, for damages because of bodily

injury including death at any time resulting therefrom sustained by any person or persons and on account of damage to property including loss of use thereof, arising out of or in consequence of the performance of this work, whether such injuries to persons or damage to property is due to the negligence of the CONTRACTOR, its agents or employees, its subcontractors, their employees, CITY OF BISMARCK, its appointed or elected officers, employees, or their agents, except only such injury or damage as shall have been occasioned by the sole negligence of the CITY, its appointed or elected officials or employees.

102-9 REQUEST FOR ALTERNATE SPECIFICATIONS

The reference to manufacturer's name and catalog or model numbers shall be interpreted as establishing a standard of quality, not as limiting competition.

Pre-approved materials and equipment are listed in the Specification or Standard Drawings. CONTRACTORS intending to price material or equipment not referenced in Specifications or Standard Drawings shall request in writing, to the CITY ENGINEER, to have the material or equipment recognized as an approved equivalent. The CONTRACTOR must include complete descriptive technical data on the proposed item consisting of: model numbers, type, size, and performance characteristics. Procedure also applies to requests by suppliers.

The request for consideration of an approved equivalent must be provided to the CITY ENGINEER no later than 10 days prior to bid opening, unless otherwise specified. All equivalents approved for bid may be listed in addenda sent to all plan holders in advance of bid opening.

CONTRACTORS choosing to use material or equipment other than those shown on Drawings or specified in detail, but approved for bid, shall be responsible for physical dimensions and coordination. The CITY OF BISMARCK will not be responsible for costs of necessary changes and additional work required by the CONTRACTOR or any other trades arising from such use.

If the alternate is deemed unacceptable to the ENGINEER, the bidder may request, in writing, that the matter be scheduled for consideration by the Board of City Commissioners. Such request must be made to the City Administrator no later than 7 days prior to the Board of City Commissioners meeting set for the award date. Requests for consideration by the Board of City Commissioners after that date shall not be honored.

102-10 AWARD AND CONTRACT SECURITY

The bidder to whom the award is made will be required to enter into a written contract with the CITY OF BISMARCK as required by Section 48 of the North Dakota Century Code. Pursuant to NDCC Section 48, simultaneously with the CONTRACTOR's delivery of the executed contract, the CONTRACTOR shall furnish a Performance Bond from a responsible surety in an amount not less than 100 percent of the total contract

amount as security for the faithful performance of the contract and a Payment Bond from a responsible surety in an amount not less than 100 percent of the total contract amount as security for the payment of all persons performing labor on the project under the contract and furnishing materials in connection with the contract.

After the proposals are opened and read, the products of the quantities and the respective unit prices bid and the summation of said products in each Proposal will be verified or corrected. In case of discrepancy, the bidder's apparent intent indicated shall govern. Discrepancies between the multiplication of units of work and unit prices will be resolved in favor of the unit prices. Discrepancies between the indicated sum of any column of figures and the correct sum thereof will be resolved in favor of the correct sum. Discrepancies between words and figures will be resolved in favor of the words. However, if the bidder's intent is not apparent, the Proposal will be rejected. The verified or corrected totals of the Proposals considered will be compared and the results of such comparison made public. Until the award of the contract, however, the right will be reserved to reject any and all proposals and to waive technicalities as may be deemed best for the interests of the CITY. All quantities are estimated and shall be equal per each item for comparison of bids per each unit of area. Addenda quantities shall govern over bid Proposal quantities which shall govern over Plan quantities.

The award of the contract, if made, will be to the lowest responsible bidder whose Proposal complies with all the requirements specified. The award, if made, will be made within the time specified in the Advertisement for Bids unless an extension of this limit is agreed to in writing by both parties. In the case of participation in the project by federal and/or state government, or any agency, subdivision, or other participating party, or if concurrence of the aforementioned parties is required by law, any award made by the Board of City Commissioners shall be deemed subject to concurrence of the participating and/or regulatory parties.

If the project includes more than one unit or contains alternates, the basis of award shall be the lowest and best bid for the units or alternates selected by the CITY. Units or alternates not selected shall not be included in forming the basis of determining the lowest bid. Bidders shall be aware that there is no guarantee that all units or alternates will be awarded and balance their bids accordingly.

Prior to the CITY execution of the contract, the successful (or apparent low) bidder shall submit to the CITY ENGINEER a schedule of proposed progress showing the proposed starting and completion dates and with curves showing the percentage of the major features of the work scheduled for completion at any date together with a composite curve showing the percentage of the entire contract which will be completed at any date.

The proposed Progress Schedule shall show the starting date and the number of working days deemed necessary by the CONTRACTOR to complete the work on or before the completion date shown in the Proposal.

The number of working days shown on the Progress Schedule shall not exceed the number of calendar days, excluding Sundays and holidays, between the proposed starting date and the completion date shown in the Proposal.

After the proposed Progress Schedule, Payment and Performance Bonds, Certificate of Insurance, and any other required documents have been submitted to the CITY ENGINEER, they will be reviewed and forwarded to the CITY ATTORNEY with a recommendation to execute the contract. By entering into a contract, the CONTRACTOR represents that it has carefully reviewed the Plans, Specifications, and General and Special Provisions and has inspected the site conditions and that it has the capability to complete a good and workmanlike project in conformance with the Plans, Specifications, and General and Special Provisions.

As provided by North Dakota statutes, no contract will be awarded to any CONTRACTOR who is not the holder of a current license in the class within which the value of the project falls. A foreign corporation must have a Certificate of Authority to do business in North Dakota before a contract can be awarded to said corporation.

The CITY OF BISMARCK reserves the right to cancel the award of any contract at any time before the execution of said contract by all parties without any liability against the CITY.

All bidders' bonds, except in case of defaults, will be returned, upon request, within a reasonable time and as provided by law.

All bidders should note that after the award of the contract to the lowest bidder is approved by the Board of City Commissioners, and the contract is fully executed, ALL bid documents submitted to the CITY will be destroyed utilizing standard office practices, with the exception of the bid of the successful bidder. Should a non-successful bidder want its bid documents returned, it should include a self-addressed, postage-paid envelope with the bid, or request that the bid documents be saved in a self-addressed envelope included with the bid to be picked up at the Engineering Department upon the signature of the bidder.

SECTION 103 – SCOPE OF WORK

103-1 SUBCONTRACTING

All work performed under the contract shall be by the company or firm to which the contract is awarded, and no portion of the work shall be awarded to a subcontractor unless authorized in writing by the ENGINEER acting on the approval of the Board of City Commissioners. The CONTRACTOR shall be responsible for the coordination and control of the subcontractor(s).

103-2 MOBILIZATION

This work consists of preparatory work and operations, including movement of personnel, equipment, and supplies and establishment of offices, CONTRACTOR's buildings, and facilities necessary for work on the project. This work and all other work and operations which must be performed along with all costs incurred before the beginning work on the project site shall be incidental to the project.

103-3 CHANGES

The Board of City Commissioners reserves the right to make any changes in the alignment, grade, or design as may be deemed advisable, and should any changes so made cause the CONTRACTOR extra expense or operate to decrease CONTRACTOR's expense, the ENGINEER shall make due allowance, as agreed upon by ENGINEER and CONTRACTOR, which action shall be binding upon both parties. The CONTRACTOR with whom the contract for the execution of the work is made will be required to make any extension which the Board of City Commissioners may require. The extensions shall be constructed at the same unit price for the same class of work as bid upon for this work, provided that should the prices of materials be increased or diminished over the prices of the same materials at the present time for the same class of work, the ENGINEER shall make due allowance. The action shall be binding upon both parties and provided further that such extensions shall be ordered prior to the completion of the contract.

103-4 PATENTS

The CONTRACTOR will be held responsible and be required to make good at the CONTRACTOR's expense any and all damages and suits for damages caused by infringements of the patent rights on devices or equipment for the requirements of the contract and is to indemnify and hold harmless the CITY OF BISMARCK from all claims, damages, or expenses by the use thereof. All fees and royalties covering the same are to be included in the price bid by the CONTRACTOR for the work to be done under the Specifications.

103-5 TRAFFIC CONTROL DEVICES

The CONTRACTOR is expected to be familiar with all federal, state, and local laws, codes, ordinances, and regulations which in any manner affect those engaged in the work or the materials or equipment used in or upon the site or in any way affect the conduct of the work. No pleas of misunderstanding or ignorance on the part of the CONTRACTOR will in any way serve to modify the provisions of the contract. The CONTRACTOR shall provide and maintain on a 24 hour basis all necessary safeguards and traffic control devices at its own expense.

The CITY OF BISMARCK has adopted the U.S. Department of Transportation Manual on Uniform Traffic Control Devices, 2009 Edition, or latest Edition, and all revisions, for all traffic control devices and their placement. For all materials and equipment used for

traffic control on all construction projects in the CITY OF BISMARCK, the CONTRACTOR shall comply with Section 704 of the Standard Specifications for Road and Bridge Construction and the Design Standard Drawings of the North Dakota Department of Transportation. The documents referred to above are available at the City of Bismarck Engineering Department.

When detours for roadway closures are not incorporated within the Plans or are required because of an emergency situation, water main break, sewer collapse, etc., the CONTRACTOR shall submit a traffic control plan to the ENGINEER for review and approval.

SECTION 104 – CONTROL OF WORK

104-1 ENGINEER

The ENGINEER is the authority on the engineering details of the project and the administrative responsibilities for the satisfactory completion of the project. The ENGINEER will give the grades and locations for all work, and no work depending upon such grades or locations shall be commenced until after the same have been established. Upon all questions concerning the interpretations of these Specifications or the plans, the decision of the ENGINEER shall be binding upon both parties. Detailed Plans of all work not completely shown on the Plans now on file will be furnished by the ENGINEER from time to time, and the work shall be executed in accordance with such detailed plans.

The ENGINEER has authority to reject defective material or work that does not meet the contract requirements or is not installed per manufacturer's recommendations. The ENGINEER has the authority to suspend the work for the following reasons:

a. The CONTRACTOR fails to carry out contract requirements;
b. The CONTRACTOR fails to carry out orders from the ENGINEER;
c. During periods of unsuitable weather;
d. For conditions considered unsuitable for performance of the work;
e. For other conditions or reasons in the public interest; or
f. For other reasons the ENGINEER and CONTRACTOR mutually agree on.

104-2 CONTRACTOR

The foreman of the CONTRACTOR in charge of the work will be held to represent the CONTRACTOR during the absence of the latter or CONTRACTOR's legal representative. Instructions given to the CONTRACTOR's foreman on the work by the ENGINEER will be held as having been given to the CONTRACTOR.

104-3 CHARACTER OF WORKMEN

If any person employed on the project, whether a CONTRACTOR'S employee or not, is intemperate, prejudiced, abusive, or disorderly, the ENGINEER may direct the CONTRACTOR in writing to discharge the person from the work. Re-employ this person

on the project only with the ENGINEER'S approval. If the CONTRACTOR fails to remove a person as directed by the ENGINEER or to provide sufficient personnel for the proper execution of the work, the ENGINEER may suspend the work by written notice until the CONTRACTOR complies.

104-4 METHODS AND APPLIANCES

The methods and appliances adopted by the CONTRACTOR shall be such as will enable the CONTRACTOR to secure a satisfactory quality of work and complete the work within the time specified. The choice of methods and appliances to complete the work in compliance with the Plans and Specifications is solely the CONTRACTOR's. It is the responsibility and obligation to produce a complete project that fully complies with the Plans and Specifications and is of satisfactory quality. The ENGINEER may at any time inform the CONTRACTOR of apparent deficiencies in the work, and the CONTRACTOR will make whatever adjustments are, in the CONTRACTOR's judgment, necessary to bring the work back into conformance. Failure of the ENGINEER to so advise the CONTRACTOR shall not in any way relieve the CONTRACTOR from its obligations which shall remain in full force and effect until the discharge of the contract. The Board of City Commissioners of the CITY OF BISMARCK reserves the right, in case of improper construction, to suspend the work at any time and to reject the work or to order the reconstruction of any part or all of the work improperly done.

104-5 MONUMENTS, BENCH MARKS, WITNESS AND GRADE STAKES

All monuments, bench marks, and witness and grade stakes are the property of the CITY, and in the event of the destruction or removal by the CONTRACTOR, such stakes shall be replaced by the ENGINEER at the CONTRACTOR's expense. Any interruption of work and/or costs incurred by the CONTRACTOR due to any delays caused during the replacement of destroyed monuments, bench marks, and witness and grade stakes shall be borne by the CONTRACTOR. The CONTRACTOR shall be responsible for notifying the ENGINEER a minimum of 72 hours prior to the expected survey.

104-6 CONTRACTOR'S RESPONSIBILITIES

Unless otherwise specified, the CONTRACTOR shall furnish all labor, materials, and equipment necessary for the completion of the Schedule of Work in accordance with the Plans and Specifications. The CONTRACTOR shall do all necessary hauling and perform all labor, incidental thereto, for which no express provisions have been made. The CONTRACTOR shall assume all risks or damages to persons or property prior to the final acceptance of the work. The CONTRACTOR shall so conduct its operation as not to interfere with the work of other contractors in the vicinity. The CONTRACTOR shall maintain at all times an efficiently sized crew headed by a competent construction foreman and the necessary skilled labor to efficiently complete the work.

The CONTRACTOR shall be responsible for maintenance and operation of all constructed facilities until final acceptance unless otherwise noted in Specifications,

notes, or Special Provisions. This includes locating of CONTRACTOR-constructed underground facilities.

104-7 SHOP DRAWINGS

Before any of the materials are delivered to the job, the CONTRACTOR shall submit to the CITY ENGINEER complete Shop Drawings.

The Shop Drawing submittal shall include two copies in paper format or one electronic copy in PDF format. An approved electronic copy will be sent to the CONTRACTOR upon approval, paper format of approved drawings will not be provided unless requested by the CONTRACTOR.

Shop Drawings shall be submitted for all types of supplied materials including water, sanitary sewer, storm sewer, and electrical. The Shop Drawings shall include catalog numbers, performance data, dimensions, and other descriptive information.

Paper format Shop Drawings may be in the form of printed catalog sheets showing all necessary information and shall be bound together, neatly indexed, and tabbed.

Each Shop Drawing folder or set of drawings shall be stamped, initialed, and dated, electronically or on hard copy, by CONTRACTOR to indicate it has thoroughly reviewed them.

The CITY review of Shop Drawings is for general compliance with contract documents. The CITY review does not relieve the CONTRACTOR from responsibility for errors, omissions, or deviations from contract requirements.

Shop Drawings not in conformance with the Specifications may be returned to the CONTRACTOR without review.

104-8 CONFORMITY WITH PLANS & SPECIFICATIONS

Unless specific tolerances are specified, all work performed and all materials furnished shall be in reasonably close conformity with the lines, grades, cross sections, dimensions, and material requirements shown on the Plans or indicated in the Specifications.

Plan dimensions and contract specification values are to be considered as the target value from which any deviations are allowed. It is the intent of the Specifications that the materials and workmanship shall be uniform in character and shall conform as nearly as realistically possible to the prescribed target value or to the middle portion of the tolerance range. The purpose of the tolerance range is to accommodate occasional minor variations from the median zone that are unavoidable for practical reasons. When a maximum or minimum value is specified, the production and processing of the material and the performance of the work shall be so controlled that the material or work will not be preponderantly of borderline quality or dimension.

In the event the ENGINEER finds the materials or the finished product in which the materials are used are not within reasonably close conformity with the Plans and Specifications but that reasonably acceptable work has been produced, the ENGINEER will then make a determination if the work will be accepted and remain in place. In this event, the ENGINEER will document the basis of acceptance by contract modification which will provide for an appropriate adjustment in the contract price for such work or materials as the ENGINEER deems necessary to conform to a determination based upon engineering judgment.

In the event the ENGINEER finds the materials or the finished product in which the materials are used or the work performed are not in reasonably close conformity with the Plans and Specifications and have resulted in an inferior or unsatisfactory product, the work or materials shall be removed and replaced or otherwise corrected by and at the expense of the CONTRACTOR.

104-9 DELAYS

The CONTRACTOR will not be entitled to any compensation for foreseeable or unforeseeable causes resulting in delays or hindrances to the work. Extensions of time will be granted for unavoidable delays, which in the opinion of the ENGINEER are clearly beyond the control of the CONTRACTOR and outside of normal occurrences including, but not restricted to, acts of God or of the public enemy, acts of the CITY, acts of another CONTRACTOR in the performance of a contract with the CITY, fires, epidemics, quarantine restrictions, strikes, freight embargoes, and abnormal weather. The ENGINEER must receive a written request for time extension from the CONTRACTOR not more than 20 days after commencement of delay before any time extension will be considered. Requests made beyond the 20-day limit will be cause for denial. Any extension of time will not relieve the CONTRACTOR or its sureties from their obligations which shall remain in full force and effect until the satisfactory discharge of the contract.

Extra time may be granted to complete the project due to material delays at the discretion of the ENGINEER. No monetary compensation shall be made for material delays.

104-10 MULTIPLE CONTRACTS ON SAME SITE

When different types of construction work on the same section of public right-of-way or site are let under separate contracts, the CONTRACTORS shall cooperate with each other to the fullest extent possible that the prosecution of the work under each contract will be carried out for the best interests of the CITY. The CITY assumes no liability for any delay caused by any CONTRACTOR, its subcontractor(s) or supplier(s), to any other CONTRACTOR, its subcontractor(s) or supplier(s).

104-11 COOPERATION BETWEEN CONTRACTORS

The CITY reserves the right to contract for and perform other work on or near the site of work and coordinate the work and cooperate with the CONTRACTOR for the other work. The CONTRACTOR is responsible for all liability, financial or otherwise, in connection with the contract, and shall hold the CITY harmless from damages or claims resulting from inconvenience, delay, or loss due to the CONTRACTOR's failure to coordinate the work or cooperate with the CONTRACTOR for the other work. If a conflict occurs between the CONTRACTOR for the other work, the ENGINEER will provide direction.

104-12 TRANSPORTATION OF MATERIALS

The CONTRACTOR is authorized to ship all construction materials which are to be incorporated into the project to the CITY OF BISMARCK in care of the CONTRACTOR. Such materials are exempt from the federal tax on transportation of said materials. The exemption of federal tax does not apply to shipments of fuel, lubricants, spare parts, and items of construction equipment belonging to the CONTRACTOR and which will not be incorporated into the construction project and which will not become the property of the CITY OF BISMARCK. This authorization is granted with the distinct understanding that the CITY OF BISMARCK will receive all benefits from the exemption from payment of the tax. The tax is not included in the CONTRACTOR's bid, and all transportation charges shall be paid by the CONTRACTOR.

104-13 EXTRA WORK

The CONTRACTOR shall perform extra work for which there is no price in the contract whenever it is deemed necessary or desirable in order to complete fully the work as contemplated. If the CONTRACTOR contends that additional compensation is due for work or material not clearly covered in the contract, the CONTRACTOR shall promptly notify the ENGINEER in writing of the intention to file a claim and the basis for additional compensation before beginning or continuing construction on the affected work. If the basis for the claim does not become apparent until after proceeding with the work, and it is not feasible to stop the work, the CONTRACTOR shall immediately notify the ENGINEER that work is continuing and that written notification of the intent to file a claim will be submitted within 10 calendar days. Failure to give the required notification or to provide the ENGINEER proper facilities and assistance in keeping strict account of actual costs will constitute a waiver of the claim for additional compensation in connection with the work already performed. Notification of a claim, and the fact that the ENGINEER has kept an account of the costs involved, shall not be construed as proving or substantiating the claim's validity. Such work shall be performed in accordance with the Specifications. Work contained in the Plans and Specifications shall not be considered extra work and shall not be paid for by the CITY as such unless specifically agreed to in writing.

When work not shown on the plans is to be performed by the CONTRACTOR, the ENGINEER and the CONTRACTOR shall determine compensation based on mutually agreed prices, on a per unit or lump sum basis, for the extra work.

The ENGINEER may request the CONTRACTOR to provide an estimate of the proposed unit prices or a lump sum for the extra work. The ENGINEER may request the CONTRACTOR to justify the estimate by providing one or more of the following: (a) Labor requirements by trade in hours for each task, (b) equipment costs and time requirements, (c) material costs, and (d) any additional costs.

If the prime CONTRACTOR uses a subcontractor to exclusively perform the revised work, the CITY shall pay the prime CONTRACTOR an additional markup on the subcontractor's agreed upon price as per section (f) "Subcontracting" below.

If the ENGINEER and the CONTRACTOR cannot agree on compensation for the extra work, or when it is considered to be in the best interest of the CITY, the ENGINEER may order the work done on a force account basis. Compensation for force account work shall be justified in the following manner:

(a) Labor. For all laborers (skilled and unskilled) and foremen in direct charge of the specific operations, the CONTRACTOR shall receive the rate of wage (or scale) agreed upon in writing before beginning work for each and every hour that said laborer and foremen are actually engaged in such work.

The wages of any foreman who is employed partly on the force account work and partly on other work will be prorated according to the number of workers in the two classes of work as shown by the payrolls.

The CONTRACTOR shall receive the actual costs paid to, or on behalf of, workmen by reason of subsistence and travel allowances, health and welfare benefits, pension fund benefits, or other benefits, when such amounts are required by a collective bargaining agreement or other employment contracts generally applicable to the classes of labor employed on the work, but excepting any amounts which are already included in the wage rates paid. Any subsistence or travel allowance paid to the workmen shall be prorated according to the number of hours employed on the force account and other classes of work.

An amount equal to 20 percent of the sum of the above items will also be paid to the CONTRACTOR.

(b) Bond, Insurance, and Tax. For premiums paid on additional bond, property damage, liability, and workers compensation insurance contributions, and Social Security Taxes on the force account work, the CONTRACTOR shall receive the actual cost, to which cost 6 percent will be added. The CONTRACTOR shall furnish satisfactory evidence of the rate or rates paid for such bond, insurance, and tax.

(c) Materials. For materials accepted by the ENGINEER and used, the CONTRACTOR shall receive the actual costs of such materials delivered on the work, including transportation charges paid by the CONTRACTOR (exclusive of machinery rentals as hereinafter set forth), to which cost 15 percent will be added plus any sales tax paid by the CONTRACTOR. For all materials used in connection with, but not entering permanently into the work, reasonable depreciation will be allowed.

(d) Equipment. For the use of authorized equipment and additional traffic control devices required by the force account work, the CONTRACTOR will receive rental rates determined in accordance with the *Rental Rate Blue Book* published by Equipment Watch. No percentage shall be added to these rates. No allowance will be allowed for equipment replacement or replacement escalators, cost of facilities capital, interest, small tools, or any other additives not listed. All equipment hours will be paid for as straight time. The only equipment payments that will be made are as follows:

(1) Owned Equipment. Payment for the actual hours of CONTRACTOR-owned equipment will be at 70 percent of the hourly ownership cost determined in accordance with the *Rental Rate Blue Book*. The hourly ownership cost equals the regionally adjusted monthly ownership cost divided by 176 hours per month.

The computed hourly equipment cost times the number of hours claimed shall not exceed the CONTRACTOR's actual purchase price for the piece of equipment being claimed.

Subcontractor-owned equipment will be paid for in the same manner as CONTRACTOR-owned equipment unless such equipment has been rented, leased, or hired by the CONTRACTOR, as provided for in (2) below.

(2) Leased, Rented, or Hired Equipment. Payment for leased, rented, or hired equipment shall be the actual invoice payment plus sales tax as verified by paid invoices signed by the lessor, or by checks issued by the CONTRACTOR. If the lease rental is weekly, the weekly rate shall be divided by 40 to get an hourly equipment cost for the claim. If the lease or rental is monthly, the monthly rate shall be divided by 176 to get an hourly equipment cost for the claim.

The computed hourly equipment cost, for each individual piece of equipment, times the number of hours claimed shall not exceed the CONTRACTOR's actual lease or rental cost for the time frame claimed.

(3) Idle Time. The number of hours of equipment use to be paid for will only be the hours that the equipment is operating on the claim item. No payment will be made for equipment on standby unless the standby is directed in writing by the ENGINEER, or the standby is proven to be as the direct result of the CITY's actions or inactions. Standby will be paid at 50 percent of the hourly base rate calculated by dividing the monthly rate by 176. The listed weekly, daily, or hourly rates will not be used. Operating costs will not be paid for hours of idle time.

Payment for standby time will not be made on any day the equipment operates for 8 or more hours. For equipment accumulating less than 8 hours operating time on any normal work day, standby payment will be limited to only that number of hours which, when added to the operating time for that day, equals 8 hours. Standby payment will not be made in any case on days not normally a work day.

The above rental rates to be paid on equipment will be on the size normally used to operate the equipment, subject to approval of the ENGINEER. The above rental rates include gas, oil, repairs, and any other incidentals necessary for the operation of the equipment but do not include the operators. No work will be paid for until unit prices, rental rates, and wages have been agreed upon in writing.

Procedures governing rented or owner-operated equipment, attachments and accessories, types and quantity of equipment, measurement of equipment time, use of equipment in excess of 50 hours per week, standby time, and equipment transportation charges will be as set forth in the *Rental Rate Blue Book*.

(e) Miscellaneous. No additional allowance will be made for general superintendence, the use of small tools, or other costs for which no specific allowance is herein provided.

(f) Subcontracting. For any extra work, including force account work, performed by a Subcontractor with the written authorization of the ENGINEER, the CONTRACTOR will receive an additional allowance for administrative and overhead expense. The additional allowance will be a percentage of the total extra work invoice equal to 10 percent of the first $10,000 plus three percent of the balance in excess of $10,000.

(g) Authority of ENGINEER. The ENGINEER has authority to require alterations in the equipment and labor force assigned to force account work, to limit authorization of overtime work to that normally used on the project for work of similar nature, or to require overtime when an emergency exists, and to require the cessation of force account work when adverse conditions severely limit productivity.

(h) Daily Records. The CONTRACTOR's representative and the ENGINEER shall compare records of the cost of work done as ordered on a force account basis at the end of each day for the purpose of resolving differences.

(i) Statements. No payment will be made for work performed on a force account basis until the CONTRACTOR has furnished the ENGINEER with duplicate itemized statements of the cost of such force account work detailed as follows:

1. Name, classification, date, daily hours, total hours, rate, and extension for each laborer and foreman.
2. Designation, dates, daily hours, total hours, rental rate, and extension for each unit of machinery and equipment.
3. Quantities of materials, prices, and extensions.

4. Transportation of materials.
5. Cost of property damage, liability and workers' compensation insurance premiums, unemployment insurance contributions, and Social Security Tax.

Statements shall be accompanied by receipted invoices for materials used including transportation charges paid by the CONTRACTOR. The statements shall be adjusted, when applicable, to reflect any discounts offered by the supplier. When materials used in the force account work are not specifically purchased for that work but are taken from the CONTRACTOR's stock, the CONTRACTOR shall furnish an affidavit certifying such materials were taken from stock, the quantity claimed was actually used, and that the price and transportation costs claimed are the CONTRACTOR's actual costs.

On or before the tenth day succeeding the completion of the extra work authorized by a "Work Order," the CONTRACTOR shall present to the ENGINEER the original "Work Order," together with a full and complete itemized statement of such extra work, with date of completion of the work mentioned therein. Upon certification by the ENGINEER or his authorized representatives as to the correctness of such items with regard to the amount and character of labor performed and materials furnished under such "Work Order," the ENGINEER shall enter the same as part of the estimate of the amount due the CONTRACTOR. The CONTRACTOR shall not be entitled to receive payment for any extra work in which he fails to present the "Work Order" within the time and in the manner hereinbefore mentioned.

The additional payment based on the percentages specified above shall constitute full compensation for all items of expense not specifically provided for the force account work. The total payment made as provided above shall constitute full compensation for such work.

104-14 OBSERVATION AND TESTING

All materials and equipment used in the construction of the project shall be subject to adequate observation and testing in accordance with generally accepted standards.

The CONTRACTOR shall provide, at ~~its~~ CONTRACTOR's expense, the necessary testing and inspection services required by the Plans and Specifications unless otherwise provided.

All concrete field testing personnel shall be certified through the American Concrete Institute (ACI) at the minimum level of an ACI Concrete Field Technician Grade I.

All independent testing laboratories provided by the CONTRACTOR shall be accredited independent testing laboratories (heretofore referred to as "independent testing laboratory") currently certified through the American Association of State Highway and Transportation Officials (AASHTO) accreditation program to perform the required testing and reporting procedures for the specific project.

All laboratory testing reports shall be supplied by the CONTRACTOR to the ENGINEER within 48 hours via email or as directed by the ENGINEER. Hard copies can also be delivered within 48 hours to the ENGINEER

If the Plans and Specifications, laws, ordinances, rules, regulations, or orders of any public authority having jurisdiction require any work to specifically be inspected, tested, or approved by someone other than the CONTRACTOR, the CONTRACTOR will give the ENGINEER timely notice of readiness. The CONTRACTOR will then furnish the ENGINEER the required certificates of inspection, testing, or approval.

Neither observations by the ENGINEER nor inspections, tests, or approvals by persons other than the CONTRACTOR shall relieve the CONTRACTOR from its obligations to perform the work in accordance with the requirements of the Plans and Specifications.

The ENGINEER will at all times have access to the work. In addition, authorized representatives and agents of any participating federal or state agency shall be permitted to inspect or observe all work, materials, payrolls, records of personnel, invoices of materials, and other relevant data and records. The CONTRACTOR will provide facilities for such access and observation of the work and also for any inspection or testing thereof.

If any work is covered contrary to the request of the ENGINEER, it must at the ENGINEER's request be uncovered for the ENGINEER's observation and replaced at the CONTRACTOR's expense.

If any work has been covered which the ENGINEER has not specifically requested to observe prior to its being covered, or if the ENGINEER considers it necessary or advisable that covered work be inspected or tested by others, the CONTRACTOR at the ENGINEER's request will uncover, expose, or otherwise make available for observation, inspection, or testing as the ENGINEER may require, that portion of the work in question, furnishing all necessary labor, materials, tools, and equipment. If it is found that such work is defective, the CONTRACTOR will bear all the expenses of such uncovering, exposure, observation, inspection, and testing and of satisfactory reconstruction. If, however, such work is not found to be defective, the work will be under Section 104, “Extra Work” or an extension of the contract time, or both, directly attributed to such uncovering, exposure, observation, inspection, testing, and reconstruction, and an appropriate work order shall be issued.

104-15 FINISHING AND CLEANUP

The CONTRACTOR shall remove all loose trash from jobsite and dispose of properly on a daily basis. From time to time or as may be ordered by the ENGINEER and immediately after completion of the work, the CONTRACTOR shall at its own expense clean up and remove all refuse, including any remaining survey stakes, and unused materials of any kind resulting from the work. Upon failure to do so within 24 hours after request by the ENGINEER, the work may be performed by the CITY and the cost thereof charged to the CONTRACTOR and deducted from the CONTRACTOR's final

estimate. All excavated areas along trails, sidewalks, curbs, and other structures shall be backfilled with earth, and the cost of such work shall be incidental to the item of construction.

104-16 WARRANTY

The CONTRACTOR shall guarantee all work and materials and guarantee the performance of the finished project free from material defect or failure for a period of two years from the date of substantial completion, and the performance bond shall remain in full force and effect for the period. The CONTRACTOR shall provide this warranty regardless of whether the cause of a failure is known or attributable to the CONTRACTOR, except for damage caused by a third party by no fault of the CONTRACTOR.

In the event the deficiency in work or performance is caused by poor workmanship by the CONTRACTOR, the CONTRACTOR's liability may, at the discretion of the ENGINEER, extend past the warranty period.

SECTION 105 – LEGAL RELATIONSHIPS AND RESPONSIBILITIES

105-1 DAMAGES

The CONTRACTOR will be held responsible and be required to make good, at the CONTRACTOR's expense, any and all damages to personal property caused by carelessness, neglect, or want of due precaution on the part of the CONTRACTOR.

105-2 UTILITIES

It shall be the responsibility of the CONTRACTOR to be familiar with the location of the existing sanitary sewer, water mains and service lines, storm sewer, oil pipelines, gas mains and service lines, telephone and communication lines, power, light and telephone poles and guys, steam lines, valve boxes and stop boxes, mail boxes, and all appurtenances pertaining to utility and public services. No additional compensation shall be made for extra work or delays due to marked or unmarked utilities, whether shown on plans or not.

The CONTRACTOR shall notify all underground facility operators at least 48 hours in advance excluding Saturdays, Sundays, or holidays, and in accordance with NDCC Chapter 49-23 of any construction and consult with personnel of said utility companies regarding any changes or conflicts.

The CONTRACTOR is responsible for repairing or replacing any lawn irrigation systems damaged by the CONTRACTOR at no cost to the CITY.

105-3 PROTECTION, MAINTENANCE, AND RESTORATION OF PROPERTY

The CONTRACTOR shall protect public and private property during all construction activities. The CONTRACTOR shall assume liability for any damage to public or private property resulting from construction activities, defective work or materials, or non-execution of the contract until project acceptance.

The CONTRACTOR shall maintain, with no additional compensation, existing or newly established grassed lawn and boulevard areas in City rights-of-way and easements within work zones. Maintenance shall include regular mowing to keep grassed areas to a maximum of six inches tall during construction and four inches tall prior to project acceptance by the ENGINEER. This shall include areas that are not disturbed during construction but where access for regular maintenance by property owner is restricted by construction activities.

The CONTRACTOR shall restore, with no additional compensation, damaged property to a condition similar or equal to preconstruction conditions. Restoration shall be done in a manner acceptable to the ENGINEER and/or property owner.

105-4 PARKING AND STORAGE OF EQUIPMENT AND MATERIALS

The CONTRACTOR shall not use private property to park or store equipment, vehicles, or materials without written permission from the property owner. The CONTRACTOR shall provide the ENGINEER with a copy of the written permission and a release from the property owner upon restoration of the property to the owner's satisfaction.

105-5 PROTECTION OF TREES

A CONTRACTOR working on public rights-of-way or properties shall be responsible for the prevention of damage to trees, shrubs, bushes, hedges, or other woody plants located within or infringing on the public rights-of-way and properties, including parks, and shall notify the City Forestry Department prior to beginning any construction near said trees.

The CONTRACTOR shall exercise care in driving or working on the root zone area of trees to prevent excessive compaction of the soil. Gaseous, liquid, or solid substances which are harmful to plantings shall not come into contact with any plantings. Nails, bolts, or other fastening materials shall not be imbedded into the trunk or limbs of a tree. Ropes, wires, or other hanging materials shall not be attached to a plant in such a manner that the bark may be damaged or cause undue stress to the plant structure. Materials or debris shall not be stored above the root zone of any tree which may impede the free passage of air, water, or nutrients, except by written permission of the City Forester.

Any overhanging branches or underlying roots which may be crushed, scarred, broken, or damaged in any way due to unavoidable construction activity shall be reported to the City Forester so that preventive action may be taken to minimize damage to plants. Any

trees damaged without prior notification of the City Forester shall be the responsibility of the CONTRACTOR to repair or replace using a licensed tree service, upon determination by the City Forester.

If it is determined by the City Forester that ditches, tunnels, trenches, or other earthmoving operations for underground utilities construction will cause damage to the health, vigor, and stability of plants, the City Forester may require that power-driven soil augers or the power push method be used wherever possible. Where this is not possible, the City Forester must be notified to assist in determining alternate methods. If trees must be pruned, fertilized, or removed prior to construction, as determined by the City Forester, all costs using prescribed methods shall be the responsibility of the CONTRACTOR. The CONTRACTOR shall become familiar with and adhere to the Forestry Department's Standard Specifications on trenching and augering around trees.

Prior to backfilling any trench or ditch, the City Forester shall be notified to inspect any repairs made to damaged roots. All exposed roots shall be pruned or trimmed using a hand pruner or hand saw. Axe cuts will not be allowed.

Upon completion of construction, the CONTRACTOR shall notify the City Forester for a final inspection of the trees whether or not any damage occurred. Any damage found to have been due to the construction activity of the CONTRACTOR shall be the remedial responsibility of the CONTRACTOR to be corrected by a licensed tree service.

105-6 CITY CONDUCTOR DAMAGE

Any cost to locate damages to CITY electrical conductors or any other components of the CITY lighting, traffic signal, or other systems will be billed to the CONTRACTOR. The CITY OF BISMARCK will bill at the current CITY rates for labor, equipment, and materials as needed. If there are any questions, contact the City of Bismarck Public Works Department at 701-355-1700.

Before any repairs are made, the damage shall be inspected by a CITY street light or traffic signal technician to determine the extent of the damage and dictate the necessary repair.

If damage causes more than two splice repairs to roadway lighting conductors in a direct-buried run between poles or to a junction box, the entire conductor run shall be replaced. Splices are not allowed on traffic signal conductors including direct-bury power supply. CONTRACTOR shall be responsible for repairs at their expense.

If damage occurs to a conductor run contained in a conduit which would require a conductor splice repair, the entire conductor run shall be replaced and the conduit must be repaired.

If damage occurs to conductors which were not located, or if due care is not exercised in exposing conductors, the entire conductor run shall be replaced.

Damaged conductors shall be replaced or repaired within 24 hours of discovery, or the CITY will cause the repairs to be made and bill the CONTRACTOR. Any underground repairs shall be made in accordance with Section 1002 “Underground Splices”.

Prior to covering up any repairs, the CITY shall be notified to inspect the repair. Once repairs are accepted, the site shall be restored.

105-7 STREET SIGN REMOVE AND REINSTALL

Any existing and permanent signs shall be removed and reinstalled by the CITY OF BISMARCK. The CONTRACTOR shall give the ENGINEER a three working day notice to schedule the removal at the time needed. The CONTRACTOR shall be assessed $300 per sign for signs removed without the ENGINEER’s approval.

SECTION 106 – PROSECUTION AND PROGRESS

106-1 TIME OF BEGINNING AND COMPLETION OF WORK

If specified, the work on the contract shall be started on a date to be specified in the Advertisement for Bids, Special Provisions, or in a written order from the Board of City Commissioners. If a start date is not specified, the CONTRACTOR may begin work at his discretion. The work on the contract shall be completed on the date specified in the Proposal. Work shall continue without interruption until the contract is completed except for weather conditions or at the discretion of the ENGINEER. The Board of City Commissioners reserves the right to determine in what order the work shall be done, and the work shall be executed in accordance with such directions.

106-2 LIMITATION OF OPERATIONS

The CONTRACTOR, unless the contract allows, shall not perform work on holidays, including Sundays, without approval from the ENGINEER a minimum of 72 hours in advance.

The CONTRACTOR perform the work in compliance with Chapter 8-10 “Noises” in the “Code of Ordinances, Bismarck, North Dakota”.

106-3 LIQUIDATED DAMAGES

The CITY and the CONTRACTOR recognize that time is of the essence of the agreement. They further recognize that not only will the CITY suffer financial loss if the work is not completed within the times specified in the contract, plus any extensions thereof allowed pursuant to the terms of the contract, but also the public of the CITY OF BISMARCK will suffer damages extremely difficult to estimate.

The parties recognize the delays, expense, and difficulties involved in proving the actual loss and damages suffered by the CITY and by the public of the CITY OF BISMARCK if any of the work is not completed on time.

The parties further recognize the CITY has made a reasonable endeavor to estimate the actual loss and damages which might be occasioned upon the CITY and the public of the CITY OF BISMARCK in the event of delay of completion of any of the work and that the CONTRACTOR was allowed input on this amount within five days prior to the bid opening.

Thus, both parties agree that the amounts of liquidated damages set forth herein to be assessed in the event of a delay in completion of any of the work are both reasonable in amount and reasonably related to the actual damages which the parties, through their reasonable endeavors, have estimated could occur upon delay in completion of any of the work.

Accordingly, instead of requiring any actual proof of damages in the event that the CONTRACTOR shall neglect, refuse, or fail to complete any work within the time specified in the contract, the CITY and the CONTRACTOR agree that, as liquidated damages for delay (and not as a penalty), the CONTRACTOR shall pay the CITY the amount required in the schedule set forth in this specification, the Project Proposal, Advertisement for Bids, or Special Provisions for each day that expires after the time specified in the contract that any of the work is not complete unless extensions are allowed pursuant to the terms of the contract.

Finally, the CITY and the CONTRACTOR specifically recognize that the recitals in this paragraph are conclusive presumptions, pursuant to Section 31-11-02 of the North Dakota Century Code. The decision of the ENGINEER for the non-completion of the work shall be binding upon both parties. Liquidated damages shall be based on the schedule below unless otherwise adjusted based on circumstances of the project as stated in the Advertisement or Special Provisions.

Liquidated damages shall be charged beginning the day after any interim, substantial, final or other completion date specified in the contract documents, or the day after any time extension granted by the ENGINEER. Liquidated damages shall be charged for each calendar day of delay until the project work items specified to be installed by the date(s) listed are complete.

Interim completion is a substantial completion date only for those work areas or scope items as defined in the project documents.

Substantial completion is defined as the improvement being operational and ready for use by the CITY OF BISMARCK. Water mains must be constructed, pressure tested, passing results achieved for both bacteriological tests, and conveying potable water; sewers must be constructed, tested, accepted, and conveying either storm water or sanitary sewer; streets must be constructed and open to traffic; and street lights and traffic signals must be installed, tested, and energized.

The CITY will not assess liquidated damages during a period when the project is in an authorized state of suspension.

Contract Amount	Damages Per Calendar Day
$0 to $250,000	$200
$250,001 to $500,000	$300
$500,001 to $1,000,000	$500
$1,000,001 to $2,500,000	$1,000
$2,500,001 to $5,000,000	$1,500
Over $5,000,000	$2,500

SECTION 107 – MEASUREMENT AND PAYMENT

107-1 QUANTITIES

The quantities shown on the Plan sheets entitled "Approximate Quantities" are estimated quantities based on information available at the time of design. It is mutually understood that these quantities may change at the time of construction due to unforeseen conditions which may be encountered during construction. The Board of City Commissioners reserves the right to designate the amount of work to be completed. Payment shall be made for the final amount of work complete and accepted at unit prices specified in the contract.

107-2 ESTIMATES AND PAYMENTS

The ENGINEER shall make a monthly approximate measurement of the work done to date and an estimate of the value of the same at the prices agreed upon in the contract. When directed by the ENGINEER, the CONTRACTOR shall measure the work completed and submit to the ENGINEER in duplicate copy form an estimate of the work completed to date and value of same at the prices agreed upon in the contract.

The ENGINEER shall retain 10 percent of the amount of each payment until 50 percent of all work in the contract documents has been completed and accepted by the ENGINEER. No further amount of retainage shall be withheld from payments after 50 percent of the contract has been completed unless the ENGINEER has on file any valid claims against the CONTRACTOR by the CITY OF BISMARCK or others. The ENGINEER may reduce the amount retained upon completion of 90 percent of all work in the contract documents and accepted by the ENGINEER. On completion and

acceptance of a part of the work on which the price is stated separately in the Contract Documents, payment in full may be made, including retained percentages less authorized deductions.

107-2.1 MATERIALS STORED. Payment for materials in storage may be added to any monthly estimate for the invoiced cost of materials to be incorporated in the work. Materials must meet the requirements of the contract, plans and specifications. The following conditions must be met for payment of materials stored:

a. The CONTRACTOR must submit the materials invoice.
b. The CONTRACTOR must furnish the CITY legal title or lien waiver (free of liens or encumbrances of any kind) to the materials stored. Lien waivers shall be submitted as follows to meet this requirement:
    i. A lien waiver from the supplier to the contractor,
    ii. A lien wavier from the subcontractor to the prime contractor (when applicable), and
    iii. A lien waiver from the prime contractor to the CITY.
c. The materials must be stored on CITY lands or rights-of-way, at the site, or as directed by the ENGINEER ~~to be eligible for payment~~. All materials not in storage as directed by the ~~CITY~~ ENGINEER shall be deducted from the materials invoice.
d. The CONTRACTOR must provide satisfactory evidence that the materials and transportation costs have been paid.
e. The CONTRACTOR must provide satisfactory evidence that the materials are insured against loss by damage to or disappearance of such materials at any time prior to use in the work.

The transfer of title and the CITY's payment for stored materials shall in no way relieve the CONTRACTOR of their responsibility for furnishing and placing such materials in accordance with the contract, plans, and specifications.

In no case will the dollar amount or quantity paid for materials stored exceed the contract price or quantity for such materials or for the contract item in which the materials are intended to be used.

No materials stored payment will be made for stored living or perishable plant materials.

The CONTRACTOR shall bear all costs associated with providing all documentation required for payment of stored materials as per this section.

No retainage will be deducted for materials stored ~~as directed by the CITY~~.

107-3 FINAL PAYMENT

After the work has been completed, the ENGINEER will prepare a final statement showing the quantities of each and every item of work performed by the CONTRACTOR. All estimates upon which previous payments have been based are partial estimates and are subject to correction in the final statement. The final

statement showing the entire quantity and value of each and every item of work performed will be submitted to the CONTRACTOR for its approval before being processed by the CITY for payment.

(a) Overpayment. If the final statement shows that the total of all partial payments made exceeds the total amount due to the CONTRACTOR, the CONTRACTOR shall promptly refund to the CITY the amount of such overpayment. If such refund is not made, the CITY shall have the right to deduct the amount thereof from any moneys due to the same CONTRACTOR under any other contract, either present or future or pursue other means of repayment.

107-4 OIL PRICE ADJUSTMENT

The CONTRACTOR may request a bituminous seal oil price adjustment agreement for a project when seal coat is to be installed the following construction season. The agreement will only be enacted with an **increase or decrease** of seal oil prices exceeding 10%.

The request, along with invoices from the current year for the bituminous seal oil shall be provided within 21 days of the executed CITY agreement for the project. Payment for these oils used during the current construction season will be based on the unit prices bid for the project. The bid prices for these oils will be **increased or decreased the amount above or below the 10% increase or decrease** based on the difference between the current construction season oil prices and those of the following construction season. The annual adjustment will be based on the invoices for these oils submitted two weeks prior to the start of construction operations that following construction season.

107-5 FUEL COST ADJUSTMENT CLAUSE W/DOT ATTACHMENT

The fuel oil adjustment clause contained herein provides for a price adjustment in the form of payment to the CONTRACTOR or a rebate to the CITY for fluctuations in the cost of motor fuel (both diesel and gasoline) consumed in the performance of applicable construction work. The price adjustment provisions are applicable only to contract items if gasoline and/or diesel are used as the primary fuel in the production of the affected items. The price adjustment provisions are also applicable to these eligible pay items when the CITY adds extra work to the contract.

The provision will remain in effect throughout the duration of the contract. Enactment of the fuel oil price adjustment clause will only be considered when the **increase or decrease** in the price of motor fuel as defined herein exceeds 10 percent.
The fuel oil adjustment clause is intended to reduce but not eliminate the cost effects of price uncertainty to the CONTRACTOR and the CITY for motor fuel used in the construction of this contract. It provides for sharing by the CITY in a portion of the CONTRACTOR's risk, which could result from unusual price fluctuations. The provision is not intended to compensate the CONTRACTOR for normal day-to-day fluctuations and seasonal changes or to serve as a guarantee of full compensation for motor fuel

price fluctuations.

Motor and burner fuels may have cost adjustments made in accordance with NDDOT Special Provision Fuel Cost Adjustment Clause dated September 8, 2006. Substitute CITY OF BISMARCK for NDDOT as it applies.

The contract unit price shall be firm for the first month of the contract period. Thereafter, the CONTRACTOR may request a price adjustment (increase or decrease) at a minimum frequency of one month. A written request for a price adjustment must be submitted to the ENGINEER and must include justification for the proposed change.

The justification should establish a base line at the time of bidding or last approved price adjustment and current pricing. For example, a copy of an invoice for burner fuel at the time of bidding would establish the base line, and an invoice at the time of the request would indicate the increase or decrease.

The CITY will respond as follows:

a. The request may be granted.
b. More justification may be requested.
c. The price paid may continue without change.

The baseline for Midwest Diesel Price shall be determined by U.S. Department of Energy weekly statistics, if it becomes a factor in justifying price increases based on material transport.

If a price adjustment is approved by the CITY, the date the adjustment will be effective along with the new unit prices will be included in the written response to the CONTRACTOR. Approval of any price adjustments renews the one month firm price period.

The CITY shall also be advised of and receive the benefit of any price decrease. The same notification and review process will apply to a decrease in cost.

SECTION 202 - EXCAVATION AND EMBANKMENT

202-1 DESCRIPTION

This item shall consist of excavating, hauling, placement, disposal, and compaction of embankment material in accordance with these specifications and in conformity with the dimensions and typical sections shown on the plans and with the lines and grades established by the ENGINEER.

"Unstable," "Unsuitable," "Suitable," and "Unsatisfactory" soil or aggregate items shall be defined as follows:

a. UNSATISFACTORY MATERIALS

Unsatisfactory materials are those materials which have been determined to be unsuitable for subgrade foundations, including all unsuitable soils, rock, shale, hardpan, loose rock, boulders, concrete chunks or slabs, debris, tree roots, stumps and any other materials deemed unsatisfactory by the ENGINEER for use in subgrades or embankments. Unsatisfactory material shall become the CONTRACTOR's responsibility to dispose of.

All suitable material taken from excavations shall be used in the formation of embankment, subgrade, and for backfilling as indicated on the plans or as directed by the ENGINEER.

When the volume of the excavation exceeds that required to construct the embankments to the grades indicated, the excess shall be used to grade the areas of ultimate development or become the CONTRACTOR's property as directed by the ENGINEER. When the volume of excavation is not sufficient for constructing the fill to the grades indicated by the ENGINEER, at locations designated on the plans or the special provisions, the additional material required shall be identified by the ENGINEER and paid as "Borrow Excavation."

b. UNSUITABLE SOILS

Unsuitable soils are those soils which in their natural state are unsuitable for subgrade foundation due to a high organic content such as vegetation, matted roots, tree roots, peat, topsoil, or muck. Soils of these types are very susceptible to consolidation due to the decaying of this organic matter. Other unsuitable soils are those which contain decomposable debris and ashes. Frozen material will not be allowed. Unsuitable material shall become the CONTRACTOR's responsibility to dispose of.

The frozen condition of any soil or material shall not constitute a basis for a change of classification. Although frozen material shall not be allowed in the trench unless otherwise indicated, it shall be recompacted after it has thawed as directed by the ENGINEER.

c. UNSTABLE SOILS

Unstable soils are those soils which in their natural or existing condition require manipulation, aeration, or wetting and recompaction to obtain the required density for a suitable subgrade foundation. This condition is usually caused by too high of a moisture content for cohesive soils and too loose and/or dry for granular soils.

In the case of cohesive soils which in their natural state the moisture content exceeds optimum moisture, they begin to behave as plastic rather than solid. Scarifying and windrowing to a depth of 6 to 18 inches and recompacting the soil in 6-inch lifts to the required density/optimum moisture relationship will usually correct this condition. The other alternative is to subcut to prescribed depth and replace the cohesive material in accordance with specifications.

In the case of granular soils that are too loose, usually subcutting to a depth of 6 to 18 inches and replacing the soil in 6-inch lifts to the required density/optimum moisture relationship will correct this condition.

In either case, these soils should not have to be replaced with more desirable soils; it is merely that in their natural state they are unstable but not unsuitable for subgrade foundation.

d. SUITABLE MATERIALS

Suitable materials are those materials which have been determined to be satisfactory for subgrade foundations, including all stable or unstable soils and any other materials deemed satisfactory by the ENGINEER, for use in subgrades or embankments.

202-2 CLASSIFICATION

All material excavated shall be defined as "Unclassified Excavation" unless, in the proposal form, prices are requested~~asked~~ and bids are taken for "Rock Excavation" and "Borrow Excavation."

"Unclassified Excavation" shall include all excavation performed under this item regardless of the material encountered.

"Rock Excavation," when provided in the proposal, shall include all solid rock in ledges, in bedded deposits, in unstratified masses, and in conglomerate deposits which are so firmly cemented they present all the characteristics of solid rock and which cannot be removed without drilling and blasting. All rock not allowed to be placed in the backfill or embankment, as directed by the ENGINEER, shall be considered “Rock Excavation.”

"Borrow Excavation" shall consist of approved material required for the construction of embankments or for other portions of the work and shall be obtained from approved

sources. Unless otherwise designated in the contract, the CONTRACTOR shall pay all costs involved.

The CONTRACTOR shall notify the ENGINEER in advance of opening any borrow areas so that the borrow material can be tested before being used. Sufficient time for testing the borrow shall be allowed.

202-3 CONSTRUCTION REQUIREMENTS

202-3.1 GENERAL. The rough excavation shall be carried to the necessary depth to obtain the specific depth of subgrade compaction shown on the plans. Likewise, on embankments the depth of subgrade compaction shall be as shown on the plans. Should the CONTRACTOR through negligence or other fault excavate below the designated lines, the excavation shall be replaced with approved materials in an approved manner and condition at the CONTRACTOR's expense.

The ENGINEER shall have complete control over the excavation, moving, placing, and disposition of all material and shall determine the suitability of material to be placed in embankments. All material determined unsuitable shall be disposed of in waste areas or as directed by the ENGINEER. Topsoil shall not be used in fills or in subgrades but shall be handled and placed as directed.

The CONTRACTOR shall inform and satisfy itself as to the character, quantity, and distribution of all materials to be excavated. No payment will be made for any excavated material which is used for purposes other than those designated. All spoil areas shall be leveled to a uniform line and section and shall present a neat appearance before project acceptance. The surface elevation of spoil pile areas shall not extend above the surface elevation of adjacent or contiguous usable areas unless approved by the ENGINEER. All spoil piles approved by the ENGINEER shall be seeded and appropriate erosion control constructed.

The roadway right of way shall be graded as per Standard Detail 200-1.

The ENGINEER shall provide centerline stakes to prepare the grading. The CONTRACTOR shall be responsible for staking all other grades necessary to complete grading as per plans or specifications.

The ENGINEER shall verify that finished grading of roadway is within 0.05 foot below to 0.15 foot above the final subgrade elevation specified, and the average grading of any 500 lineal foot section of roadway shall be within 0.1 foot above the final subgrade elevation specified. If grading does not meet tolerance, the CONTRACTOR shall be responsible for regrading to meet tolerance.

The ENGINEER shall verify that finished grading of stormwater ponds shall be within plus/minus 0.25 foot of design grade. If grading does not meet tolerance, the CONTRACTOR shall be responsible for regrading to meet tolerance.

Those areas outside of the pavement areas in which the top layer of soil material becomes compacted due to hauling or to any other activity of the CONTRACTOR, shall be scarified and disked to a depth of 4 inches, as directed, to loosen and pulverize the soil.

If it is necessary to interrupt existing surface drainage, sewers, or underdrainage, conduits, utilities, or similar underground structures, or parts thereof, the CONTRACTOR shall be responsible for and shall take all necessary precautions to protect and preserve or provide temporary services. When such facilities are encountered, the CONTRACTOR shall notify the ENGINEER, who shall arrange for their removal, if necessary. The CONTRACTOR shall assume all costs to repair all damage to such facilities or structures which may result from operations of the CONTRACTOR during the period of the contract.

The CONTRACTOR shall engage an independent soils testing laboratory approved by the ENGINEER to determine the soil proctors for each soil condition to be encountered on the project and perform the required compaction testing to be determined by the ENGINEER.

The compaction control tests for this section are based on 1 individual compaction test per 200 cubic yards of fill or 750 square yards of area, whichever is greater. The CONTRACTOR shall be responsible for all retesting of failed tests. The time, locations, depths, and frequency of compaction testing shall be at the discretion of the ENGINEER during construction. Should it become necessary to conduct an additional number of initial compaction tests, over and above the number specified, the CITY OF BISMARCK shall be responsible for all costs associated with additional testing performed by an independent soils testing laboratory. The CONTRACTOR, however, will be required to assume the cost of all retesting of failed tests, regardless of the total number required during construction. Compaction testing to determine densities may be accomplished with a nuclear density testing apparatus, the sand cone method or drive cylinder. Should disputes arise concerning test results, they will be resolved by using only the sand cone method of testing.

Written reports of all test results shall be supplied to the ENGINEER and the CONTRACTOR by the testing laboratory as soon as possible.

Compaction control tests as stated above shall be incidental to the price bid for related bid items.

202-3.2 EXCAVATION. Excavation shall be performed as indicated on the contract plans to the lines, grades, and elevation shown or as directed by the ENGINEER, and shall be made so the requirements for formation of embankments can be followed. No excavation or stripping shall be started until the ENGINEER has taken cross-sectional elevations and measurements of the existing ground surface and has staked out the proposed work. All material encountered within the limits indicated shall be removed and disposed of as directed. During the process of excavation, the grade shall be

maintained so that it will be well drained at all times. When directed, temporary drains and drainage ditches shall be installed to intercept or divert surface water which may affect the work.

If, at the time of excavation, it is not possible to place any material in its proper section of the permanent construction, it shall be stockpiled in approved areas for later use.

Rock, shale, hardpan, loose rock, boulders, or other material unsatisfactory for subgrades, streets, roads, shoulders, intermediate areas, or any areas intended for turfing shall be excavated to a minimum depth of 12 inches, or to the depth specified by the ENGINEER, below the contemplated surface of the subgrade or the designated grades. Muck, peat, topsoil, matted roots, tree roots, rock, grasses, or other yielding material, unsatisfactory for subgrade foundation, shall be removed to the depth specified to provide a satisfactory foundation. Unsatisfactory materials shall be disposed of at locations designated by the ENGINEER. All material so excavated shall be paid for at the unit price bid per cubic yard for "Unclassified Excavation" or for "Rock Excavation," as the case may be, when the classification for the last 2 items is provided in the proposal. The portion so excavated shall be refilled with suitable selected material as specified, obtained from the grading operations or borrow area, and thoroughly compacted by rolling. The necessary refilling will constitute a part of the embankment. Where rock cuts are made and refilled with selected material or where trenching out is done to provide for a course of pavement, the depths thus created shall be ditched at frequent intervals to provide drainage.

The CONTRACTOR shall make the distribution as indicated on the plans. Widening or narrowing of the section and raising or lowering of the grade to avoid haul will not be permitted. The ENGINEER reserves the right to make minor adjustments or revisions in lines or grades, if found necessary, as the work progresses due to discrepancies in the plans or to obtain satisfactory construction.

Overbreak, including slides, is that portion of any material displaced or loosened beyond the finished work as planned or authorized by the ENGINEER. The ENGINEER, whose decision shall be final, shall determine if the displacement of such material was unavoidable. All overbreak shall be removed by the CONTRACTOR and disposed of as directed; however, payment will not be made for the removal and disposal of overbreak which the ENGINEER determines as avoidable. Unavoidable overbreak will be classified as "Unclassified Excavation."

The removal of existing structures and utilities required to permit the orderly progress of work will be accomplished by local agencies unless otherwise shown on the plans. All existing foundations or footings shall be excavated by the CONTRACTOR and the material disposed of as directed. All foundations thus removed shall be backfilled with suitable material and compacted.

In cut areas, the subgrade under areas to be paved shall be compacted to the depths and to the densities and moisture as shown on the plans or as specified in the specifications, or when not otherwise shown or specified, to a minimum depth of 6

inches and to a density of not less than 90 percent of the maximum dry density with a moisture content falling within plus or minus 3 percentage points of optimum moisture as determined by the compaction control tests specified in ASTM D1557. Any unsuitable materials encountered shall be removed and paid for as specified.

No payment or measurement for payment will be made for suitable materials removed, manipulated, and replaced in order to obtain density. Any removal, manipulation, aeration, replacement, and recompaction of suitable materials necessary to obtain the required density shall be considered as incidental to the excavation and embankment operations and shall be performed by the CONTRACTOR at no additional cost to the project.

Stones or rock fragments larger than 2 inches in their greatest dimension will not be permitted in the top 6 inches of the subgrade. The finished grading operations conforming to the typical cross section shall be completed and maintained at least 400 feet (1 block) ahead of the paving operations.

In cut areas, all loose or protruding rocks on the back slopes shall be barred loose or otherwise removed to line or finished grade of slope. All cut-and-fill slopes shall be uniformly dressed to the slope, cross section, and alignment shown on the plans or as directed by the ENGINEER.

202-3.3 BORROW EXCAVATION. When provided for in the proposal, borrow excavation shall consist of excavation made from borrow areas outside the plan grading limits. Upon completion of borrow operations, the borrow area shall be finished to a neat and uniform grade acceptable to the ENGINEER.

The borrow excavation shall be handled and placed as specified in these specifications for excavation and embankment.

202-3.4 DITCH EXCAVATION. Ditch excavation shall consist of excavating for drainage ditches such as intercepting, inlet or outlet, temporary levee construction, or any other type as designated or as shown on the plans. The work shall be performed in the proper sequence with the other construction. The location of all ditches or levees shall be established on the ground. All satisfactory material shall be placed in fills; unsatisfactory material shall be placed in spoil areas as shown on the plans or removed from the project area as directed by the ENGINEER. Waste or surplus material shall be disposed of as shown on the plans or as directed by the ENGINEER. Intercepting ditches shall be constructed prior to the start of adjacent excavation operation. All necessary handwork shall be performed to secure a finish true to line, elevation, and cross section, as designated.

Ditches constructed on the project shall be maintained to the required cross section and shall be kept free from debris or obstructions until the project is accepted. Where necessary, sufficient openings shall be provided through spoil banks to permit drainage from adjacent lands. All ditches constructed shall be secured with erosion and sediment control as shown on the plans, or as directed by the ENGINEER.

Unless otherwise specified, no separate payment will be made for ditch excavation other than for the material removed which will be paid for at the unit price for "Unclassified Excavation" or "Rock Excavation," as the case may be, if the proposal includes classification of these excavated materials.

202-3.5 EMBANKMENT FOUNDATION PREPARATION. Immediately prior to the placing of the fill materials, the entire area upon which the embankment is to be placed, except where limited by rock, shall be scarified and broken by means of a disc harrow or plow or other approved equipment to a minimum depth of 6 inches or as specified by the ENGINEER. Scarifying shall be done approximately parallel to the axis of the fill. All roots, debris, large stones, or objectionable material that would cause interference with the compaction of the foundation or fill shall be removed from the area and disposed of as directed by the ENGINEER. A thin layer (approximately 3 inches) of all the fill material shall be spread over the scarified foundation and the whole area compacted as required in the specifications. Payment will be made for the material excavated for the embankment foundation at the unit price bid for “Unclassified Excavation.”

Where embankments are to be placed on natural slopes steeper than 3-to-1, horizontal benches shall be constructed as shown on the plans or as directed by the ENGINEER. Payment will be made for the material excavated on the embankment slopes at the unit price bid for “Unclassified Excavation.”

202-3.6 STRIPPING. All vegetation such as brush, heavy sods, heavy growth of grass, peat, topsoil, rubbish, tree roots and stumps, and any other unsuitable material within the area upon which embankment is to be placed shall be stripped or otherwise removed before the embankment is started, and in no case shall such objectionable material be allowed in or under the embankment. No direct payment will be made for stripping. The yardage removed and disposed of shall be paid for at the unit price bid per cubic yard for "Unclassified Excavation."

202-3.7 FORMATION OF EMBANKMENTS. Embankments shall be formed of satisfactory materials placed in successive horizontal layers of not more than 8 inches in loose depth for the full width of the cross section.

The grading operations shall be conducted and the various soil strata shall be placed to produce a soil structure as shown on the typical cross section, or as directed by the ENGINEER. All materials placed in the embankment shall be reasonably free of organic matter such as leaves, grass, tree roots, peat, and other objectionable material. Soil, granular material, shale, and any other material permitted for use in embankment shall be spread in successive layers as specified.

Earthwork operations shall be suspended at any time when satisfactory results cannot be obtained because of rain, freezing weather, or other unsatisfactory conditions of the field. The CONTRACTOR shall drag, blade, compact, or slope the embankment to provide proper surface drainage.

The material in the layers shall be of the proper moisture content before rolling to obtain the prescribed compaction. Wetting or drying of the material and manipulation when necessary to secure a uniform moisture content throughout the layer shall be required. Should the material be too wet to permit proper compaction or rolling, all work on all portions of the embankment thus affected shall be delayed until the material has dried to the required moisture content. Sprinkling shall be done with approved equipment that will sufficiently distribute the water. Sufficient equipment to furnish the required water shall be available at all times. Samples of all embankment materials for testing, both before and after placement and compaction, shall be completed as per Section 202. Based on these test results, corrections, adjustments, and modifications of methods, materials, and moisture content will be made to construct the embankment.

For all areas within the CITY right-of-way, the embankment shall be compacted to a density of not less than 90 percent of the maximum dry density with a moisture content falling within plus or minus 3 percentage points of the optimum moisture as determined by ASTM D1557 (modified proctor). On all areas outside of the pavement, curb and gutter, and sidewalk areas, no compaction will be required on the top 4 inches. On all areas outside of the right-of-way, the embankment shall be compacted to a density of not less than 85 percent of the maximum dry density with a moisture content falling within plus or minus 3 percentage points of the optimum moisture as determined by ASTM D1557. Any areas inaccessible to a roller shall be consolidated and compacted by mechanical tampers.

In the construction of embankments, starting layers shall be placed in the deepest portion of the fill. As placement progresses, layers shall be constructed approximately parallel to the finished pavement grade line.

When rock and other embankment material are excavated at approximately the same time, all rock shall be stockpiled and removed by the CONTRACTOR. Stones or fragmented rock larger than 2 inches in their greatest dimension will not be allowed in the top 6 inches of the subgrade. Rocks or boulders shall not be disposed of outside of the excavation or embankment areas, except at places and in the manner designated by the ENGINEER.

Frozen material shall not be placed in the embankment nor shall embankment be placed upon frozen material.

The CONTRACTOR shall be responsible for the stability of all embankments made under the contract and shall replace any portion which, in the opinion of the ENGINEER, has become displaced due to carelessness or negligence on the part of the CONTRACTOR. All embankments constructed shall be secured with erosion and sediment control as shown on plans.

There will be no separate measurement or payment for compacted embankment. All costs incidental to placing in layers, compacting, disking, watering, mixing, sloping, and

other necessary operations of the embankments will be included in the unit price bid for excavation, borrow, or related bid items.

When stockpiling of excavated material and later rehandling of such material is directed by the ENGINEER in order to produce the specified subgrade structure, the material shall be paid for at the unit price bid per cubic yard (cy) for "Unclassified Excavation."

202-3.8 EQUIPMENT. The CONTRACTOR may use any type of earthmoving, compaction, and watering equipment, provided the equipment is in a satisfactory condition and is of such capacity that the construction schedule can be maintained as planned by the CONTRACTOR, and as approved by the ENGINEER, in accordance with the total days or working days bid for the construction. The CONTRACTOR shall furnish, operate, and maintain such equipment as is necessary to control uniform compaction, layers, section, and smoothness of grade.

202-3.9 PREPARATION AND PROTECTION OF THE TOP OF THE SUBGRADE. On areas to be paved, the specified depth in cut areas and the top of embankment shall be compacted to the density/moisture specified. The typical section for areas to be paved shall be graded such that the roadway is graded as per Standard Plate 200-1. When completed, the surface shall be true to the lines, grades, and cross section shown on the plans, or as directed by the ENGINEER. After all drains, structures, ducts, and other underground appurtenances along the edges or under the pavement have been completed, the subgrade shall be compacted to the depth specified at not less than 90 percent of the maximum dry density with a moisture content falling within plus or a minus 3 percent at the optimum moisture as determined by ASTM D1557. Any irregularities or depressions that develop during rolling shall be corrected by loosening the material at these places and adding, removing, or replacing material until the surface is smooth and uniform. Any portion of the area which is not accessible to a roller shall be compacted in lifts not to exceed 6-inches to the required density/moisture tolerances by approved mechanical tampers. The material shall be sprinkled with water during rolling or tamping, when directed by the ENGINEER.

All soft and yielding material, and material which will not compact readily when rolled or tamped, shall be removed as directed by the ENGINEER and replaced with suitable material. After grading operations are complete, all loose stones larger than 2-inches in their greatest dimension shall be removed from the surface of all proposed graded paving areas and disposed of as directed by the ENGINEER.

At all times, the top of the subgrade shall be kept in such condition that it will drain readily and effectively. In handling materials, tools, and equipment, the CONTRACTOR shall protect the subgrade from damage by laying planks when directed and shall be reshaped and recompacted to required density and moisture tolerances. Storage or stockpiling of materials on the top of the subgrade will not be permitted. Until the subgrade has been checked and approved, no aggregate base, surface course, or pavement shall be laid thereon.

202-3.10 HAUL. No payment will be made separately or directly for haul on any part of the work. All hauling will be considered a necessary and incidental part of the work, and its cost shall be considered by the CONTRACTOR and included in the unit price bid for the pay items of work involved.

202.3.11 TOLERANCES. In those areas upon which an aggregate base or pavement base course is to be placed, the top of the subgrade shall be of such smoothness that, when tested with a 16-foot straightedge applied parallel and at right angles to the centerline, it shall not show any deviation in excess of 1/2 inch, or shall not be more than 0.05 of a foot from true grade as established by grade hubs or pins. Any deviation in excess of these amounts shall be corrected by loosening, adding, or removing materials, and reshaping and recompacting to required density.

On areas to be turfed under the project or in the future, outside the sidewalk, curb and gutter, and pavement limits, the surface shall be of such smoothness that it will not vary more that 0.10 of a foot from true grade as established by grade hubs. Any deviation in excess of this amount shall be corrected by loosening, adding or removing materials, and reshaping.

202-4 MEASUREMENT AND PAYMENT

202-4.1 UNCLASSIFIED EXCAVATION. Unclassified Excavation shall be measured by the cubic yard (CY) in its original position by the method of average end areas of materials acceptably excavated and stripped as specified. Measurements shall not include the yardage of material excavated without authorization beyond normal slope lines, or the yardage of material used for purposes other than those directed. The plans shall state an assumed shrinkage factor to be used to compute embankment volume placed using “Unclassified Excavation.”

Payment shall be made at the unit price bid per cubic yard (CY) for "Unclassified Excavation."

202-4.2 ROCK EXCAVATION. All rock found in the excavation and not allowed to be placed in the backfill or embankment shall be classified as Rock Excavation, measured by the cubic yard (CY) and disposed of by the CONTRACTOR, or as directed by the ENGINEER.

The CONTRACTOR shall place all rocks not allowed to be placed in the backfill or embankment and less than 1 cubic yard in a pile to be measured by the ENGINEER. The total volume of the stockpile shall be reduced by 25 percent to account for voids in the rock stockpile.

All rock greater than 1-cubic yard shall be individually measured by the ENGINEER.

Payment shall be made at the unit price bid per cubic yard (CY) for "Rock Excavation".

202-4.3 BORROW EXCAVATION. Borrow Excavation shall be measured by the cubic yard (CY) in its original position. Borrow Excavation in its original position shall include an assumed shrinkage factor to be used to compute embankment volume placed. Borrow Excavation in a stockpile shall not include an allowance for shrinkage. Payment shall be made at the unit price bid per cubic yard (CY) for "Borrow Excavation".

SECTION 204 – SUBGRADE PREPARATION

204-1 DESCRIPTION

This work shall consist of scarifying, shaping, compacting, and maintaining the subgrade prior to construction of an aggregate base, AC base course, or surface course and shall include excavation and/or shifting of materials resulting from rough grading, trenching, or other prior construction activities. Subgrade preparation shall include all work to the depths specified on the plans or in the special provisions. When subgrade preparation depths are not specified, the depth shall be assumed to be a minimum of 6-inches below the surface of the finished subgrade.

Prior to subgrade preparation, the ENGINEER shall verify the grading is within tolerance specified in Section 202. Work shall not begin on the subgrade preparation until the ENGINEER has approved that the grading has met the tolerances.

"Unstable," "Unsuitable," "Suitable," and "Unsatisfactory" soil or aggregate items are referred to in Section 202.

204-2 CONSTRUCTION REQUIREMENTS

204-2.1 GENERAL. When required to achieve compaction requirements, prior to placing any of the subsequent materials, the entire subgrade surface shall be scarified to a specified depth of not less than 6-inches and meet required moisture and compaction requirements. Excess suitable excavated material shall be stockpiled and reused whenever possible in the project. Stockpiled material which is reused shall be measured in its final section and paid for as “Unclassified Excavation.”

When subgrade preparation without scarification is required to achieve compaction requirements, this item will be paid for under “Subgrade Preparation (0 depth).”

Excavation of material for curb and gutter, or base course installation shall be measured by the cubic yard (CY) and paid for at the unit price bid for “Unclassified Excavation” complete and accepted by the ENGINEER.

Excavation and hauling of material from one point to another point on the roadbed or city rights-of-way to adjust the grade line and stockpiling excess material, if any, adjacent to the project shall be considered incidental to the "Subgrade Preparation" bid items.

All rocks larger than 4 inches in size and other unsuitable material shall be removed and replaced with suitable material. Any portions of the subgrade not easily accessible to machine operations, such as valley gutters, manholes, gate valves, and electrical lines shall be brought to the proper elevation and compacted by methods approved by the ENGINEER.

During the course of preparing the subgrade, and until the curb and gutter and pavement courses have been constructed, it shall be the CONTRACTOR's responsibility to protect the subgrade against, and repair any damage caused by, adverse weather, public traffic, and the CONTRACTOR's operations. The subgrade shall at all times be completed for a sufficient distance ahead of hauling and spreading base or surface material to allow adequate opportunity for inspection. No materials shall be placed on the subgrade until it has been checked and approved by the ENGINEER.

204-2.2 COMPACTION. The subgrade shall be compacted by approved compaction equipment. Approved compaction equipment shall include sheepsfoot rollers, pneumatic packers, mechanical packers, mechanical rammers, and vibratory equipment. Subgrade preparation depths specified on the plans or special provisions or the minimum 6-inches required below the surface of the finished subgrade shall be compacted to 90 percent of maximum dry density as determined by ASTM D1557 with a moisture content falling within plus or minus 3 percentage points of the optimum moisture content as determined by said testing method. The surface after compaction shall be true to line, grade, and cross section.

Before preparations begin for application of a surface treatment or for a surface course, the CONTRACTOR shall proof roll the subgrade, including the curb line, under the supervision of the ENGINEER. The proof roll shall be performed with a minimum gross weight of 44,000 lbs. on a tandem axle truck with four tires per rear axle. Any failing areas in the subgrade shall be the responsibility of the CONTRACTOR to remove and replace, at the discretion of the ENGINEER.

The CONTRACTOR shall then engage an independent soils testing laboratory, approved by the ENGINEER, to determine the soil proctors and perform the required compaction testing to be determined by the ENGINEER.

The compaction control tests for this section are based on one individual compaction test per 750 square yards of area. The CONTRACTOR shall be responsible for all retesting of failed tests and a proctor determination to represent each soil condition to be encountered on the project. The locations and depths of compaction testing shall be at the discretion of the ENGINEER during construction. Should it become necessary to require an additional number of initial compaction tests, over and above the number specified for bidding purposes, the City of Bismarck shall be responsible for all costs associated with additional testing performed by an independent testing laboratory. The CONTRACTOR, however, will be required to assume the cost of all retesting of failed tests regardless of the total number required during construction.

Compaction testing to determine densities may be accomplished with a nuclear density testing apparatus and/or the sand cone method. Should disputes arise concerning test results, they will be resolved by using the sand cone method of testing.

Written reports of all test results shall be supplied to the ENGINEER and the CONTRACTOR by the testing laboratory as soon as possible.

Compaction control tests as stated above shall be incidental to the unit price bid for related bid items.

No payment or measurement for payment will be made for suitable materials removed, manipulated, and replaced to obtain density in the specified depth of subgrade preparation. The moisture content of the subgrade materials shall fall within the range of plus or minus 4 percentage points of the optimum moisture content before any attempt is made to obtain the specified density. Any removal, manipulation, aeration, replacement, watering, and recompaction of suitable or unstable materials necessary to obtain the required density shall be considered as incidental to the subgrade preparation operation and shall be performed by the CONTRACTOR at no additional cost to the project.
If the desired compaction cannot be obtained by manipulation, wetting, or drying of the specified depth of the subgrade because the material is found to be "Unsuitable" or "Unsatisfactory," as defined in Section 202, or when the ENGINEER directs manipulation, wetting, or drying below the specified subgrade preparation depth, or when materials below the specified subgrade preparation depth must be removed because they are found to be "Unsuitable," or "Unsatisfactory," thus hampering subgrade operations, this work will be paid for in accordance with Section 104 of said construction specifications unless a "Subcut Excavation" item is included as a bid item on the proposal for the particular unit of the project.

If the instability of suitable materials below the specified subgrade preparation depth is a result of excessive moisture from rains, surface runoff, or frost action, the ENGINEER reserves the right to suspend the work to allow the materials to recover strength or to agree upon another method to use without any liability for the costs that may be claimed by the CONTRACTOR due to the suspension of work. Extension of time, however, will be granted in this case.

204-2.3 TOLERANCES. In those areas upon which an aggregate base or pavement is to be placed, the top of the subgrade shall be of such smoothness that, when tested with a 16-foot straightedge applied parallel and at right angles to the centerline, it shall not show any deviation in excess of 1/2 inch, or shall not be more than 0.05 of a foot from true grade established by grade hubs or pins.

The CONTRACTOR shall perform all surveying required to prepare the subgrade, to the tolerances specified, incidental to other bid items. The CONTRACTOR shall place a survey stake at the crown line on 50-foot intervals on all streets at the elevation approved by the ENGINEER. Additional staking may be required on sharp vertical and horizontal curves and at intersections and valley gutters as determined by the ENGINEER. The CONTRACTOR may use GPS equipment in lieu of staking for required surveying.

Staking shall not be the responsibility of the CONTRACTOR for curb and gutter construction.

204-3 MEASUREMENT AND PAYMENT

204-3.1 SUBGRADE PREPARATION. Subgrade Preparation shall be measured by the square yard (SY) and paid for at the unit price bid for "Subgrade Preparation" complete and accepted by the ENGINEER.

204-3.1A SUBGRADE PREPARATION (1 FOOT DEEP). Subgrade Preparation shall be measured by the square yard (SY) and paid for at the unit price bid for “Subgrade Preparation (1’ Deep)” complete and accepted by the ENGINEER.

204-3.1B SUBGRADE PREPARATION (1.5 FEET DEEP). Subgrade Preparation shall be measured by the square yard (SY) and paid for at the unit price bid for “Subgrade Prep (1.5’ Deep)” complete and accepted by the ENGINEER.

204-3.1C SUBGRADE PREPARATION (0 DEPTH). Subgrade Preparation shall be measured by the square yard (SY) and paid for at the unit price bid for “Subgrade Preparation (0 Depth)” complete and accepted by the ENGINEER.

SECTION 205 – EROSION AND SEDIMENT CONTROL

205-1 DESCRIPTION

The CONTRACTOR shall be responsible for installing, maintaining, replacing, and removing all of the erosion and sediment control measures existing, as shown on the plans, or as deemed necessary by the ENGINEER, to effectively control pollution of waterways and sedimentation onto adjacent properties or into any downstream drainage facilities. Installation shall be done in accordance with the North Dakota Department of Environmental Quality, North Dakota Department of Transportation or plan details.

CONTRACTOR shall have a copy of the SWPPP, copy of the general permit, and inspection records on-site or at a location immediately available at the construction site at all times.

The CONTRACTOR shall be responsible for constructing and maintaining erosion control measures to prevent silt and loose materials such as chips, millings, etc., from entering ditches, gutter or inlets. Additionally, the CONTRACTOR shall apply for a Large Site Construction Stormwater Management Permit (CSMP) with the City of Bismarck Department of Public Works. No City permit fees for the erosion control will be required of the CONTRACTOR for a City-bid project. The Large Site CSMP permit application is available on the City of Bismarck website.

The CONTRACTOR shall abide by the provisions of the CSMP. Any tracking or sedimentation shall be removed and disposed of on a regular basis, incidental to other bid items. Stockpiles shall not be left on streets overnight. All work associated with installing and maintaining the erosion control shall be considered incidental to other bid items.

Erosion and sediment control measures shall be sufficient to contain sediments within the construction limits. If any excavation or embankment material does flow onto adjacent properties or downstream, the CONTRACTOR shall immediately rectify the problem and repair any damages.

Any failure of the erosion and sedimentation control measures shall be repaired within 48 hours of the runoff event, along with any erosion damages, at the CONTRACTOR's expense. The CONTRACTOR shall be required to maintain erosion and sediment control installations until such time as the project is accepted as complete by the ENGINEER, thence notifying and forwarding the responsibility to maintain the erosion and sediment control measures over to the next CONTRACTOR, developer, or builder/owner.

The CONTRACTOR may request additional compensation for extra clean up or erosion control items, in the event two or more rainfalls occur within 48 hours which overwhelm

the normally expected and approved control features causing excessive failures and/or erosion repairs, as directed by the ENGINEER.

If directed by the ENGINEER, the CONTRACTOR shall remove and dispose of erosion control items before the end of the warranty period. Cleanup shall be according to Section 104. All removal and cleanup items shall be considered incidental to other bid items.

205-1.1 PROTECTION OF WATER RESOURCES. The CONTRACTOR shall dispose of all fuels, lubricants, and other organic or inorganic wastes at locations approved by regulatory agencies. Fueling, lubricating, and overhauling of all equipment shall be accomplished at locations, and in such a manner, that contaminants can be controlled and disposed of without polluting surface or subsurface waters.

Surface drainage from cuts and fills within the project limits, whether or not complete, and from borrow and waste disposal areas, shall be held in suitable sedimentation ponds or the site shall be graded to control erosion within acceptable limits. Temporary erosion and sediment control measures such as berms, dikes, drains, silt fences, bales, wattles, fabrics, and sedimentation basins, if required to meet the above standards, shall be provided and maintained until permanent drainage and erosion control facilities are complete and operative.

The CONTRACTOR shall be required to maintain all excavations, embankments, stockpiles, haul roads, plant sites, waste areas, borrow areas, and all other work areas to be free from dust which would cause a hazard or nuisance to others. The CONTRACTOR must have sufficient, competent equipment on the job to control dust. Dust control will be performed as the work proceeds and whenever a dust nuisance or hazard occurs, or as directed by the ENGINEER.

The CONTRACTOR shall maintain all facilities constructed for pollution control for as long as the operations creating the particular pollutant are being carried out or until the materials of concern become stabilized to the extent that pollution is no longer being created.

205-2 MATERIALS

205-2.1 SILT FENCE WITH WIRE BACKING. Silt fence fabric shall conform to AASHTO M288 silt fence specification. Filter fabric shall be composed of fibers consisting of long chain synthetic polymers composed of at least 95 percent by weight of polyolefins, polyesters, or polyomides. The fibers shall be formed into a network such that the filaments or yarns retain dimensional stability relative to each other. The filter fabric shall be free of any treatment or coating which might adversely alter its physical properties after installation. The fabric shall be free of defects or flaws that affect its physical and/or filtering properties. The fabric shall have a minimum width of 36 inches. The filter fabric shall be furnished with suitable wrapping for protection against moisture and extended ultraviolet exposure prior to placement. Backing for a

filter fabric silt fence shall consist of steel wire fence fabric. A woven wire fence shall conform to ASTM A116 Class 1 zinc coating for wire. The woven wire support fence shall be at least 32 inches high with a maximum opening size of 6 inches by 6 inches. The wire shall be a minimum of 14-gauge grade 60.

205-2.2 POSTS. Either wood or steel posts shall be used. Wood posts shall be treated (penta or green treated) and shall be a minimum of five feet long with minimum dimensions of 2 inches in diameter for round posts or 1½ by 1½ inches for rectangular posts. Steel posts shall be a minimum of 5 feet long, weigh a minimum of 1.3 pounds per square foot, and have projections to aid in fastening the wire or fabric. Steel posts shall also have a metal plate welded near the bottom such that when the post is driven to the proper depth, the plate will be below the ground level for added stability.

205-2.3 WEIGHTED FIBER ROLL. Weighted fiber roll shall be a photodegradable, extruded netting tube filled with wood curled excelsior and a weighted inner core. The roll diameter shall be 6 inches and the lengths shall be as required. The weight shall be a minimum of 8⅓ pounds per foot. An adequate number of weighted fiber rolls shall be placed around an inlet to provide complete protection.

205-2.4 EROSION CONTROL BLANKET. The erosion control blanket (ECB) shall be of organic biodegradable mulch material such as straw, wood curled excelsior, coconut fiber, or any combination of these materials. The ECB shall have a consistent thickness of mulch material evenly distributed over the entire area. The ECB materials must be secured on at least one side with netting. The netting must be of photodegradable polypropylene or other plastic material fused to the strand intersections. The ECB shall be a minimum width of 48 inches and shall be weed and pest free.

A. **Wood Excelsior Blanket.** The wood excelsior blanket shall consist of a machine-produced blanket of cured wood excelsior in which 80 percent of the fibers are six inches or longer. The wood excelsior blanket shall be smolder-resistant without using additives.

B. **Straw Blanket.** The straw blanket shall consist of a machine-produced, 100 percent weed-free, agricultural straw, certified by an accredited agency, in which 80 percent of the fibers are three inches or longer.

C. **Straw and Coconut Blanket.** The straw and coconut blanket shall consist of a machine-produced blanket of 70 percent straw and 30 percent coconut fibers by weight in which 80 percent of the fibers are three inches or longer.

EROSION CONTROL BLANKET

TYPE	ECB 1		ECB 2		ECB 3		ECB 4	
	Straw	Wood	Straw	Wood	Straw/Coconut	Wood	Coconut	Wood
Material	100% Straw	100% Excelsior Fibers	100% Straw	100% Excelsior Fibers	70% Straw and 30% Coconut Fibers	100% Excelsior Fibers	100% Coconut Fibers	100% Excelsior Fibers
Fiber Length 80% Must be Greater Than	3 inches	80%> 6 inches	3 inches	6 inches	3 inches	80%> 6 inches	3 inches	80%> 6 inches
Min Mass Per Unit Area ASTM D6475	0.5 lbs/sy	0.51 lbs/sy	0.5 lbs/sy	0.51 lbs/sy	0.5 lbs/sy	0.69 lbs/sy	0.5 lbs/sy	0.88 lbs/sy

Min Thickness ASTM D6525	0.25 inch	0.25 inch	0.25 inch	0.25 inch	0.25 inch	0.25 inch	0.25 inch	0.50 inch
Net Opening Minimum	0.5x0.5 inch	0.75x0.75 inch	0.5x0.5 inch	0.75x0.75 inch	0.5x0.5 inch	0.75x0.75 inch	0.6x0.6 inch	0.75x0.75 inch
Max Shear Stress @ 0.50 inches soil loss ASTM D6460	–	1.50 lbs/sy	1.50 lbs/sy	1.75 lbs/sy	1.75 lbs/sy	2.0 lbs/sy	2.25 lbs/sy	2.25 lbs/sy
Slope Gradient Application	≤3H:1V	<3H:1V - 2H:1V	≤2H:1V	2H:1V 1.5H:1V	≤1.5H:1V	≤1.5H:1V	≤1H:1V	≤1H:1V
Net Backing Type	Rapid Photodegradable Polypropylene		Polypropylene		Polypropylene		Black UV Stabilized Polypropylene	
Functional Longevity	≤ 3 months		≤ 12 months		12 to 24 months		> 24 months	
Min Machine Direction Tensile Strength ASTM D6818	50 lbs/ft		75 lbs/ft		100 lbs/ft		125 lbs/ft	

The information in this table has been derived from information obtained from the Erosion Control Technology Council. All values must be within (+/-)10 percent of the minimums shown on the table.

U-shaped wire staples or metal geotextile pins shall be used to anchor the blanket(s) to the ground surface. Wire staples shall be a minimum thickness of 8-gauge. Metal pins shall be at least 0.20-inch diameter steel with 1½-inch steel at the head of the pin. All anchors shall be between 6 to 18 inches long and have sufficient ground penetration to resist pulling out. Longer anchors may be required for loose soils. Heavier metal stakes may be required in rocky soils.

205-2.5 BALE DITCH CHECKS. Bale ditch checks shall be placed in ditches with slopes not exceeding six percent. Bale ditch checks shall be constructed of wheat straw, oat straw, prairie hay, or brome grass hay that is free of weeds declared noxious by the North Dakota State Board of Agriculture.

The stakes used to anchor the bales shall be made of hardwood material with the following minimum dimensions: 2 by 2 inches square (nominal) by 4 feet long.

Twine shall be used to bind the bales. The use of wire binding is prohibited because it does not biodegrade readily.

205-2.6 ROCK CHECKS. Rock checks shall be placed in ditches with slopes steeper than six percent. Rock gradations and size shall be as specified in the plans or by the ENGINEER.

205-2.7 STRAW WATTLES, 9-INCH DIAMETER. Straw wattles shall consist of rice or wheat straw fibers as filler within a containment netting. Filler shall be certified as weed-free. Fibers must have an average length greater than three inches and shall contain ultraviolet inhibiters. The strand thickness shall be no less than 0.030 inches, the knot thickness no less than 0.0555 inches, and the netting weight no less than 0.35 ounces per foot. The entire wattle unit shall be sufficiently durable to withstand weather, construction, and installation conditions for at least 3 months, including

multiple movements and reinstallations. Wattles shall have a 9-inch diameter (1-inch tolerance) and a minimum unit weight of 1.6 pounds per square foot. Wood posts of sufficient strength to withstand installation and weather shall be used for anchoring. Stakes shall be wooden, 1⅛ inches thick by 30 inches long.

205-2.8 STRAW WATTLES, 12-INCH DIAMETER. Wattles shall consist of rice or wheat straw fibers as filler within a containment netting. Filler shall be certified as weed-free. Fibers must have an average length greater than 3 inches and shall contain ultraviolet inhibiters. The strand thickness shall be no less than 0.035 inches, the knot thickness no less than 0.0555 inches, and the netting weight no less than 0.35 ounces per foot. The entire wattle unit shall be sufficiently durable to withstand weather, construction, and installation conditions for at least three months, including multiple movements and reinstallations. Wattles shall have a 12-inch diameter (1-inch tolerance) and a minimum unit weight of 3.75 pounds per linear foot. Wood posts of sufficient strength to withstand installation and weather shall be used for anchoring. Stakes shall be wooden, 1⅛ inches thick by 30 inches long.

205-2.9 DRAINAGE STRUCTURE INLET FILTER. The drainage structure inlet filter shall be Top Guard by ERTEC, or approved equivalent. The inlet filter shall include an overflow feature designed to allow full flow of water into the structure if the filter is filled with sediment. The inlet filter shall have a minimum flow rate of 130 gallons per minute per square foot.

205-2.10 CONCRETE EROSION CONTROL BLANKET. Prefabricated concrete erosion control blanket shall be Creflex 40F. The concrete erosion control blanket shall be a minimum 40 pounds per square foot with 7-ounce geotextile fabric backing. Concrete used to fabricate the erosion control blanket shall have a compressive strength of 4,000 pounds per square inch.

205-2.11 EROSION CONTROL BERM. Inlet pipes shall be 6-inch PVC and coiled pipes shall be six inch perforated pipe. Wire and/or twine shall be used to tie the pipe coils together. Wooden stakes shall be used with a minimum length of 24 inches. Riprap shall consist of rocks with diameters of 9 to 12 inches placed on a woven fabric.

205-3 CONSTRUCTION REQUIREMENTS

205-3.1 GENERAL. The CONTRACTOR shall furnish all labor, materials, and services necessary for and incidental to the completion of all work as shown on the drawings and specified herein. All machinery and equipment owned or controlled by the CONTRACTOR shall be of sufficient size to meet the requirements of the work and shall produce satisfactory work. All work shall be subject to the inspection and approval of the ENGINEER. The CONTRACTOR shall employ at all times a sufficient force of workmen of such experience and ability that the work can be completed in a satisfactory and workmanlike manner.

205-3.2 SILT FENCE WITH WIRE BACKING. The CONTRACTOR shall be responsible to furnish and install silt fence with wire backing as directed by the ENGINEER to effectively control erosion and sedimentation. The CONTRACTOR shall install and maintain silt fence with wire backing by the end of the work day upon notification by the ENGINEER. The CONTRACTOR shall construct silt fences as presented in Standard Detail 205-3. The reuse of silt fence materials without prior approval by the ENGINEER will not be allowed.

The CONTRACTOR shall check the operation and maintenance of the silt fence each week and within 24 hours following rainfall events of 1/2 inch or more until final acceptance of the contract. Rainfall shall be measured on site. The CONTRACTOR shall be responsible for all maintenance to silt fences. Sediment shall be maintained in such a way that it does not exceed one-third of the silt fence height.

The CONTRACTOR shall remove silt fences as directed by the ENGINEER. This shall include the removal of wire backing, silt fence fabric, and all stakes. All sediment accumulation shall be removed and all excavations shall be backfilled and properly compacted. The site shall be graded to blend with the terrain, and all disturbed areas shall be seeded.

205-3.3 WEIGHTED FIBER ROLL. The CONTRACTOR shall be responsible to furnish and install weighted fiber rolls as directed by the ENGINEER to effectively control erosion and sedimentation. The CONTRACTOR shall install and maintain weighted fiber rolls by the end of the work day upon notification by the ENGINEER. Approximately 3 to 6 inches shall be left between the weighted fiber rolls and the inlet. The ends shall overlap 12 inches.

When silt is one-third the height of the roll, the CONTRACTOR shall remove and dispose of the silt and debris to allow the device to function properly. The CONTRACTOR shall inspect the operation and maintenance of the weighted fiber roll each week and within 24 hours following rainfall events of 1/2 inch or more until final acceptance of the contract, incidental to the price bid for “Weighted Fiber Roll.” Rainfall shall be measured on site.

The CONTRACTOR shall remove fiber rolls as directed by the ENGINEER. Removal shall include any size of fiber roll and shall include removal of all stakes. All sediment accumulation shall be removed, and all excavations shall be backfilled and properly compacted. The site shall be graded to blend with the terrain, and all disturbed areas shall be seeded.

205-3.4 EROSION CONTROL BLANKET. The CONTRACTOR shall be responsible to furnish and install erosion control blankets as directed by the ENGINEER to effectively control erosion and sedimentation. The CONTRACTOR shall install and maintain erosion control blankets by the end of the work day upon notification by the ENGINEER.

The area to be covered shall be properly prepared and seeded before the blanket is applied. All rocks or clods over 1½ inches in diameter and all sticks and other foreign

material shall be removed. Blankets shall be rolled out in the direction of the flow. Blanket ends shall be overlapped by a minimum of one foot where additional rolls are needed. When implementing multiple blankets, upstream/upslope blankets shall overlap downstream/downslope blankets. Wire staples and metal pins shall be driven flush to the soil surface.

The CONTRACTOR shall inspect the operation and maintenance of the erosion control blankets each week and within 24 hours following rainfall events of 1/2 inch or more until final acceptance of the contract. Rainfall shall be measured on site. Blankets damaged by construction operations shall be repaired by the CONTRACTOR and at the CONTRACTOR's expense. The area shall be restored to the proper contour, seeded and fertilized, and re-covered with the same type of erosion control blanket as the one which was damaged.

The CONTRACTOR shall allow erosion control blankets to degrade naturally, unless otherwise specified in plans or by the ENGINEER.

205-3.5 BALE DITCH CHECKS. The CONTRACTOR shall furnish and install bale ditch checks as directed by the ENGINEER to effectively control erosion in channels or ditches with slopes not exceeding 6 percent. The CONTRACTOR shall install and maintain the bale ditch checks by the end of the work day upon notification by the ENGINEER.

Bale ditch checks shall be placed perpendicular to the flow line of the ditch. The ditch check shall extend far enough so that the ground level at the ends of the check is higher than the top of the lowest center bale. This prevents water from flowing around the check.

The following table provides bale ditch check spacing for given ditch grades.

Ditch Check Spacing

Ditch Grade (percent)	Check Spacing (feet)
<1.0	200
2.0	98
3.0	66
4.0	49
5.0	39
6.0	10
>6.0	Do not use bales

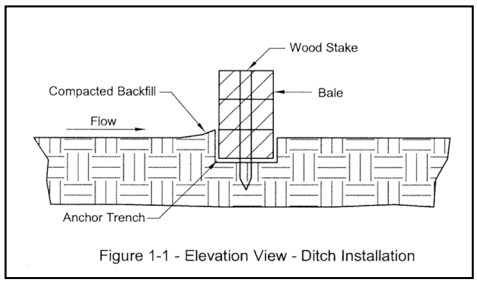


Figure 1-1 - Elevation View - Ditch Installation

Perpendicular to the ditch flow line, excavate a trench 6 inches deep and a bale's width wide. Extend the trench in a straight line along the entire length of the proposed ditch check. Place soil on the upstream side of the trench to save for later use. Place the bales in the trench, making sure they are butted tightly. Two stakes shall be driven

through each bale along the centerline of the ditch check, approximately 6 to 8 inches in from the bale ends. Stakes shall be driven a minimum of 18 inches into the ground.

Once all the bales have been installed and anchored, place the excavated soil against the upstream side of the check and compact it. The compacted soil shall be no more than 3 to 4 inches deep and shall extend upstream no more than 24 inches.

The CONTRACTOR shall inspect the operation and maintenance of bale ditch checks each week and within 24 hours following rainfall events of 1/2 inch or more until final acceptance of the contract. Rainfall shall be measured on site. The CONTRACTOR shall be responsible for all maintenance to bale ditch checks. Sediment shall be maintained in such a way that it does not exceed one-third of the bale height.

The CONTRACTOR shall remove bale ditch checks appropriately after all sediment-producing areas have been stabilized. All sediment accumulation at the barrier(s) shall be removed, and all excavations shall be backfilled and properly compacted. The site shall be graded to blend with the terrain, and all disturbed areas shall be seeded.

205-3.6 ROCK DITCH CHECKS. The CONTRACTOR shall furnish and install rock ditch checks as directed by the ENGINEER to effectively control erosion in channels or ditches with slopes steeper than 6 percent. The CONTRACTOR shall install and maintain the rock ditch checks by the end of the work day upon notification by the ENGINEER.

Rock ditch checks shall be placed perpendicular to the flow line of the ditch. The rock ditch check shall extend far enough that the ground level at the ends of the check is higher than the lowest point on the crest of the check. This prevents water from flowing around the check. The ditch check shall be 18 to 24 inches high with side slopes not steeper than 1:1.

The following table provides rock ditch check spacing for given ditch grades.

Rock Ditch Check Spacing	
Ditch Grade (Percent)	**Check Spacing (Feet)**
4.0	75
5.0	60
6.0	50
7.0	45
8.0	35
9.0	33
10.0	30

The CONTRACTOR shall inspect the operation and maintenance of rock ditch checks each week and within 24 hours following rainfall events of 1/2 inch or more until final acceptance of the contract. Rainfall shall be measured on site. The CONTRACTOR

shall be responsible for all maintenance to rock ditch checks. Sediment shall be maintained in such a way that it does not exceed one-third of the rock height.

The CONTRACTOR shall remove rock ditch checks upon stabilization of the site. All sediment accumulated at the barrier(s) shall be removed, and all excavations shall be backfilled and properly compacted. The site shall be graded to blend with the terrain. All disturbed areas shall be seeded following the removal of rock ditch checks.

205-3.7 STRAW WATTLES (9-INCH AND 12-INCH DIAMETER). The CONTRACTOR shall be responsible to furnish and install straw wattles as directed by the ENGINEER to effectively control erosion and sedimentation. The CONTRACTOR shall install and maintain straw wattles by the end of the work day upon notification by the ENGINEER.

Trenches must be dug to a depth of 3 to 5 inches. Lay the first straw wattle snug into the trench. No daylight shall be seen under the wattles. Pack soil from trenching against the wattle on the uphill side. When installing running lengths of straw wattles, the second wattle shall be installed 9 to 12 inches from the first wattle. Do not overlap the ends. Each wattle shall have a minimum of three stakes with additional stakes spaced at a minimum of one every three feet. Stakes shall not extend more than two inches above straw wattle.

The following table provides maximum downslope spacing for various slopes.

Maximum Spacing - Downslope	
9-Inch Diameter Straw Wattles	**12-Inch Diameter Straw Wattles**
1:1 slopes = 10 feet apart	1:1 slopes = 10 feet apart
2:1 slopes = 20 feet apart	2:1 slopes = 20 feet apart
3:1 slopes = 30 feet apart	3:1 slopes = 30 feet apart
4:1 slopes = 40 feet apart	4:1 slopes = 40 feet apart

Adjustments may have to be made for the soil type with approval by the ENGINEER. For soft, loamy soils: adjust the rows closer together. For hard, rocky soils: adjust the rows farther apart.

The CONTRACTOR shall inspect the operation and maintenance of straw wattles each week and within 24 hours following rainfall events of 1/2 inch or more until final acceptance of the contract. Rainfall shall be measured on site. The CONTRACTOR shall be responsible for all maintenance to straw wattles. Sediment shall be maintained in such a way that it does not exceed one-third of the straw wattle height.

The CONTRACTOR shall remove straw wattles appropriately after all sediment-producing areas have been stabilized, or as directed by the ENGINEER. Removal shall include any size of straw wattle and shall include the removal of all stakes. All sedimentation shall be removed, and all excavations shall be backfilled and properly compacted. The site shall be graded to blend with the terrain, and all disturbed areas shall be seeded.

205-3.8 DRAINAGE STRUCTURE INLET FILTER. The CONTRACTOR shall be responsible to furnish and install drainage structure inlet filters as directed by the ENGINEER to effectively control erosion and sedimentation.

The CONTRACTOR shall inspect the operation and maintenance of drainage structure inlet filters each week and within 24 hours following rainfall events of 1/2 inch or more until final acceptance of the contract. Rainfall shall be measured on site. The CONTRACTOR shall be responsible for all maintenance to drainage structure inlet filters. Maintenance and periodic cleaning of the filter shall be incidental. Periodic removal and replacement due to large rain events may be required at the Engineer's discretion and shall be considered incidental.

The inlet filter assembly shall remain in place until removal is directed by the ENGINEER and shall include the disposal of debris or silt that has accumulated in the bag.

205-3.9 CONCRETE EROSION CONTROL BLANKETS. The CONTRACTOR shall be responsible to furnish and install concrete erosion control blankets to line and grade as shown on the plans or as directed by the ENGINEER to effectively control erosion and sedimentation. Each concrete erosion control blanket panel shall be butted against adjacent panels. The inslopes shall be free of debris and dressed to a smooth, firm surface.

Concrete erosion control blankets shall be anchored as follows:

A. A six inch loop is strung around the loops of two mats that are to be anchored together. The six inch loop is strung through the eyehole of the anchor and clamped. The driven depth of the anchor shall be 36 to 42 inches deep. Drive the anchor until the top side loops are pulled down into the soil. This provides sufficient tension on the anchor, so if movement of the mats should occur, the anchor will turn and set itself.

B. Each concrete erosion control panel shall be tied together with each adjacent panel at a maximum of eight foot spacing.

The CONTRACTOR shall inspect the operation and maintenance of concrete erosion control blankets each week and within 24 hours following rainfall events of 1/2 inch or more until final acceptance of the contract. Rainfall shall be measured on site. The CONTRACTOR shall be responsible for all maintenance to concrete erosion control blankets.

The CONTRACTOR shall remove concrete erosion control blankets appropriately after all sediment-producing areas have been stabilized or as directed by the ENGINEER. All sedimentation shall be removed and all excavations shall be backfilled and properly compacted. The site shall be graded to blend with the terrain and all disturbed areas shall be seeded.

205-3.10 EROSION CONTROL BERM. The CONTRACTOR shall be responsible to furnish and install erosion control berms as directed by the ENGINEER to effectively control erosion and sedimentation at curb and road ends. The CONTRACTOR shall construct erosion control berms as per Standard Detail 205-2.

The CONTRACTOR shall inspect the operation and maintenance of erosion control berms each week and within 24 hours following rainfall events of 1/2 inch or more until final acceptance of the contract. Rainfall shall be measured on site. The CONTRACTOR shall be responsible for all maintenance to erosion control berms.

The CONTRACTOR shall remove erosion control berms appropriately after all sediment-producing areas have been stabilized or as directed by the ENGINEER. All sedimentation shall be removed and all excavations shall be backfilled and properly compacted. The site shall be graded to blend with the terrain, and all disturbed areas shall be seeded.

205-3.11 TRANSITION MAT. Transition Mat shall be Scourstop as manufactured by Hanes Geo Components or approved equivalent. Construct transition mat as noted on the plans. CONTRACTOR shall provide and install transition mat per manufacturer's recommendations. CONTRACTOR shall abut the mats and provide and install a minimum of 8 anchors per mat. Mats shall be placed a minimum of 6-inches behind the storm pipe flared end sections and to the dimensions shown on the plans.

CONTRACTOR shall provide and install erosion control blanket (ECB) beneath the transition mat and as shown on the plans. ECB shall meet the requirements of Section 205 for ECB Type 2. Torn or punctured ECB shall not be used. ECB shall be placed a minimum of 12-inches behind the storm pipe flared end sections, or as required by the transition mat manufacturer, and to the dimensions shown on the plans. All costs for providing and installing ECB shall be incidental to the Transition Mat.

205-4 MEASUREMENT AND PAYMENT

205-4.1 SILT FENCE WITH WIRE BACKING. Silt Fence With Wire Backing shall be measured by the linear foot (LF) based on one-time installations (i.e., repair and maintenance is incidental) and paid for at the unit price for "Silt Fence With Wire Backing" complete, in place, and accepted by the ENGINEER.

205-4.2 WEIGHTED FIBER ROLL. Weighted Fiber Roll shall be measured by the linear foot (LF) and paid for at the unit price for "Weighted Fiber Roll" complete, in place, and accepted by the ENGINEER

205-4.3 EROSION CONTROL BLANKET TYPE 1. Erosion Control Blanket Type 1 shall be measured by the actual surface area covered to the nearest square yard complete, in place, and accepted by the ENGINEER. No allowance will be made for overlaps and buried blankets or mats. Staples or any other material required to place the material, as specified, will not be measured for payment but shall be included in the

price for the pay item. Material that is damaged, wasted, or not properly placed will not be measured for payment.

205-4.4 EROSION CONTROL BLANKET TYPE 2. Erosion Control Blanket Type 2 shall be measured by the actual surface area covered to the nearest square yard complete, in place, and accepted by the ENGINEER. No allowance will be made for overlaps and buried blankets or mats. Staples or any other material required to place the material, as specified, will not be measured for payment but shall be included in the price for the pay item. Material that is damaged, wasted, or not properly placed will not be measured for payment.

205-4.5 EROSION CONTROL BLANKET TYPE 3. Erosion Control Blanket Type 3 shall be measured by the actual surface area covered to the nearest square yard complete, in place, and accepted by the ENGINEER. No allowance will be made for overlaps and buried blankets or mats. Staples or any other material required to place the material, as specified, will not be measured for payment but shall be included in the price for the pay item. Material that is damaged, wasted, or not properly placed will not be measured for payment.

205-4.6 EROSION CONTROL BLANKET TYPE 4. Erosion Control Blanket Type 4 shall be measured by the actual surface area covered to the nearest square yard complete, in place, and accepted by the ENGINEER. No allowance will be made for overlaps and buried blankets or mats. Staples or any other material required to place the material, as specified, will not be measured for payment but shall be included in the price for the pay item. Material that is damaged, wasted, or not properly placed will not be measured for payment.

205-4.7 BALE DITCH CHECK. Bale Ditch Checks shall be measured by the linear foot (LF), and paid for at the unit price bid for "Bale Ditch Check" complete, in place, and accepted by the ENGINEER.

205-4.8 STRAW WATTLES (9-INCH DIAMETER). Straw Wattles (9-Inch Diameter) shall be paid for by the linear foot (LF) and paid for at the unit price bid for “Straw Wattles, 9 Inch Diameter)” complete, in place, and accepted by the ENGINEER.

205-4.9 STRAW WATTLES (12-INCH DIAMETER). Straw Wattles (12-Inch Diameter) shall be paid for by the linear foot (LF) and paid for at the unit price bid for “Straw Wattles, 12 Inch Diameter)” complete, in place, and accepted by the ENGINEER.

205-4.10 FIBER ROLL REMOVAL. Fiber Roll Removal shall be measured by the linear foot (LF) and paid for at the unit price bid for “Fiber Roll Removal” complete and accepted by the ENGINEER.

205-4.11 SILT FENCE REMOVAL. Silt Fence Removal (Wired and Regular) shall be measured by the linear foot (LF) and paid for at the unit price bid for “Silt Fence Removal” complete and accepted by the ENGINEER**.**

205-4.12 DRAINAGE STRUCTURE INLET FILTER. Drainage Structure Inlet Filter shall be measured on an individual unit basis (EA) and be paid for at the unit price bid for “Drainage Struct. Inlet Filter” complete, in place, and accepted by the ENGINEER

205-4.13 CONCRETE EROSION CONTROL BLANKET. Concrete Erosion Control Blanket shall be measured by the square yard (SY) and paid for at the unit price bid for "Concrete Erosion Control Blkt." complete, in place, and accepted by the ENGINEER. Anchors and installation shall be included in bid for concrete erosion control blanket.

205-4.14 EROSION CONTROL BERM. Erosion Control Berm shall be measured by each (EA) and paid for at the unit price bid for "Erosion Control Berm" complete, in place, and accepted by the ENGINEER. This item to include all items shown on Standard Detail 205-2.

205-4.21 ~~SCOURSTOP MAT~~TRANSITION MAT. ~~Scourstop~~ Transition Mat shall be measured by each (EA) and paid for at the unit price bid for "~~Scourstop~~ Transition Mat (4’x4’)” complete, in place, and accepted by the ENGINEER. Anchors and installation shall be included in bid price.

DIVISION 500

RIGID PAVEMENT

SECTION 501 – PORTLAND CEMENT CONCRETE PAVEMENT

501-1 DESCRIPTION

This work shall consist of a pavement composed of air-entrained portland cement concrete, with or without reinforcement as specified, constructed on a prepared subgrade or aggregate base course in accordance with these specifications, and in conformity with the lines, grades, thicknesses, and typical cross sections shown on the plans or established by the ENGINEER.

501-2 MATERIALS

501-2.1 GENERAL. Prior to construction, the CONTRACTOR shall submit for approval by the ENGINEER a certified analysis of materials listed in Sections 501-2.2, 501-2.2(a), 501-2.7, 501-2.8, 501-2.9, 501-2.10, 501-2.12, and 501-2.13.

501-2.2 PORTLAND CEMENT. The portland cement used in the work shall be Type IL, Type II, or Type II A, meeting the requirements of ASTM C150 and C595.

501-2.2a FLY ASH. The CONTRACTOR shall have the option of substituting fly ash for portland cement in the concrete mixture up to a maximum of 35 percent by weight. Each source of fly ash shall be approved by the ENGINEER prior to use. Fly ash shall conform to the requirements of ASTM C311, ASTM C618, and ASTM C684, Class C fly ash, or Class F fly ash. Class C and Class F fly ash chemical and physical specifications shall be as follows:

Chemical Requirements		
	Class C	**Class F**
Silicon dioxide (SiO_2) plus aluminum oxide (Al_2O_3) plus iron oxide (Fe_2O_3), min %	50.0	66.0
Sulfur Trioxide (SO_3), max %	5.0	5.0
Moisture content, max %	3.0	3.0
Loss on ignition, max %	5.0	5.0

Physical Requirements		
	Class C	**Class F**
Fineness: Amount retained with wet sieve (No. 325 sieve), max %	34.0	34.0
Pozzolanic activity index: With Portland Cement, at 28 days, min, percent of control	75.0	75.0
Water requirement, max %	105.0	105.0
Uniformity requirements: The specific gravity and fineness of individual samples shall not vary from the average established by the 10 preceding test, or by all preceding tests if the number is less than 10, by more than: Specific gravity, max variation from average, %	5.0	5.0
Percent retained on (No. 325) wet sieve, max variation from average	5.0	5.0

Supplementary Optional Physical Requirements		
	Class C	**Class F**
Increase of drying shrinkage of mortar bars at 28 days, max %	0.03	0.03
Uniformity requirements: In addition when air-entraining concrete is specified, the quantity of air-entraining agent required to produce an air content of 18.0 Vol % of mortar shall not vary from the average established by the 10 preceding tests or by all preceding tests if less than 10, by more than, 1%	20.0	20.0
Reactivity with Cement Alkalies: Mortar expansion at 14 days, mix %	100	100

Fly ash that fails to meet the requirement of the tests shall not be used unless specified otherwise by the ENGINEER.

A complete chemical and physical analysis must be submitted to the ENGINEER for approval 14 days prior to use.

A test result of loss on ignition and amount retained on No. 325 wet sieve must accompany every 25 tons delivered, and these results must be on file at the ready-mix producer's office. Random checks and samples shall be taken to ensure testing accuracy. Any extensive error in test results could cause the material's use to be discontinued.

No fly ash will be allowed which contains oil residue or chemical pollution control contaminates.

Each source of fly ash shall be approved by the ENGINEER prior to use. If more than one source of fly ash is used on a project, each shall be stored and used separately.

The ENGINEER shall have the right to sample and test the fly ash as deemed necessary during the course of the construction season. The fly ash shall be tested in accordance with ASTM C311.

501-2.3 AGGREGATE. The CONTRACTOR shall notify the ENGINEER of the source of the coarse and fine aggregate which is proposed for use on the contract. Sufficient time shall be allowed so that sampling and testing can be completed prior to the beginning of construction. During the construction period, the CONTRACTOR shall at all times make available to the ENGINEER the sampling of aggregate. All aggregate shall meet the requirements of these specifications.

501-2.4 COARSE AGGREGATE. Except as noted herein, the coarse aggregate used shall conform to the requirements of ASTM C33, Class 4M. Coarse aggregate shall consist of gravel or broken stone composed of strong, hard, durable, uncoated pebbles or rock fragments washed clean and free from injurious amounts of shale, coal, clay lumps, soft fragments, dirt, glass, organic, or any other deleterious substances.

Coarse aggregate shall be graded from coarse to fine within the limits in the following table, when tested in conformity with ASTM C136. If the coarse aggregate size is not designated in the contract, either gradation may be used, but once adopted, no change in gradation will be made during the course of the work.

COARSE AGGREGATE SIZE		
Sieve Size	**Percent by Weight Passing**	**Percent by Weight Passing**
1½"	100	–
1"	95-100	100
3/4"	–	90-100
1/2”	25-60	–
3/8”	–	20-55
No. 4	0-10	0-10
No. 8	0-5	0-5
No. 200	0-1	0-1
Shale	1*	1*
Iron Oxide, Coal, and Soft particles	5*	5*

*Max. percent by weight of the plus No. 4 fraction.

501-2.5 FINE AGGREGATE. Except as noted herein, the fine aggregate shall conform to the requirements of ASTM C33. Fine aggregate shall be natural sand, consisting of

hard, strong, sharp, uncoated grains, free of dust, lumps, mica, shale, organic matter, or other deleterious substances.

Fine aggregate shall be graded within the limits of the following table when tested in conformity with ASTM C136.

FINE AGGREGATE SIZE			
Mortar Sand		**Concrete Sand**	
Square Mesh Sieve Size	**Percent by Weight Passing**	**Square Mesh Sieve Size**	**Percent by Weight Passing**
No. 4	100	3/8”	100
No. 8	95-100	No. 4	95-100
–	–	No. 16	45-80
–	–	No. 50	10-30
No. 100	25 (max.)	No. 100	0-10
No. 200	10 (max.)	No. 200	0-3

The quality, sampling, and testing of mortar sand for use in cement mortar shall conform to ASTM C144.

501-2.6 WATER. Water used in mixing concrete shall be clean and shall not contain deleterious amounts of acids, alkalies, or organic materials. Water shall be subject to test and approval by the ENGINEER.

501-2.7 ADMIXTURES. Substances other than cement, water, aggregates, and air-entraining agents shall be approved by the ENGINEER for use in concrete.

Unless otherwise provided in the plans or special provisions, no reduction will be made in the specified cement content of the concrete mixture by reason of using any admixtures. Admixtures containing calcium chloride must be preapproved and conform to ASTM D98. No admixture shall be used which interferes with proper control of the entrained air content of concrete. Permission to use any admixtures may be withdrawn at any time if the properties of the admixture are not uniform or if satisfactory results are not being obtained.

Should the CONTRACTOR request and obtain permission to use admixtures for its own benefit, no additional compensation will be allowed for the cost of furnishing the admixtures and incorporating them into the concrete mixture.

Should the ENGINEER direct the CONTRACTOR to use admixtures when their use is not required by these specifications or by the plans or special provisions, furnishing the admixtures and incorporating them into the concrete mixture will be paid for as extra work as provided in Section 104-13.

Air-entraining admixtures shall conform to ASTM C260.

Retardant admixtures shall conform to ASTM C494 Type B. Retardant admixture submittals shall include specifications and a dosage calculation chart of the admixture. The use of any retardant admixture shall not change the concrete discharge specification found in 501-3.6. The maximum dosage rate shall not exceed a maximum of 2 hours of additional working time. Additional retarder admixtures to achieve over 2 hours of working time shall be approved by the ENGINEER prior to use.

501-2.8 EXPANSION JOINT MATERIAL. Pre-molded bituminous fiber expansion joint material shall be used in expansion joints and shall consist of preformed strips of one continuous piece per joint which have been formed from cane or other suitable fibers of cellular nature securely bound together and uniformly impregnated with a suitable asphaltic binder. Said joint materials shall conform to ASTM D1751 (pre-molded material). Closed cell polyethylene expansion joint filler shall conform to ASTM D1056. Expansion joint filler shall be ProFlex vinyl expansion joints from Oscada Plastics, Nomaflex polypropylene joint filler from Nomaco, or approved equivalent, and shall conform to ASTM D1752. The cost for all expansion joint material shall be considered incidental.

501-2.9 JOINT SEALING MATERIAL. Joint sealing material shall conform to the following:

Type of Sealant	**ASTM**
Hot-poured	D6690 Type I, II, and IV
Cold applied elastomeric	C920
Preformed polychloroprene elastomeric	D2628 (6 celled)
Silicone sealant	D5893 Type SL or NDDOT 826.02B Type 5

The cost for joint sealing shall be considered incidental.

501-2.10 REINFORCEMENT STEEL AND DOWEL BARS. Reinforcing steel, except as otherwise specified, shall be Grade 60 or Grade 40 deformed bars epoxy coated (meeting ASTM A775) rolled from take out billet stock and shall conform to the requirements of ASTM A615. Reinforcing steel shall consist of tie bars and steel used in structural concrete slabs.

Dowel bars shall be intermediate grade plain bars epoxy coated (meeting ASTM A775) rolled from take out billet stock and shall conform to the requirements of ASTM A663 or A675.

Bar supports and spacers shall be constructed of steel and of suitable design and strength to hold reinforcement accurately in place before and during the placing of concrete. Hy-chairs shall be of welded steel construction, and all spacers, bar supports,

and chairs shall be approved by the ENGINEER. The cost for all Reinforcement Steel shall be considered incidental.

Tie wire shall be No. 16-gauge annealed wire.

501-2.11 SELECT BACKFILL. The material furnished under this item shall be "Subcut Gravel" in accordance with Section 801, class 5 gravel, blended base or crushed concrete and shall be mechanically tamped in place in layers not exceeding 6 inches in depth.

501-2.12 CURING COMPOUNDS. Curing compounds shall conform to ASTM C309, Type 2 white pigmented. Additionally, Aquapel Plus™ has been approved for use in CITY rights-of-way.

501-3 CONSTRUCTION REQUIREMENTS

501-3.1 GENERAL. The CONTRACTOR shall furnish all labor, materials, and services necessary for and incidental to the completion of all work as shown on the drawings and specified herein. All machinery and equipment owned or controlled by the CONTRACTOR shall be of sufficient size to meet the requirements of the work and shall produce satisfactory work. All work shall be subject to the inspection and approval of the ENGINEER. The CONTRACTOR shall employ at all times a sufficient force of workmen of such experience and ability that the work can be completed in a satisfactory and workmanlike manner.

The CONTRACTOR shall identify an acceptable concrete wash out area(s). Dumping concrete or concrete waste within the CITY's right-of-way or easements including the storm water system or on adjacent properties is prohibited without the written consent of the CITY or the affected property owner.

501-3.2 MATERIALS STORAGE

(a) Portland Cement. Portland cement shall be stored as specified in ASTM C150. The portland cement shall be stored in such a manner as to permit easy access for proper inspection and identification of each shipment and in a suitable weather-tight building that will protect the portland cement from becoming damp and minimize warehouse set. Storage shall be of such capacity to provide ample space for consignments of cement as may be required to carry on the work in accordance with approved progress schedules.

(b) Aggregates. Aggregates shall be stored in such a manner as to afford good drainage, prevent the intrusion of foreign matter, and preserve the gradation. Any material which has deteriorated, or which has been damaged shall not be used for concrete.

To avoid changes in consistency, the aggregates shall be obtained from a source which will ensure uniform quality and grading during any single day's operation, and they shall be delivered to the work and handled in such manner that variations in moisture content will not interfere with the steady production of concrete of uniform quality and consistency.

(c) Fly Ash. Fly ash shall be stored in weather-tight facilities to be approved by the ENGINEER.

501-3.3 ADVANCE DESIGN OF CONCRETE MIXES. Designs and tests for each concrete mix to be used under the contract shall be made using aggregates which have been approved for this work. Except as otherwise specified, mixes shall be designed in accordance with ACI 613 to attain the required strengths using the various slumps (including the maximum allowable), the various size aggregates expected to be used in the work, and the admixtures as called for by the ENGINEER. The concrete mixes shall be designed by an independent testing laboratory as required per Section 104 of these specifications or otherwise approved by the ENGINEER and shall be incidental to other items.

Standard concrete mix shall contain 550-600 pounds of portland cement per cubic yard (CY). High early strength concrete shall contain 650-700 pounds of portland cement per cubic yard (CY).

Advance tests of each of the proposed mixes shall be made in accordance with ASTM C192. A set of six standard 6-inch diameter or a set of six standard 4-inch diameter compression test cylinders shall be made for each mix design. Three cylinders per set shall be tested for compressive strength at 7 days and three cylinders per set shall be tested for compressive strength at 28 days. The high early strength concrete mix design shall have nine standard 6-inch or 4-inch diameter compression test cylinders. Three shall be tested at 3 days, three shall be tested at 7 days, and three shall be tested at 28 days. Concrete tested shall contain all required and/or proposed admixtures and in addition to the testing required by ASTM C192 shall be tested for air content by ASTM C231.

The advance mix designs and the results of tests on cylinders made from advance mix designs are required before work of concrete placing is started. Tests for aggregates as required in Section 501-2 may be made a part of these tests if suitably referenced on the reports which shall be issued at 7 and 28 days.

Two additional aggregate samples are to be submitted throughout the construction season to an independent testing laboratory and shall be in accordance with Sections 501-2

The CONTRACTOR shall pay for all advance design and testing as required per this section, including tests for aggregates and flexural strength.

The above tests shall be repeated, if necessary, due to changes in materials or unsatisfactory results. The mix design and the advance testing of aggregates specification may be waived at the request of the CONTRACTOR and with the ENGINEER's approval if a mix design approved by the CITY OF BISMARCK is being produced by an established ready mix plant with suitable records of mixes and testing. The CONTRACTOR shall submit the following items for approved historical mix designs.

1. Copy of the originally approved mix design, performed by a certified testing lab.
2. Current material data sheet for admixtures, cement, fly ash, etc.
3. Verification of aggregate source.
4. New course and fine aggregate gradation and physical properties shall be taken at the yard of dispatch, performed by a certified testing lab.
5. 5 different concrete compressive strength test, performed by a certified testing lab dated within the past year for the plant at the yard of dispatch. Reports shall include mix identification number that match approved mix designs. Compressive strength test shall include 7 day and 28 day test results, if the mix design is for high early concrete a 3 day compressive strength test results is required as well.

501-3.4 FIELD QUALITY CONTROL. The CONTRACTOR shall engage an independent testing laboratory approved by the ENGINEER to test consistency, proportioning, and strength of the concrete mixture. The CONTRACTOR shall be responsible for scheduling the testing firm. The time and location of testing shall be at the discretion of the ENGINEER. The independent testing laboratory personnel testing in the field shall be responsible for immediately notifying the CONTRACTOR and the ENGINEER in the field of failures. If any tests have not met the specifications, testing shall continue on all batches until the specific ranges have been met and the limits of the area not meeting the requirements are established.

Aggregate samples shall be taken from stockpile at the yard of dispatch.

The cost of testing, including retesting of failed tests, shall be considered incidental. All tests requested by the ENGINEER, other than frequencies specified below, shall be considered extra items.

For construction of new concrete pavements, tests shall be taken for each 120 cubic yards (CY).

For new and repaired driveways, sidewalks and valley gutters tests shall be taken every 60 cubic yards (CY), and no less than one per week or less than 60 cubic yards (CY) utilized.

The concrete test shall be taken for each 75 cubic yards of concrete pavement repair material placed, and no less than one per week

Where less than 1,000 linear feet (LF) of new curb and gutter is placed, one concrete test shall be taken for each side of the street.

Where more than 1,000 linear feet (LF) of new curb and gutter is placed, one test shall be taken for each 1,000 LF of new curb and gutter placed on each side of the street.

A set of three standard 6-inch diameter compression test cylinders shall be cast in the field in accordance with ASTM C31 and C172 for each sample taken. The cylinders cast from the given standard concrete mixtures sample shall be tested in the laboratory, one at 7 days and the remaining two at 28 days with the required minimum strength of the concrete being 4,000 pounds per square inch at 28 days.

If the testing firm chooses to use 4-inch diameter cylinders, a set of four standard four inch diameter compression test cylinders shall be cast in the field in accordance with ASTM C31 and C172 for each sample taken. The cylinders cast from the given standard concrete mixtures sample shall be tested in the laboratory, one at 7 days and the remaining three at 28 days with the required minimum strength of the concrete being 4,000 pounds per square inch at 28 days.

High early strength concrete mixtures shall be tested in the laboratory, one at 3 days, one at 7 days, and the remaining one at 28 days for 6-inch cylinders and two at 28 days for four inch cylinders.

One additional test cylinder shall be taken during cold weather construction as defined in Section 501-3.10. This cylinder shall be cured on the job site under the same conditions as the concrete it represents and tested in the laboratory after 28 days. Each sample taken or cylinders shall also be tested for slump in accordance with ASTM C143 and air content in accordance with ASTM C231.

The maximum allowable slump of the concrete mixture shall be 4 inches unless otherwise approved by the ENGINEER. The air content shall fall within the range of 5 percent to 8 percent. Concrete test specimens for flexural strength shall be made at the discretion of the ENGINEER according to ASTM C31.

Construction and public traffic shall not be allowed on newly placed concrete pavement until the concrete has attained an initial cure.

Initial cure shall be defined as reaching a minimum compressive strength of 3,000 psi or a minimum flexural strength of 450 psi.

Concrete not meeting required specifications for slump or air content during placement may be accepted or rejected at the discretion of the ENGINEER.

Written reports of all tests shall be supplied to the ENGINEER and the CONTRACTOR by the testing laboratory as soon as possible. To expedite construction, it is necessary that the CONTRACTOR and ENGINEER be furnished with the results of all tests as soon as testing is completed.

The availability of the independent testing laboratory when needed and speed of testing and reporting are to be considered the responsibility of the CONTRACTOR.

During the course of concrete construction, it may be deemed necessary by the ENGINEER to verify concrete composition and/or thickness. This will be accomplished by coring the completed and in place concrete. The CONTRACTOR shall remove and replace the samples at no extra charge. If the concrete is deficient in composition, compaction, or thickness, satisfactory correction shall be made immediately.

Should the CONTRACTOR require any of the above verification sampling, the CONTRACTOR agrees to assume all costs incurred, including the testing of the sample.

The pavement shall not be opened to traffic prior to 7 days after construction and not before flexural strengths of 450 psi or compressive strengths of 3,000 psi are attained. Pavement shall not be opened to traffic without approval by the ENGINEER.

501-3.5 PROPORTIONING MATERIALS. Concrete shall be composed of portland cement, fly ash, fine aggregate, coarse aggregate, admixtures, and water as specified. The mix shall be designed in accordance with Section 501-3.3 of these specifications.

The amount of water specified shall include the surface moisture carried by the aggregates at the time of mixing. This amount of water shall be determined by tests made by the CONTRACTOR, and the quantity of mixing water to be added to the batch shall be added to that found to be carried by the aggregates to total the rate specified. The number of tests required and the consequent changes in the amount of mixing water to be added will depend on the control exercised in the gradation and moisture contents of the aggregate.

The amount of water shall also include that liquid added to the batch in the form of admixtures.

The amounts and proportions of fine and coarse aggregates to be used in each mix shall be such as to produce a plastic, workable mix, free from harshness, which can be readily placed into the corners and angles of the forms and around reinforcement and other embedded work without undue accumulation of water laitance on the surface, and such that there will be no honeycombing in the structure.

Proportions of fine and coarse aggregates shall be such that the ratio for the coarse to the fine aggregate shall not be less than one nor more than two. On all work under these specifications, a cubic yard of concrete shall contain not less than six sacks (564 lbs.) of cement or cement and fly ash mixture.

501-3.6 BATCHING AND MIXING CONCRETE. Mixing of concrete shall be done in a rotary batch mixer of a type acceptable to the ENGINEER. The volume of the mixed material for each batch shall not exceed the manufacturer's rated capacity of the mixer.

The batch materials shall be delivered to the mixer measured accurately to the required proportions and shall be mixed continuously for not less than 1½-minutes after all materials including water are in the mixer during which time the mixer shall rotate at the speed recommended by its manufacturer. The entire batch shall be discharged before recharging the mixer. The mixer shall be cleaned as required to ensure adequate and complete mixing.

In lieu of jobsite mixing, ready mixed concrete meeting the requirements specified herein and all applicable requirements of ASTM C94 may be approved provided the quantity and rate of delivery of materials will be such as to permit unrestricted progress of the work in accordance with the placing schedule. When the air temperatures are above 90°F, the concrete shall be discharged within one hour. When air temperatures are below 90°F, the concrete shall be discharged within a maximum of one and 1½-hours. Mixing shall not be less than 60 revolutions nor more than 300 revolutions of the drum after the introduction of the mixing water to the cement and aggregates. Truck mixers shall be equipped with a means by which the number of revolutions of the drum, blades, or paddles may be readily verified.

Two copies of complete data concerning mixing and transportation methods shall be submitted to the ENGINEER for approval.

501-3.7 SUBGRADE/BASE COURSE. Subgrades or aggregate base for placing concrete shall be prepared in accordance with Section 200 "Earthwork" or Section 300 “Base Courses” and shall be damp but not wet before the concrete is placed. Hand tamping of subgrades/bases will not be permitted. Approved mechanical type shall be used.

The CONTRACTOR shall engage an independent testing laboratory as required per Section 104 of these Specifications, or otherwise approved by the ENGINEER, to perform subgrade/base compaction tests. Subgrade/base compaction tests in accordance with ASTM D1557 shall be performed and reported at the following frequencies:

a. One for each 12 lots of new sidewalks, driveways, and/or driveway widenings.

b. One for each 20 repairs of sidewalks, driveways, curb and gutters, and valley gutters.

c. One for each 400 square feet (SF) of full-depth pavement repair.

d. Two for each new construction unit where 1,000 linear feet (LF) or less of curb and gutter is constructed.

e. One for each valley gutter placed.

f. One for each 750 square yards (SY) of concrete pavement placed.

Note: (a.) and (b.) from above may only be required when in situ subgrade is found to be of questionable suitability as determined by the ENGINEER.

Should it become necessary to require an additional number of initial compaction tests, over and above the number specified, the ENGINEER will consider additional testing as extra work.

501-3.8 FORMS. After the subgrade and aggregate base course, if required, have been graded and compacted, the forms shall be set and secured in such a manner as to retain line and grade when poured and tamped with concrete. Forms shall be metal, wood or alternative material approved by the ENGINEER. The top edge of each form shall be true and straight and when set and secured shall conform to the grade of the finished pavement. All forms shall be clean and coated with oil or other approved material before the concrete is placed. Forms shall have a depth not less than the depth of the concrete to be constructed, wood forms may be of nominal board width equal to the depth of the concrete.

501-3.9 PLACING CONCRETE. The subgrade/base shall be sprinkled directly ahead of the placing of concrete. The concrete shall be placed on the moist subgrade/base and spread uniformly to the required depth with as little handling as possible and shall be mechanically vibrated to the forms or header boards to prevent voids and honeycombed surfaces. The concrete shall be consolidated so as to produce a uniformly dense concrete and so as to flush sufficient mortar to the surface to permit a proper finish without additional water added to the surface. Excessive water, laitance, or other inert material shall be floated from the surface.

501-3.10 COLD WEATHER. When the temperature remains below 40°F for more than 3 days prior to placement, or when the temperature is forecasted to fall below 40°F during the initial cure period, as defined in Section 501-3.4, following placement, special provisions shall be taken. Except as otherwise specified, mixing, placing, and protection shall be in accordance with the latest edition of the Portland Cement Association Manual entitled *Design and Control of Concrete Mixtures*. Curing shall be specified in Section 501-3.13.

Concrete poured outside of specification shall be immediately removed upon direction of the ENGINEER and replaced with new concrete at no expense to the Owners.

In order to maintain the temperatures specified, the concrete shall be entirely enclosed with concrete insulating blankets, or other methods approved by the ENGINEER. Suitable heating equipment and the necessary labor and supervision shall be furnished. Unvented heaters shall not be used. The CONTRACTOR shall be responsible for the protection of the work with no additional compensation, any extra work needed to protect concrete due to weather conditions shall be considered incidental. Cover shall remain in place through the duration of the initial cure period or until temperatures reach and are forecasted to remain above 40°F.

At the discretion of the ENGINEER, during freezing weather, temperature records shall be kept by the CONTRACTOR and furnished to the ENGINEER daily. Records shall show the temperature at four hour intervals of the: (a) outside air, (b) concrete as it is placed, (c) air in the coldest part of the enclosure near the concrete, and (d) concrete in place at such points as the ENGINEER may direct.

501-3.11 HOT WEATHER. Concrete materials shall be placed at the lowest practicable temperature except as specified in Section 501-3.10 for cold weather. When hot weather conditions exist that would seriously impair the quality and strength of the concrete, the concrete shall be placed in accordance with the latest edition of the Portland Cement Association Manual entitled *Design and Control of Concrete Mixtures*, except as otherwise specified herein.

During hot weather conditions, the temperature of the concrete immediately before it is placed in the forms shall be between 50°F and 90°F.

Shaved ice may be used in the mixing water to reduce the temperature of the concrete at the mixer, but there shall be no ice in the concrete when it is discharged from the mixer.

The CONTRACTOR shall be responsible for the protection of the work with no additional compensation, any extra work needed to protect concrete due to weather conditions shall be considered incidental.

501-3.12 SURFACE FINISH. Concrete pavement surfaces shall be floated to a true and even plane. The CONTRACTOR shall provide factory-made straightedges, 10 feet in length for use in checking forms and final finish of all pavement sections. The maximum allowable deviation from a true plane shall be 1/8 inch in 10 feet on the top and face of forms and all exposed surfaces of the finished pavement section.

New Pavements. After surface irregularities have been removed, and before the concrete attains an initial set, a seamless strip of stiff-fiber artificial grass carpet shall be dragged longitudinally along the full width of the pavement. The surface texture shall be uniformly roughened leaving corrugations in the surface that are uniform in appearance. The width of material in the drag shall be in contact with the full width of the pavement. The drag shall be operated off a string line with its leading edge attached to bridge riding on the forms or adjacent slabs. The drag shall be maintained clean and free from encrusted mortar. A drag that cannot be cleaned shall be replaced with new fabric.

Repaired Pavements. After the irregularities have been removed and before the concrete attains an initial set, a broom shall be drawn transversely across the pavement. The brooming shall be sufficient to leave significant marking in the pavement.

501-3.13 PROTECTION AND CURING. All concrete work shall be carefully protected from sun, wind, storms, and travel until thoroughly set, and the CONTRACTOR will be

held responsible and must make good at the CONTRACTOR's expense any damage from any cause until approved and accepted by the ENGINEER.

A chemical curing agent shall be used at all times, applied immediately after installation in accordance with the manufacturer's specifications and Section 501-2.

During the curing period, only equipment necessary for curing and for sawing joints will be allowed on the concrete.

Immediately after placement, protect concrete from premature drying, excessively hot or cold temperatures, and mechanical injury.

Maintain concrete with minimal moisture loss at relatively constant temperature for periods necessary for hydration of cement and hardening of concrete.

Grade site to maintain positive drainage away from new concrete.

If the ambient temperature falls below 40°F, maintain the concrete surface temperature between 40°F and 90°F for the duration of the curing period.

If high-early strength concrete is used, maintain the surface temperature between 50°F and 90°F.

Conduct heating operations to avoid sudden temperature changes in the concrete. Before removing any enclosures, decrease the concrete's surface temperature to the air temperature at a rate not to exceed 15°F per hour.

Submit a detailed temperature maintenance plan before placing concrete if the ambient temperature is expected to drop below 40°F within the curing period.

501-3.14 CONCRETE STAMPS. The CONTRACTOR shall mark in each 500 linear feet of new pavement, either by stamping or inlaying, an approved metal plate, with CONTRACTOR's name, address, and year in which the pavement was constructed. The stamped letter shall be 1 inch high and 1/4 inch deep. If a metal plate is used, the top of the plate shall be flush with the top of the pavement. CONTRACTOR's stamp must be approved by the ENGINEER prior to beginning of the construction year. CONTRACTOR shall be responsible for changing the date on the stamp each year.

501-3.15 CONCRETE DISPOSAL. The disposal area(s) for this item shall be within a 9-mile radius of the project when said area is specified on the plans, in the special provisions, or by the ENGINEER. When a disposal area is not specified, the CONTRACTOR shall be required to either provide such an area which shall be approved by the ENGINEER or haul to the City of Bismarck Solid Waste Facility and pay the required disposal fees.

501-3.16 SAWING CONCRETE. All concrete sawing designated on the plans and/or as directed by the ENGINEER, shall have a minimum depth equal to one third the thickness of the concrete.

Prior to sawing, an inspection of the adjacent slab shall be made to determine if any micro cracks exist. If any micro cracks exist, the saw cut line may be positioned so the cracked area may be removed.

501-3.17 SELECT BACKFILL. The material furnished under this item shall be "Subcut Gravel" in accordance with Section 801, class 5 gravel, blended base or crushed concrete and shall be mechanically tamped in place in layers not exceeding 6 inches in depth.

501-3.18 BACKFILL. The newly constructed concrete pavement shall be backfilled within 14 days and compacted in accordance with Section 202.

501-3.19 JOINTS. Joints in concrete pavement shall be of the design specified and shall be constructed at the spacings and locations shown. The CONTRACTOR shall be responsible to establish joint locations as approved by the ENGINEER.

Transverse Contraction Joints. The contraction joints shall consist of weakened planes created by either sawing, inserting preformed inserts, or forming grooves in the pavement surface on small areas. The location of the grooves to be formed or sawed shall be clearly and accurately marked on the plastic concrete surface by the CONTRACTOR. When specified, the contraction joints shall include a load transfer device.

Sawed contraction joints shall be cut to the required dimensions with proper equipment. Concrete saws shall be adequately powered and furnished with suitable blades to effectively cut pavement joints to required dimensions. Each blade of multiple-blade saws shall be maintained in accurate alignment to the other blades. A device shall be provided to guide the saw along the required joint alignment. Manual guidance of the saw will be permitted if specified results are obtained. A sufficient amount of sawing equipment shall be available to maintain required progress and provide prompt replacement in case of breakdown. Adequate artificial lighting shall be provided for night sawing.

The time and sequence of sawing shall be adjusted so all joints are cut before uncontrolled cracking occurs and to permit sawing without excessive raveling. Joints shall be sawed within 24 hours to prevent uncontrolled cracking. Uncontrolled cracking that occurs shall be routed, cleaned, and sealed according to Section 501-3.23, at the CONTRACTOR's expense. Immediately after sawing, the joint shall be flushed with water under sufficient pressure to remove residue left by the sawing operation. If an uncontrolled crack occurs within 3 feet of a proposed joint location before or during sawing, the joint shall be omitted and sawing of the joint discontinued. Any joint sawed within 3 feet of an uncontrolled crack shall be repaired at the CONTRACTOR's expense. When sawing is performed before removing side forms, the initial saw cut

shall extend to within 1/2 inch or less of the side forms. If the forms have been removed, the saw cut will be extended to the edges of the slab. Any curing media removed during sawing shall be immediately replaced.

Formed Contraction Joints. A formed contraction joint shall be constructed by installing an approved, preformed insert into the plastic concrete before final surface finishing. The inserts shall be vibrated into place or installed in a groove formed by a vibrating cutting bar. The inserts' top edges shall be flush with the concrete surface. Any voids, depressions, or ridges of concrete caused by installing inserts shall be filled or removed by hand-finishing methods, and the surface across the joint shall be straight edged according to Section 501-3.12. The groove formed by the inserts shall be perpendicular to the pavement surface, true to the required alignment, and continuous along the full length of the joint. Inserts, except those designed to remain, shall be removed without damage to adjacent concrete.

When specified for use with transverse contraction joints, the dowel bars shall be held in the specified position parallel to the slab surface and to the centerline within a tolerance of 1/8 inch per foot vertically and horizontally. The dowel bar assembly shall be an approved metal supporting device securely staked to the roadbed and shall hold the dowel bars at the correct spacing, alignment, and elevation. The position of these load transfer devices shall be accurately marked with steel pins, or other precise methods, to locate the transverse joint over the center of the dowels.

Dowel bars shall have a uniform coat of Tectyl 506 applied by the manufacturer, or a thin, uniform coat of multipurpose lithium grease, NLGI Grade 2, shall be used as the release agent. Multipurpose lithium grease shall be applied to the entire length of the dowel bars within 2 hours of being covered with concrete.

Transverse Construction Joints. A transverse construction joint shall be installed at the end of each day's pour and whenever the elapsed time between placement of successive batches or loads of concrete exceeds 45 minutes.

The transverse construction joint shall be formed by installing an approved dowel splicer bar basket assembly. The assembly shall hold the dowel splicer bars parallel to the centerline and slab surface. The dowel splicer bars shall be placed with a tolerance of 1/8 inch per foot vertically and horizontally. The assembly shall be staked perpendicular to the centerline and marked. The CONTRACTOR shall pave over the assembly far enough to maintain the elevation of the top of the slab. A full-depth saw cut shall be made to expose the dowel splicer bar, the excess concrete shall be disposed of, and the threaded dowel extension bar shall be installed.

After the adjacent slab is placed, the construction joint shall be sawed and sealed as specified.

Other Concrete Joints. Other concrete joints shall be formed by an approved header shaped to conform to the cross section of the slab being placed. The header shall be rigid and secure to prevent bulging or displacement while adjacent concrete is being

placed and finished. The face of the header in contact with the concrete shall be perpendicular to the pavement surface and shall be at right angles to the pavement centering. A two-piece or other approved header shall be designed to accommodate proper placement of any dowel bars or reinforcement extending across the joint and to allow removal without damage to the concrete.

The concrete adjacent to the header shall be thoroughly consolidated by an internal vibrator or other approved methods. Segregated or improperly consolidated concrete shall be removed after the pavement has been finished, and the surface adjacent to the header shall be edged to the specified radius.

Longitudinal Weakened Plane Joints. Planes of weakness for longitudinal joints shall be created by sawing grooves in the pavement surface. Grooves shall be sawed to meet dimensions shown and shall be true to the required alignment of the joint.

Longitudinal Construction Joints. The longitudinal joint between adjoining, separately constructed lanes of pavement shall be constructed as shown on the plans. Tie bars across longitudinal construction joints shall be at the locations, spacing, and depth shown. Tie bars may be bent at right angles against the form to the first lane constructed and straightened into final position before the concrete of the adjacent lane is placed. The tie bars may be inserted through small, accurately positioned holes in the side forms. Two-piece connectors may also be used, if approved by the ENGINEER.

All dowel bars, drilled in dowels, dowel bar baskets, tie bars, headers, dowel bar basket assemblies, and sawing of longitudinal and transverse joints shall be considered incidental to concrete pavements placed or repaired and accepted by the ENGINEER.

501-3.20 EXPANSION JOINTS. Expansion joints, which are specified to be sealed, shall be constructed with the top of the expansion joint material 1/2 inch to 3/4 inch lower than the adjacent concrete or form.

501-3.21 SEALING OF JOINTS. All joints specified herein or in the standard details shall be sealed within 14 days of the construction and prior to opening to public traffic.

Just before sealing, each joint shall be thoroughly cleaned of all foreign material, including membrane-curing compound. Joint faces shall be dry when sealant is applied. Material for sealant applied hot shall be stirred during heating to prevent localized overheating.

Joints shall be sealed within 1/4 inch of the surface as per Standard Detail 500-1. The joint filling shall be done without spilling material on the exposed surface of the concrete. Any excess material on the surface of the concrete shall be removed immediately and the concrete surface cleaned. The use of sand or similar material to cover the seal shall not be permitted. Joint sealing material shall not be placed when the air temperature in the shade is less than 32°F, unless approved by the ENGINEER.

501-3.22 DRILLED IN DOWEL AND TIE BARS. Dowels shall be drilled into widened, existing, or repaired concrete pavements. Transverse dowels shall be 1¼ inches by 18 inches long smooth epoxy coated or #9 by 18 inches deformed (reinforcing bar) epoxy coated.

Holes drilled for dowels shall be located at mid-depth of the slab and spaced at 12 inches on center in accordance with the standard details or as directed by the ENGINEER. Holes drilled for dowels shall use a rigid frame mounted drill rig. The holes shall be a maximum diameter of 1 3/8 inches. Transverse doweled holes shall be air blown clean to the back of the hole. For smooth dowels, inject high-viscosity epoxy (meeting AASHTO M 235 Type 4, Grade III) into the back of the hole with a pressurized caulking apparatus. Insert 1¼ inches by 18 inches smooth dowel to allow air to escape and ensure completely filled holes with bars permanently fastened to the existing concrete. Apply a small form to face of hole to keep epoxy from flowing out and remove it prior to placing concrete. Align smooth dowel bars with the pavement direction parallel to the plane of the surface. Lightly coat the end of the smooth dowels extending into the concrete with grease.

Longitudinal tie bars shall be #6 by 18 inches deformed bars (grade 40) and shall be installed at 3 feet on center. Drills shall be mounted on a rigid frame to provide proper position and alignment. The holes shall be a maximum diameter of 7/8 inch. Tie bars shall be located at mid-depth of the slab and spaced as indicated on the details, or as directed by the ENGINEER. The cost for drilled in tie bars shall be considered incidental.

501-3.23 RANDOM CRACK SEALING. Random cracks narrower than 1/2 inch in portland cement pavement and curb and gutter that are not settled or displaced shall be sealed as directed by the ENGINEER. Before sealing, each crack shall be thoroughly cleaned mechanically of all dust, dirt, concrete scale, or other foreign matter and blown out with a jet of compressed air. The crack shall be clean and dry when sealed. Random cracks shall not be sealed when the air temperature is below 40°F.

Random cracks wider than 1/2 inch shall be widened and sealed with silicon sealant according to Section 501-2. All other random cracks shall be sealed with hot pour in accordance with Section 501-2.

Random cracks shall be sealed within 1/4 inch of the surface.

501-3.24 SAW AND SEAL. Repaired working joints and random cracks on portland cement pavements and curb and gutters shall be sawed and sealed as follows:

Saw and seal any single, transverse, uncontrolled crack that penetrates the full slab length.

All uncontrolled cracking and repaired working joints shall be sawed and sealed to the following dimensions:

Sawed Joint Width, Inches	Sealant Bead Thickness, Inches	Backer Rod Diameter, inches	Minimum Sawed Joint Depth, Inches	Backer rod Placement Depth, Inches
1/2	1/4	5/8	1 1/4	1/2
5/8	5/16	3/4	1 1/2	9/16
3/4	3/8	1	1 3/4	7/8
7/8	7/16	1	1 3/4	1 1/16
1	1/2	1 1/4	2	3/4

Joints shall be sawed to the nearest 1/8 inch in width and to the nearest 1/4 inch in depth.

The joint shall be cleaned of any materials such as rocks, dirt, oil, asphalt, paint, and rust, and blown out with compressed air immediately prior to installing sealant. Backer rod, if utilized, shall be 25 percent larger than joint width and installed full width of joint repair. Sealant shall be installed from inside the joint with an approved mechanical device. Sealant shall be filled to 1/4 inch below pavement surface. Sealant shall conform to Section 501-2. Joints to be sealed by this method will be marked by the ENGINEER.

Compression joint material within 1/2 inch from surface of pavement shall be removed and sealed, which will be paid at the unit price bid for joint and crack sealing.

501-3.25 CASTING ADJUSTMENTS. Construction materials, methods, and measurements and payments shall conform to Section 1206**.**

501-3.26 WRAPPED UTILITY BOXES. Construction materials, methods, and measurements and payments shall conform to Section 1206.

501-4 MEASUREMENT AND PAYMENT

501-4.1 PORTLAND CEMENT CONCRETE PAVEMENT. Portland Cement Concrete Pavement shall be measured by the square yard (SY) as indicated and paid for at the unit price bid for “P.C.C. Pavement” complete, in place, and accepted by the ENGINEER. Pavement thickness shall be as designated in the bid item.

501-4.3 SAW AND SEAL JOINTS. Saw and Seal Joints shall be measured by the linear foot (LF) and paid for at unit the price bid for "Conc. Saw & Seal Joints" complete, in place, and accepted by the ENGINEER.

501-4.5 SAWING CONCRETE. Sawing Concrete shall be measured by the linear foot (LF) and paid for at the unit price bid for “Sawing Concrete” completed to the required depth and approved by the ENGINEER.

501-4.10 SELECT BACKFILL. Select Backfill when listed on the Proposal form shall be measured by the ton (TON) and paid for at the unit price bid for “Select Backfill” complete, in place, and accepted by the ENGINEER.

501-4.11 ADDITIONAL PORTLAND CEMENT. During the course of construction, the ENGINEER may require the use of additional portland cement in the concrete mix. When requested and used, all cement greater than six sacks (564 lbs.) per cubic yard of concrete except for full-depth repairs shall be measured by the sack (94 lbs.) and paid for at the unit price bid for “Additional Portland Cement” complete, in place, and accepted by the ENGINEER.

DIVISION 600

CONCRETE SIDEWALKS, DRIVEWAYS, AND CURB AND GUTTER

SECTION 601 – CONCRETE SIDEWALKS

601-1 DESCRIPTION

This work shall consist of the construction of air-entrained portland cement concrete sidewalks in accordance with these specifications and standard details at the locations and to the lines and grades shown on the plans or as directed by the ENGINEER. This work shall also include the removal of sidewalk or block walk, when listed on the proposal, as shown on the plans, or as directed by the ENGINEER.

The construction of concrete sidewalks in or along any street shall be executed in strict conformity with the provisions of the City Ordinances.

601-2 MATERIALS

601-2.1 GENERAL. Materials shall meet the requirements of Section 501-2 with the following additional provisions.

601-2.2 PORTLAND CEMENT. Type IL, Type 1, or Type 1A will be allowed.

601-2.3 FLY ASH. Fly ash shall conform to the requirements listed in Section 501.

601-2.4 DETECTABLE WARNING PANELS. The following detectable warning panel cast-in-place systems have been pre-approved:

- Advantage Tactile Systems - Advantage Cast Iron Premier ADA Tactile Detectable Warning Tile
- TufTile - Cast Iron ADA Detectable Warning Tile
- TufTile - 10 guage Galvanized Steel Detectable Warning panel
- ADA Arcis Tactile
- Detectable Warning Paver by Hanover Architectural Products
- Cast Iron Coated by East Jordan Iron Works
- ADA Replaceable (Wet-Set) Stainless Steel Tactile Unit by ADA Solutions, Inc
- Stainless Steel Detectable Warning Tile by Advantage Tactile Systems, Inc.
- MetalPanel by Metadome, LLC
- Access Tile by Access Products, Inc.
- Neenah Foundry Detectable Warning Plates

Detectable warning panels shall consist of a surface of truncated domes aligned in a square grid pattern in the predominant direction of travel.

Dome Size: Truncated domes in a detectable warning surface shall have a base diameter of 0.9 inches minimum to 1.4 inches maximum, a top diameter of 50 percent of the base diameter minimum to 65 percent of the base diameter maximum, and a height of 0.2 inches.

Dome Spacing: Truncated domes in a detectable warning surface shall have a center-to-center spacing of 1.6 inches minimum and 2.4 inches maximum and a base-to-base spacing of 0.65 inches minimum measured between the most adjacent domes on the square grid.

Detectable warning panels shall be safety yellow, U.S.-ANSI Z535.1-1991, unless otherwise directed by the ENGINEER.

601-2.5 STEEL REINFORCEMENT. All steel used for reinforcement in sidewalks shall be Grade 40 or higher and conform to Section 501-2 with the following addition:

Epoxy coating for steel reinforcement for sidewalks may not be required at the discretion of the ENGINEER.

Fiberglass rebar may be used in lieu of #4 steel reinforcement in the construction of sidewalks and driveways, fiberglass rebar shall be #3 or #4 Owens Corning, #3 or #4 Gatorbar or approved equal.

601-2.6 AGGREGATE BASE. Aggregate base for sidewalks shall be Class 5, blended base or recycled concrete as per Section 302.

601-2.7 MACRO-FIBER REINFORCEMENT. Macro-fiber reinforcement shall meet requirements of ASTM C1116 and shall be Type III synthetic. Reinforcing fibers shall be a minimum one and one-half inches (1.5”) in length. Any fibers utilized must be approved by the ENGINEER prior to usage. The following fibers have been pre-approved: Fibermix PolyMesh, Euclid Tuf-Strand SF & MasterFiber MAC Matrix.

601-3 CONSTRUCTION REQUIREMENTS

Construction requirements shall conform to Section 501-3 with the following additional provisions:

601-3.1 SIDEWALK CONCRETE REMOVAL. All sidewalk concrete removed shall be disposed of in accordance with Section 501-3. All sidewalk concrete removal shall be to the nearest joint, unless directed by the ENGINEER

601-3.2 JOINTS. Expansion joints shall be placed in sidewalk concrete at intervals as shown on the standard details or as directed by the ENGINEER. Expansion joints shall

be used when adjoining private concrete slabs unless otherwise approved by the ENGINEER. The expansion joint material shall have a thickness of 1/2 inch to 3/4 inch.

The sidewalk concrete shall be divided into sections by contraction joints formed by a jointing tool or sawing.

601-3.3 FORMS. Forms shall conform to Section 501-3. Forms for use on curves shall be capable of installation to within 1/2 inch of the true curve; if the radius is less than 400 feet, the forms shall be either flexible material or shaped to fit the curve.

601-3.4 AGGREGATE BASE. Unless otherwise specified, all new and replacement sidewalk installation shall have aggregate base installed, per Section 302, to the width of the sidewalk plus 0.5' (extended 0.25' on each side). The thickness shall be as per the table below:

	Concrete Thickness	Min. Base Thickness
Sidewalk	4"	2"
ADA Ramps	6"	2"
Driveways - Residential	6"	2"
Driveways - Residential*	8"	2"
Driveways - Commercial	8"	6"
Trench Drains	6"	2"
Valley Gutters	8"	6"

*Residential Driveways with load concerns as determined by engineer

601-3.5 CONCRETE STAMPS. The CONTRACTOR shall mark at the ends of the sidewalk, either by stamping or by inlaying an approved metal plate, which shall conform to Section 501-3.

601-3.6 BACKFILL. The four inch concrete shall be backfilled within 14 days of placement to a level width of at least two feet along all edges and to a height equal to the top finished grade of the sidewalk. The backfill shall be compacted in accordance with Section 202 "Excavation and Embankment."

601-3.7 SIDEWALK EXPANSION JOINT WITH REINFORCING STEEL. Expansion joints shall be doweled in accordance with standard details. Reinforcing steel shall be ½ inch by 12 inches long, smooth or #4 bar by 12 inches deformed (reinforcing bar). Reinforcing Steel shall be centered on the slab parallel to the surface of the slab at 12-inch centers. Paper tubes or speed dowels shall be used at the expansion joint. Expansion joints with reinforcing steel shall be incidental to other bid items.

601-3.8 CONCRETE QUALITY CONTROL AND SUBGRADE TESTING. Testing frequencies shall conform to Section 501-3. Payment shall be considered incidental to other bid items.

601-3.9 ADA CURB RAMPS. ADA curb ramps must be installed when installation of new sidewalks, new asphaltic or concrete pavements, and repair of existing sidewalks, curbs, valley gutters, and utility cuts are made at an intersection. For any repair done to an existing ADA curb ramp that does not have the detectable warning panels for each direction of the ramp, the CONTRACTOR shall remove the additional concrete to install a detectable warning panel.

The ADA curb ramps shall be tied to adjacent concrete pavements and curb with one foot #4 reinforcing steel spaced at one foot centers.

The curb ramp landing lengths, directions, and placements of the detectable warning panels shall be determined by the ENGINEER in the field.

Curb ramps installed prior to installation of adjacent sidewalk shall be placed such that the top of the ramp matches the low side of the future sidewalk. The top of ramp elevation shall comply with ADA standards and the City of Bismarck Standards for placement and elevations of sidewalks as per Standard Drawing 600-18.

The ADA curb ramps for new asphaltic pavements or concrete pavements shall be protected by steel fence posts until construction of adjacent sidewalks is completed. The number of fence posts and the location shall be in accordance with Standard Details or shall be determined by the ENGINEER. Cost of furnishing and installing steel fence posts shall be considered incidental to the price bid for “Detectable Warning Panel and Detectable Warning Panel CI.”

Detectable warning surface shall extend 24 inches in the direction of pedestrian traffic and the full width of the curb ramp landing.

The detectable warning surface shall be located so that the nearest edge is 6 inches minimum and 8 inches maximum from the face of the curb, or determined by the ENGINEER in the field.

The detectable warning panels shall be installed according to the manufacturer’s recommendation and in accordance with Standard Details 600-3 and 600-4.

When installing ADA ramps with two directional ramps, the distance between the ramps must be no less than three feet. If three feet cannot be maintained between the ramps, a full drop radius ramp shall be installed.

601-3.10 SIDEWALK TRENCH DRAIN. Sidewalk trench drains shall be installed to the dimensions shown on the plans and in accordance with Standard Detail 600-5. All

costs of labor, materials, and equipment to install sidewalk trench drains shall be included in the price for "Sidewalk Trench Drain."

601-3.11 RELOCATION OF LAWN IRRIGATION SYSTEM. Any concrete work requiring the relocation of any part of a lawn irrigation system shall be paid for as extra work as outlined in Section 104.

601-3.12 MACRO-FIBER REINFORCED CONCRETE. All sidewalk concrete shall contain macro-fiber reinforcement unless otherwise directed by the ENGINEER. All macro-fiber shall be in conformance with 601-2.7. Dosage of macro-fiber shall be per the manufacturer recommended dosage rates. The cost for macro-fiber reinforcement shall be considered incidental.

601-4 MEASUREMENT AND PAYMENT

MISCELLANEOUS PAY ITEMS:

Aggregate base for sidewalks shall be paid under bid item 302-4.3 "Blended Base" by the cubic yard (CY) at plan quantity as calculated from the required width and depth given in Section 601-3.4

601-4.1 4" CONCRETE. Concrete shall be measured by the square foot (SF) and paid for at the unit price bid for "4" Concrete" complete, in place, and accepted by the ENGINEER.

601-4.3 4" CONCRETE REMOVAL. Four-inch concrete removed and disposed of shall be measured by the square foot (SF) and paid for at the unit price bid for "4" Concrete Removal." All concrete removed which is less than 6 inches in thickness shall be paid for under this item.

601-4.4 DETECTABLE WARNING PANEL (CI). Measurement and payment shall be by the square foot (SF) for "Detectable Warning Panel (CI)" complete, in place, and accepted by the ENGINEER.

601-4.5 DETECTABLE WARNING PANEL. Measurement and payment shall be by the square foot (SF) for "Detectable Warning Panel" complete, in place, and accepted by the ENGINEER.

601-4.6 thru 4.9 SIDEWALK TRENCH DRAIN – (X)" INCHES WIDE. Measurement and payment shall be by the linear foot (LF) for "Sidewalk Trench Drain - Width" complete, in place, and accepted by the ENGINEER.

SECTION 602 – CONCRETE DRIVEWAYS

602-1 DESCRIPTION

This work shall consist of new installation, removal, replacement and/or widening of existing driveways. Construction shall utilize air-entrained portland cement 6-inch for residential construction or 8-inch for commercial construction in accordance with these specifications, standards and standard detail drawings at the locations and to the lines and grades shown on the plans or as directed by the ENGINEER.

The construction of concrete driveways on the public right-of-way shall be as per City Ordinances and Standard Drawing 600-19.

602-2 MATERIALS

Materials for portland cement concrete shall meet the requirements of Sections 501 and 601.

602-3 CONSTRUCTION REQUIREMENTS

Construction requirements shall conform to Section 501 with the following additional provisions:

602-3.1 CONTRACTOR'S STAMP OR NAME PLATE. The CONTRACTOR shall mark in each driveway, either by stamping or by inlaying, an approved metal plate conforming to Section 501.

602-3.2 AGGREGATE BASE. All new and replacement driveway installation shall have aggregate base installed, per Section 302, to the width of the driveway plus 0.5' (extended 0.25' on each side). The thickness shall be as per Section 601-3.4.

602-3.3 BACKFILL. Concrete shall be backfilled within 14 days of placement to a level width of at least two feet along all edges and to a height equal to the top finished grade of the driveway, including topsoil. The backfill shall be compacted in accordance with Section 202 "Excavation and Embankment."

602-3.4 CONCRETE REMOVAL. Six inch Concrete Removal shall consist of removal of concrete that is five inches or greater in thickness and less than eight inches. Eight-Inch Concrete Removal shall consist of removal of concrete eight inch or thicker. Disposal shall be in accordance with Section 501.

602-3.5 SEALING JOINT. Joint sealing shall be in accordance with Section 501, with the following exception: Joints shall be sealed prior to the substantial completion date of the project.

Joint sealing shall be incidental to other bid items.

602-3.6 TREE REMOVAL OR TREE ROOT CUTTING. Construction methods and measurement and payment shall be as per Section 201.

602-3.7 FORMS. Forms shall conform to Section 501. All driveway forms for 6-inch or 8-inch concrete shall be set at a 90-degree angle to the street or curb and gutter alignment, unless otherwise approved by the ENGINEER.

602-3.8 JOINTS. Expansion joint materials shall not be used in 6-inch or 8-inch concrete aprons unless approved by the ENGINEER. Jointing shall conform to Detail Drawings 600-7 and 600-8 or joints approved by the ENGINEER. Jointing shall be done with appropriate jointing tools or sawed. All joints sawed into driveways must be sawed with a double blade in order for joint sealant to be installed to proper width and depth.

Joints for driveways adjacent to existing concrete streets shall follow the joint pattern of the existing concrete pavement. Other joints, such as in flare cuts, shall be allowed without following the joint pattern of the existing concrete pavement.

602-3.9 REINFORCING STEEL. Reinforcing steel shall be used for six inch or eight inch driveways where the new concrete meets the sidewalk section, keyways shall not be allowed. Reinforcing steel shall be 1/2 by 12 inches long smooth or #4 by 12 inches deformed (reinforcing bar). Reinforcing steel shall be drilled in at two feet on center. When using 1/2-inch drill bit, paper tubes or speed dowels shall be used.

For driveway widening, reinforcing steel shall be drilled at one foot on center for the entire length of the existing driveway. When using 1/2-inch drill bit, paper tubes or speed dowels shall be used.

When curbing for the driveway exists, the CONTRACTOR shall drill into the existing curb at two feet on center. When using 1/2-inch drill bit, paper tubes or speed dowels shall be used.

Epoxy coating for steel reinforcement for driveways may not be required at the discretion of the ENGINEER.

Fiberglass rebar may be used in lieu of #4 steel reinforcement in the construction of sidewalks and driveways, fiberglass rebar shall as per Section 601.

602-3.10 CONCRETE QUALITY CONTROL AND SUBGRADE TESTING. Testing shall conform to Section 501. Payment shall be considered incidental to other bid items.

602-3.11 MACRO-FIBER REINFORCED CONCRETE. All driveway concrete shall contain macro-fiber reinforcement unless otherwise directed by the ENGINEER. All macro-fiber shall be in conformance with 601-2.7. Dosage of macro-fiber shall be per the manufacturer recommended dosage rates. The cost for macro-fiber reinforcement shall be considered incidental.

602-4 MEASUREMENT AND PAYMENT

MISCELLANEOUS PAY ITEMS:

Aggregate base for driveways shall be paid under bid item 302-4.3 “Blended Base” by the cubic yard (CY) at plan quantity as calculated from the required width and depth given in Section 602-3.2.

602-4.1 6” CONCRETE. Six inch Concrete shall be measured by the square foot (SF) and paid for at the unit price bid for "6” Concrete" complete, in place, and accepted by the ENGINEER.

602-4.2 8” CONCRETE. Eight inch Concrete shall be measured by the square foot (SF) and paid for at the unit price bid for "8” Concrete " complete, in place, and accepted by the ENGINEER.

602-4.2A 8” CONCRETE REMOVAL. Eight inch or thicker concrete removed and disposed of shall be measured by the square foot (SF) and paid for at the unit price bid for "8” Inch Concrete Removal."

602-4.3 6” CONCRETE REMOVAL. Six-inch concrete removed and disposed of shall be measured by the square foot (SF) and paid for at the unit price bid for "6” Inch Concrete Removal."

DIVISION 800

SEWERS

SECTION 801 − SANITARY SEWER

801-1 DESCRIPTION

This item shall consist of pipe of the types, classes, sizes, and dimensions required on the plans, and furnished and installed at the places designated on the plans and profiles or by the ENGINEER in accordance with these specifications and with the lines and grades given.

The bid price per linear foot of pipe in place shall include the cost of excavation and backfill, the cost of furnishing and installing all trench bracing, all fittings required to complete the sewer pipe as shown on the plans, and the material for and the making of all joints, including all connections to existing sewer pipe and manholes.

"Unstable," "Unsuitable," "Suitable," and "Unsatisfactory" soil or aggregate items shall be defined as stated in Section 202.

801-2 MATERIALS

801-2.1 GENERAL. The pipe shall be of the type selected by the CONTRACTOR and shall be in accordance with the following appropriate requirements unless otherwise specified.

801-2.2 CONCRETE SANITARY SEWER PIPE. Concrete sanitary sewer pipe, reinforced, shall conform to the requirements of ASTM C76.

801-2.3 POLYVINYL CHLORIDE SANITARY SEWER PIPE. Polyvinyl chloride sanitary sewer (PVC) pipe 15 inches or smaller shall conform to the requirements of ASTM D3034 for type PSM. PVC sewer pipe and fittings with a bury depth less than 18 feet shall have an SDR of 35. PVC sewer pipe with a bury depth equal to or greater than 18 feet shall have an SDR of 26. Pipe must be of the same type and have the same SDR for full run lengths between manholes. Pipe specifications must be stamped on the pipe.

Polyvinyl chloride sewer pipe 18 inches or larger shall conform to the requirements of ASTM F679-PS46. PVC sewer main line pipe and PVC sewer service pipe shall have the elastomeric gasket-type joint providing a watertight seal. A solvent cement-type joint will not be allowed. PVC wye branches shall be of the "factory assembled type."

801-2.4 RUBBER GASKET JOINT FOR CONCRETE SANITARY SEWER PIPE. Rubber-type gaskets for concrete non-pressure pipe shall conform to the requirements of ASTM C443 or ASTM C361.

801-2.5 MORTAR. Mortar for connections to manholes shall be composed of one part, by volume, of portland cement and two parts of mortar sand. The portland cement shall conform to the requirements of Section 501. The sand shall conform to the requirements of Section 501. Hydrated lime may be added to the mixture of sand and cement in an amount equal to 15 percent of the volume of cement used. The hydrated lime shall meet the requirements of ASTM C6.

801-2.6 RUBBER GASKET JOINT FOR PVC SEWER PIPE. Rubber gaskets for PVC sewer pipe joints shall be of the elastomeric type providing a watertight seal and shall conform to ASTM D3212.

801-2.7 CONCRETE. Concrete for pipe cradles and saddles shall conform to the requirements of Section 501.

801-2.8 BEDDING MATERIAL. The bedding material shall consist of granular material in accordance with the requirements for gradation shown in the following table:

Square Mesh Sieve Size	Percent by Weight Passing
2”	100%
1”	90-100%
3/4”	80-100%
No. 4	30-90%
No. 30	10-60%
No. 100	0-15%

One gradation test shall be made for each source and change in material provided for each 500 tons of screened and/or blended material and for each 200 tons of non-screened or “bank run” material. Gradation testing shall be incidental to the pipe or other bid items.

The CONTRACTOR may provide a controlled density fill in lieu of the bedding material bed for underground pipe if approved by the ENGINEER prior to installation. The controlled density fill shall conform to Section 503.

If the controlled density fill is placed in the trench in a plastic state, the remaining backfill may not be completed for 48 hours. One compression test shall be made for each 60 cubic yards of control density fill or a minimum of one per day. A testing firm normally engaged in materials testing shall make the test at the expense of the CONTRACTOR. The CONTRACTOR shall remove and replace any material not meeting the requirements at CONTRACTOR's expense. All controlled density fill shall be designed

for easy removal should it become necessary to repair or remove the pipe in the future. The pipe shall be protected from floating to maintain line and grade.

Controlled density fill shall be paid as bedding material unless otherwise specified. Controlled density fill utilized on the remainder of the trench may be provided incidental if approved by the ENGINEER.

Bedding quantities are based on trench width in Section 801 “Excavation and Preparation of Trench.” Any additional bedding material due to a wider ditch shall be the responsibility of the CONTRACTOR.

801-2.9 SUBCUT GRAVEL. The subcut gravel shall consist of granular material in accordance with the requirements of gradation shown in the following table:

Square Mesh	Percent by Weight Passing
2”	100%
No. 4	0-10%

801-2.10 MARKING TAPE. The CONTRACTOR will be required to furnish and install marking tape located two feet above the top of all sanitary sewer mains installed under the contract. The tape shall be of the non-detectable type and shall have a minimum width of five inches. The tape shall be green in color with the words "CAUTION SEWER LINE BELOW" imprinted on the tape in black capital letters. The marking tape shall be equal to that manufactured by Presco standard grade.

Cost of marking tape and installation shall be considered incidental to other items.

801-2.11 SANITARY SEWER FORCE MAIN MATERIALS. All materials for construction of sanitary sewer force mains shall conform to Section 901 “Water Mains.”

801-2.12 SANITARY SEWER WYE BRANCHES. Wye branches shall be of the same material as the main line sanitary sewer pipe.

801-2.13 INSULATION BOARDS. Insulation boards shall conform to Section 901.

801-3 CONSTRUCTION REQUIREMENTS

801-3.1 EQUIPMENT. All equipment necessary and required for the proper construction of sanitary sewers shall be on the project in first-class working condition and approved by the ENGINEER before construction is permitted to start.

The CONTRACTOR shall handle the pipe while unloading and placing it in its final position without damage to the pipe.

The CONTRACTOR shall provide methods and means to obtain the required compaction of the pipe bed and the backfill as specified.

The CONTRACTOR shall provide a sufficient number of watertight sewer plugs to prevent infiltration of water and any other foreign material from entering the existing sewer system and the newly constructed sewer lines.

801-3.2 EXCAVATION AND PREPARATION OF TRENCH. The trench shall be dug to the alignment and depth required and only so far in advance of pipe laying as the ENGINEER may permit. The discharge from pumps shall be led to natural drainage channels, drains, or storm sewer as per erosion control and storm water pollution control standards.

The trench width may vary depending upon the depth of the trench and the nature of the excavated material but shall be of ample width to permit the pipe to be laid and joined properly and the backfill to be placed and compacted to the required density. The maximum width of trench for calculating bedding material quantities for pipe sizes 15 inches and larger shall not be more than 36 inches greater than the outside diameter of the pipe barrel. For pipe sizes under 15 inches, the maximum width shall be no more than 48 inches.

The trench shall be excavated below the required grade so that the pipe may be laid on four inches of bedding material. See Standard Detail 801-2.

Where the bottom of the trench uncovered at subgrade is unsuitable and, in the opinion of the ENGINEER, cannot support the pipe, further depth and/or width shall be excavated and refilled to the pipe foundation grade with subcut gravel thoroughly compacted. In this instance, subcut gravel shall be considered a pay item.

If other approved means shall be adopted to assure a firm foundation for the pipe, the CONTRACTOR will be allowed extra compensation. Extra compensation shall not be allowed for extra excavation and gravel used for seepage and ground water control.

If ordered in writing by the ENGINEER, the CONTRACTOR will be paid for any sheathing that the ENGINEER orders left in the trench in order to protect existing utilities or other items. The price to be paid for such sheathing material will be the current invoice price of said materials or such lesser price as the CONTRACTOR and the ENGINEER may agree that the material is worth at the time it is left in the trench.

All broken pavement or sidewalks shall be removed from the site of the work and deposited at a place approved by the ENGINEER. It shall be the responsibility of the CONTRACTOR to remove and replace at its own expense all sidewalk and curb and gutter necessary for the installation of the pipe and manholes as shown on the plans and as directed by the ENGINEER, unless items are noted on plans. The removal shall be complete to the nearest joint. No additional compensation will be allowed for this work and shall be included in the price bid for pipe or manhole installation. Replacement of sidewalk and curb and gutter shall be as per Section 600.

Bell holes of ample dimension shall be dug in earth trenches at each joint to permit the joints to be made properly.

All excavated material shall be piled in a manner that will not endanger the work and that will avoid obstructing sidewalks and driveways. Gutters shall be kept clean or otherwise satisfactory provisions made for street drainage.

The use of trench digging machinery will be permitted except in places where operation of same will cause damage to trees, buildings, or existing structures above or below ground, in which case hand methods shall be employed.

The CONTRACTOR is assumed to be familiar with all federal, state, and local laws, codes, ordinances, and regulations which in any manner affect those engaged or employed in the work, the material, or equipment used in or upon the site, or in any way affect the conduct of the work. No pleas of misunderstanding or ignorance on the part of the CONTRACTOR will, in any way, serve to modify the provisions of the contract. The CONTRACTOR shall provide and maintain on a 24-hour basis all necessary safeguards such as watchmen, traffic control devices, and night lights at CONTRACTOR's expense in accordance with Section 103-5.

Excavation for pipe laying operations shall be conducted in a manner so as to cause the least interruption to traffic. Where traffic must cross open trenches, the CONTRACTOR shall provide suitable bridges at street intersections and driveways. Hydrants under pressure, valve boxes, curb stop boxes, and other utility controls shall be left unobstructed and accessible during the construction period.

Adequate provisions shall be made for the flow of sewers, drains, and water courses encountered during construction, and the structures which may have been disturbed shall be satisfactorily restored upon completion of the work.

Prior to making any connections to the existing sanitary sewer system, the CONTRACTOR shall furnish and install watertight plugs in such a manner as to prevent infiltration and foreign material from entering the existing sewer system. The plugs shall be installed so as to not disrupt existing sewage flow and shall remain in place until the construction has been accepted by the ENGINEER.

Trees, fences, poles, and all other property shall be protected unless their removal is authorized by the ENGINEER, and any property damages shall be satisfactorily restored by the CONTRACTOR. The cost of removal shall be included in the price bid per linear foot of sewer pipe in place unless listed separately in the proposal. Tree removal and root cutting shall be in conformance with Section 201.

801-3.3 ROCK EXCAVATION. All rock found in the trench area shall be classified as solid rock and measured for payment if each individual rock, boulder, or continuous slab of ledge rock is 1 cubic foot or more in content. Solid rock shall be measured for payment on the basis of and limited to the maximum trench width allowed under Section

801 "Excavation and Preparation of Trench." If solid rock extends below the necessary depth of excavation, it shall be measured for payment to a horizontal plane 6 inches below the bottom of the pipe. All rocks smaller in volume than 1 cubic foot shall not be classified as solid rock, but may be used in backfilling as directed by the ENGINEER.

Whenever ledge rock is encountered, the CONTRACTOR shall strip all overlying earth and he shall then notify the ENGINEER that the rock is ready for measurement. The ENGINEER may then take levels upon the rock or he may at his discretion defer measurement until after the excavation is completed. CONTRACTOR shall not refill any trench where rock is encountered until notified by the ENGINEER that measurement has been made. Payment will not be allowed for any rock unless measurement has been made as herein provided. The rock shall be excavated to a depth of six inches below the bottom of the pipe, and the trench shall be refilled to the proper grade with bedding material.

All rock found in the trench having greater volume of 1 cubic foot shall not be used as backfill but shall be disposed of as directed by the ENGINEER.

801-3.4 PIPE LAYING. All watermain and sanitary sewer crossings shall conform to the following policy:

a. Where both water and sewer are of new construction:
   1. No additional protections needed if water main crosses at least 5 feet above the sewer.
   2. If the water main crosses within 3 to 5 feet above the sewer, a full length of water main shall be centered over the sewer.
   3. If the water main crosses within 3 feet above the sewer, a full length of water main shall be centered over the sewer, and the sewer joints located within 10 feet of the crossing shall be able to withstand 25 psi internal pressure.

b. Where water main crosses over an existing sewer:
   1. No additional protection needed if water main is at least 3 feet above the sewer. The intervening dirt must be left undisturbed.
   2. If crossing is within 3 feet above sewer, a full length of water main must be centered over the sewer main.

c. Where water main crosses under the sewer:
   1. In all cases, additional protection shall be provided by centering a full length of water main under the sewer main. All sewer joints located within 10 feet of the crossing shall be able to withstand 25 psi internal pressure.

Proper implements, tools, and equipment satisfactory to the ENGINEER shall be provided and used by the CONTRACTOR for the safe and convenient prosecution of the work. All pipe and fittings shall be carefully lowered into the trench piece by piece by means of a derrick, ropes, or other suitable tools or equipment in such a manner as to prevent damage to the pipe. Under no circumstance shall pipe be dropped or dumped into the trench.

After the trench has been excavated to the proper grade, the first pipe at the outlet end of the sewer shall be bedded to the proper line and grade with the bell end upstream. All pipe shall be laid to line and grade. The pipe shall be held in place by backfilling along the bottom and sides of the pipe section with bedding material thoroughly tamped up to the centerline of the pipe and protected from movement.

During the pipe laying operation, the CONTRACTOR shall have a watertight plug available to install in the last pipe laid at the end of each work day, or to install during the work day, to prevent water, sediment, or other foreign material from entering the newly installed pipe.

The CONTRACTOR shall exercise due care so as to prevent water and other foreign matter from entering the newly constructed sewer mains at new manhole locations.

All joints shall be installed in accordance with the pipe manufacturer's instructions.

Where polyvinyl chloride sewer pipe is installed in a vitrified clay sewer line, V.C. to PVC adaptors shall be used at each junction. Adapters shall be Shear Guard repair couplers as manufactured by Indiana Seal, or approved equivalent.

The cost of adapters shall be considered incidental to the unit price bid for cast iron sewer pipe or polyvinyl chloride sewer pipe.

The interior of the pipe shall be cleaned as the work progresses. The manholes and sewer pipe shall be flushed with clean water after completion and prior to acceptance by the ENGINEER.

All connections to existing utilities shall be considered incidental to other bid items.

801-3.5 BACKFILLING OF PIPE TRENCH. After the pipe has been laid to line and grade, the trench shall be backfilled under and along the sides of the pipe up to the centerline of the pipe by thoroughly compacting bedding material into place so as to form a uniform bed for the pipe. The compaction may be obtained by any approved method or equipment which will produce a uniform density meeting the requirement to obtain not less than 85 percent maximum dry density at optimum moisture made in accordance with ASTM D1557. Care shall be exercised not to displace the pipe or damage the pipe during the compaction operations. If the material in the trench is sand or gravel and acceptable to the ENGINEER, it will not be necessary to furnish any other material than that found within the trench to backfill up to the centerline of the pipe. If sand or gravel is not found within the trench, the CONTRACTOR will be required to furnish bedding material as per Pipe Bedding Standard Plate. It will be required to keep the bedding completed within 3 lengths of the last pipe being laid and shall all be completed at the end of each day's work.

~~After bedding operations,~~ After the pipe has been inspected and bedding material is in place, two (2) feet of backfill shall be placed over the pipe to prevent damage to the

installed pipe. The remaining trench shall be backfilled ~~the trench shall be backfilled to a point 4 feet below finish grade by any approved method or equipment which will produce a uniform density meeting the requirements to obtain not less than 85 percent of the maximum dry density at optimum moisture as determined by ASTM Compaction Control Test Designation D1557.~~as per Section 801-3.6. On all areas outside of the pavement, curb and gutter, and sidewalk areas, no compaction will be required on the top 4 inches.

The use of drop pile hammers, loaded or unloaded clam shells or backhoe buckets, or other similar equipment will not be permitted to obtain the required density. The CONTRACTOR shall use specialized equipment or hand tamping around appurtenances such as manholes to ensure proper density. The remaining trench shall be backfilled in accordance with the specifications for the class of backfill as set forth in Section 801-3.6. The areas for each class of backfill specified shall be designated on the plans.

The CONTRACTOR shall engage an independent soils testing laboratory, approved by the ENGINEER, to determine the soil moisture density relationships and perform the required compaction testing.

The compaction control tests for this section are based on one individual compaction test per 200~~300~~ feet of trench per 36 inches of backfill or where directed by ENGINEER. Compaction tests shall be taken for service lines a minimum of one test per line, ~~2 feet below finish grade or as~~at a depth as directed by ENGINEER. The CONTRACTOR shall be responsible for all retesting of failing tests and a proctor determination to represent each soil condition to be encountered on the project. The time, locations, depths, and frequency of compaction testing shall be at the discretion of the ENGINEER during construction. Should it become necessary to require an additional number of initial compaction tests, over and above the number specified for bidding purposes, the ENGINEER will assume the responsibility to perform said additional testing. The CONTRACTOR, however, will be required to assume the cost of all retesting of failing tests regardless of the total number required during construction.

Compaction testing to determine densities may be accomplished with a nuclear density testing apparatus and/or the sand cone method. Should disputes arise concerning test results, they will be resolved by using the sand cone method.

Written reports of all test results shall be supplied to the ENGINEER and the CONTRACTOR by the testing laboratory as soon as possible.
The availability of the independent testing laboratory when needed and speed of testing and reporting are to be considered the responsibility of the CONTRACTOR.

Compaction control tests as stated above shall be incidental to other bid items.

All excess excavated material and material unsuitable for backfill shall become the CONTRACTORs property. The CONTRACTOR shall be responsible for disposal of and transporting material from the site.

The CONTRACTOR shall restore all shrubbery, fences, sod, or other surfaces disturbed to a condition equal to that before the work began, furnishing all labor and material incidental thereto. If the area cannot be restored to the original line and cross section without the aid of grade stakes, they will be furnished by the ENGINEER at the CONTRACTOR expense.

Following the certification of completion by the ENGINEER, the CONTRACTOR shall maintain the surface of unpaved trenches, adjacent curbs and gutters, sidewalks, driveways, shrubbery, fences, sod, or other surfaces disturbed for a period of 3 months thereafter. All material and labor required for maintenance of the trenches and adjacent structures shall be supplied by the CONTRACTOR and the work done in a manner satisfactory to the ENGINEER. The cost of backfilling and cleanup shall be included in the price per linear foot of sewer pipe in place.

801-3.6 BACKFILL CLASSIFICATIONS. ~~The top 4 feet of backfill~~ Backfill ~~below finish grade~~ shall meet the requirements of the backfill classification as per plans, if no backfill class is specified, backfill shall be Class A.
~~Class AA backfill shall be used in areas where the trenches fall beneath special improved areas and under special conditions, and these areas shall be indicated as Class AA backfill and shown on the plans. Under Class AA backfill all the excavated material shall be transported to another site and wasted in a workmanlike manner, and selected material meeting bedding material specifications shall be imported to the site for backfill material.~~
Any costs associated with obtaining ~~moisture~~ backfill requirements shall be incidental.

After the pipe has been inspected and bedding material and initial two feet of backfill is in place, as per Section 801-3.5, the remaining trench shall be backfilled in layers of not more than twelve inches and compacted by any approved method or equipment which will produce a uniform density meeting the requirements to obtain the following at optimum moisture in accordance with ASTM D1557 (modified proctor):

(a) Class AA Backfill. Not less than 95 percent maximum dry density.

(b) Class A Backfill. Not less than ~~85~~ 90 percent maximum dry density.

(c) Class B Backfill. Not less than 85 percent of maximum dry density.

(d) Class C Backfill. Equal to the adjacent undisturbed soil but not to exceed 85 percent of maximum dry density.

(e) Class D Backfill. No minimum.

When Class D backfill is specified, material shall be backfilled in 24 to 36 inch layers compacted by any approved method or equipment which will obtain a uniform density.

801-3.6 BACKFILL CLASSIFICATIONS. The top 4 feet of backfill below finish grade shall meet the requirements of the backfill classification as per plans, if no backfill class is specified, backfill shall be Class A. Any costs associated with obtaining moisture requirements shall be incidental.

(a) Class AA Backfill. Class AA backfill shall be used in areas where the trenches fall beneath special improved areas and under special conditions, and these areas shall be indicated as Class AA backfill and shown on the plans. Under Class AA backfill all the excavated material shall be transported to another site and wasted in a workmanlike manner, and selected material meeting bedding material specifications shall be imported to the site for backfill material.

After the pipe has been inspected and bedded with bedding material, and upon completion and approval for the initial backfill requirements specified under Subsection 801-3.5, the remaining trench shall be backfilled in layers and compacted by any approved method or equipment which will produce a uniform density meeting the requirements to obtain not less than 95 percent maximum dry density at optimum moisture in accordance with ASTM D1557.

(b) Class A Backfill. Class A backfill shall be used in areas where trenches fall beneath improved areas or areas to be improved, and these areas shall be indicated as Class A backfill and shown on the plans.

After the pipe has been inspected and bedded with bedding material, and upon completion and approval of the initial backfill requirements specified under Section 801-3.5, the remaining trench shall be backfilled in layers and compacted by any approved method or equipment which will produce a uniform density meeting the requirement to obtain not less than 85 percent maximum dry density at optimum moisture made in accordance with ASTM D1557, except for the top four feet of the trench which shall meet the requirement to obtain not less than 90 percent at maximum dry density with a moisture content falling within plus or minus three percentage points of the optimum moisture made in accordance with ASTM D1557.

(c) Class B Backfill. Class B backfill shall be used in areas where the trenches fall beneath improved areas or areas to be improved, and these areas shall be indicated as Class B backfill and shown on the plans.

After the pipe has been inspected and bedded with bedding material, and upon completion and approval of the initial backfill requirements specified under Section 801-3.5, the remaining trench shall be backfilled in layers and compacted by any approved method or equipment which will produce a uniform density meeting the requirement to obtain not less than 80 percent of maximum dry density at optimum moisture made in accordance with ASTM D1557, except for the top four feet of trench which shall meet the requirement to obtain not less than 85 percent of maximum dry density at an optimum moisture in accordance with ASTM D1557.

~~**(d) Class C Backfill.** Class C backfill shall be used in areas where the trenches fall beneath improved areas or areas to be improved, and these areas shall be indicated as Class C backfill and shown on the plans.~~

~~After the pipe has been inspected and bedded with bedding material, and upon completion and approval of the initial backfill requirements specified under Section 801-3.5, the remaining trench shall be backfilled in layers and compacted by any approved method or equipment which will produce a uniform density equal to the adjacent undisturbed soil but not to exceed 85 percent of maximum dry density at optimum moisture made in accordance with ASTM D1557.~~

~~**(e) Class D Backfill.** Class D backfill shall be used in unimproved areas. These areas shall be indicated as Class D backfill and shown on the plans. After the pipe has been inspected and bedded with bedding material and upon completion and approval of the initial backfill requirements specified under Section 801-3.5, the remaining trench shall be backfilled in 24 to 36 inch layers compacted by any approved method or equipment which will obtain a uniform density.~~

801-3.7 CONNECT TO MANHOLE. Connections to existing structures and new manholes, which are not pre-formed, for sanitary sewer shall be core drilled unless otherwise approved by the ENGINEER. Connections shall be made with flexible pipe to manhole connector (rubber boot) or with PVC manhole adaptor per Section 1205. If connector is used, two bands shall be required to secure boot to pipe.

801-3.8 PROTECTING UNDERGROUND AND SURFACE STRUCTURES. Temporary support, adequate protection, and maintenance of all underground and surface structures, culverts, storm sewer, sanitary sewer, watermain, service connections for both sewer and water, and other obstructions encountered in the progress of the work shall be furnished by the CONTRACTOR all at its own expense as approved by the ENGINEER.

(a) Deviations Occasioned by Other Utility Structures. Wherever existing utility structures or branch connections leading to main sewer or water mains or other conduits, ducts, pipes, or structures form obstructions to the grade and alignment of the sewer to be laid, they shall be permanently supported, removed, relocated, or reconstructed by the CONTRACTOR through cooperation with the Owner of the utility, structure, or obstruction involved. In those instances where their relocation or reconstruction is impracticable, a deviation from the line and grade will be ordered by the ENGINEER, and the change shall be made in the manner directed by the ENGINEER.

Wherever possible, all existing utility structures, or branch connections leading therefrom, will be located in advance of the excavation of the trench and properly marked. The CONTRACTOR shall not cut any existing utility lines unless it is determined by the ENGINEER that it is necessary in order to install the new sewer pipes. All utility lines that are cut by the CONTRACTOR with the approval of the ENGINEER shall be repaired or replaced by the CONTRACTOR as Extra Work. All

utility lines that are damaged by the CONTRACTOR shall be repaired or replaced by the CONTRACTOR at the CONTRACTOR's expense.

Wherever the ENGINEER shall determine it is necessary to remove or relocate any existing utility in order to properly install the new sewer pipe, the change shall be made in a manner directed by the ENGINEER and for which extra compensation will be allowed the CONTRACTOR.

(b) Deviation Without Engineer's Consent. No deviation shall be made from the required line and grade established by the ENGINEER without the consent of the ENGINEER.

(c) Subsurface Explorations. Whenever necessary to determine the location of existing pipes, valves, or other underground structures, the CONTRACTOR, after examination of available records and upon written order from the ENGINEER, shall make all exploration and excavations for such purpose for which the ENGINEER may allow extra compensation.

801-3.9 CIRCULAR DEFLECTION TEST. All flexible pipe of 8 inches in diameter or larger shall be tested by the CONTRACTOR to ensure that circular deflections do not exceed the maximum allowable deflection. Maximum allowable deflections shall be governed by the mandrel requirements stated herein and shall nominally be 5 percent.

The maximum average inside diameter shall be equal to the average outside diameter per applicable ASTM Standards minus 2 minimum wall thicknesses per applicable ASTM Standards. Manufacturing and other tolerances shall not be considered for determining maximum allowable deflections.

Deflection tests shall be performed a minimum of 30 days after the pipe has been fully backfilled and received passing compaction tests per Section 801-3.5 “Backfilling of Pipe Trench.”

The mandrel shall be pulled through the pipe by hand to ensure that maximum allowable deflections have not been exceeded. Prior to use, the mandrel shall be certified by the ENGINEER. If the mandrel fails to pass through the pipe, it will be deemed to be over-deflected.

The rigid ball or mandrel used for the deflection test shall have a diameter not less than 95 percent of the base inside diameter or average inside diameter of the pipe depending on which is specified in the ASTM Specification, including the appendix, to which the pipe is manufactured. The test shall be performed without mechanical pulling devices. The mandrel shall be fabricated of steel or aluminum and shall have pull rings at either end. The mandrel shall be stamped or engraved indicating the pipe material specification, nominal size, and mandrel outside diameter. The maximum average inside diameter of the pipe shall be measured and calculated by the ENGINEER in the field prior to installation.

Unless otherwise permitted by the ENGINEER, any over-deflected pipe shall be uncovered and, if not damaged, removed and reinstalled. Damaged pipe shall be removed from the work site and replaced with new pipe. Any pipe requiring replacement shall be retested at the expense of the CONTACTOR.

All costs incurred by the CONTRACTOR attributable to mandrel and deflection testing, including any delays and reinstallation of deflected pipe, shall be considered incidental to the installation of the pipe.

801-3.10 LEAKAGE TESTS. The CONTRACTOR shall provide one of either a Hydrostatic Test or an Air Test as specified below:

(a) Hydrostatic Test. The CONTRACTOR shall perform an exfiltration or infiltration test with a minimum positive head of 2 feet.

Allowable exfiltration or infiltration shall not exceed 100 gallons per inch of internal pipe diameter, per mile, per day.

(b) Air Test. The CONTRACTOR shall conduct an air test, as a minimum, conforming to the test procedure as described in ASTM F1417 for plastic pipe. For other materials, test procedures shall be approved by the ENGINEER.

801-3.11 TELEVISE SEWER MAIN. All newly constructed sanitary sewer mains shall be televised. If not specified as a bid item, the televising shall be considered incidental to the price bid for the sanitary sewer installation. After flushing the sewer main, under Section 801-3.4, the CONTRACTOR shall have the sewer main televised and recorded by a firm normally engaged in such type of work. The CONTRACTOR shall provide a high-quality digital video with a report for each section of sewer main televised. The recording shall be clearly marked as to the project number and recording number. The recording shall describe locations and conditions of the sewer and shall have a visual footage counter showing the distance of the camera from the manhole. After the CONTRACTOR has submitted the recordings and report, they will be viewed by the ENGINEER for acceptance. Any sewer failing inspection shall be replaced and re-televised at the expense of the CONTRACTOR.

801-3.12 CLEANOUT. Cleanouts shall be constructed in accordance with the standard detail 801-1.

801-3.13 SANITARY SEWER FORCE MAIN. The construction requirements for sanitary sewer force mains shall comply with Section 901 “Water Mains,” with the exception of the hydrostatic pressure tests, disinfection, and bacteriological testing. The hydrostatic pressure test shall be the same as Section 901 “Water Mains,” except the hydrostatic pressure test shall be 125 pounds per square inch and shall be held for 2 hours. No pipe disinfection or bacteriological testing shall be required.

801-3.14 CONNECTION TO EXISTING SEWER MAIN. Whenever a wye branch is not available for a sewer service connection, the connection to the sewer main shall consist of one of the following:

a. A “factory assembled” wye branch may be cut into an existing PVC sewer main using gasketed repair couplings to the existing PVC sewer main.

b. A “factory assembled” wye branch may be cut into an existing VC sewer main using Shear Guard couplings, or approved equivalent, to the existing VC sewer main.

c. PVC, VC, or RCP sewer main may be connected to the existing VC sewer main service using an Inserta Tee as manufactured by Inserta Fittings Co., or approved equivalent. The City of Bismarck Public Works Department will make the tap into the existing PVC, VC, or RCP sewer main. Call to schedule and for the current price of tap.

801-3.15 WYE BRANCH LOCATIONS. Wye branches shall be marked with a 2 inch by 2-inch by 4-foot stake placed perpendicular to the main line sewer at the end of the wye.

801-3.16 INSULATE SEWER. The CONTRACTOR shall furnish and install the insulation required to insulate the sewer as shown on the plans. The insulation shall be at least 4 inches thick by 8 feet wide centered on the sewer main. The material between the top of the sewer bedding and the insulation shall consist of a concrete sand.

801-4 MEASUREMENT AND PAYMENT

801-4.1 thru 4.13 (X)” SANITARY SEWER PIPE.

Sanitary sewer pipe shall meet the requirements of Section 801-2. Sanitary sewer pipe shall be measured by the linear foot (LF) from the centerline of manhole to centerline of manhole and shall be paid for at the unit price bid for "(X)” Sanitary Sewer Pipe" complete, in place, and accepted by the ENGINEER.

801-4.50 thru 4.59 (X)” WYE BRANCH. Wye branches shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "(Size) Wye Branch" complete, in place, and accepted by the ENGINEER.

801-4.60 BEDDING MATERIAL. Bedding Material gravel shall be measured by the ton and paid for at the unit price bid for "Bedding Material" complete, in place, and accepted by the ENGINEER.

801-4.61 SUBCUT GRAVEL. Subcut Gravel shall be measured by the ton and paid for at the unit price bid for "Subcut Gravel" complete, in place, and accepted by the ENGINEER.

801-4.62 ROCK EXCAVATION. All rock found in the trench area greater than 1 cubic foot shall be classified as Rock Excavation, measured by the cubic yard (CY), and disposed of by the CONTRACTOR or as directed by the ENGINEER.

The CONTRACTOR shall place all rocks greater than 1 cubic foot and less than 1 cubic yard in a pile to be measured by the ENGINEER. The total volume of the stockpile shall be reduced by 25 percent to account for void in the rock stockpile.

All rocks greater than 1 cubic yard shall be individually measured by the ENGINEER.

Payment shall be made at the unit price bid per cubic yard (CY) for "Rock Excavation."

801-4.64 TELEVISE SEWER MAIN. Televise Sewer Main shall be measured by the linear foot (LF) from centerline of the manhole to centerline of the manhole or an end point and shall be paid for at the unit price bid for “Televise Sewermain” complete, and accepted by the ENGINEER.

801-4.65 thru 801-4.68 (X)” CLEANOUT. Cleanouts shall meet the requirements of this section and the standard detail and shall be measured and paid for per each (EA) "(X)” Cleanout" complete, in place, and accepted by the ENGINEER.

801-4.80 INSULATE SEWER. Insulate Sewer shall be measured by the linear foot (LF) of sewer to be insulated and paid for at the unit price bid for "Insulate Sewer" complete, in place, and accepted by the ENGINEER.

SECTION 802 − STORM SEWER

802-1 DESCRIPTION

This item shall consist of pipe and related items of the types, classes, sizes, and dimensions required on the plans, furnished and installed at the places designated on the plans and profiles, or by the ENGINEER, in accordance with these specifications and with the lines and grades given.

The bid price per linear foot of pipe in place shall include the cost of excavation and backfill, the cost of furnishing and installing all trench bracing, all fittings required to complete the pipe drain, as shown on the plans, and the material for and the making of all joints, including all connections to existing drainage pipe and manholes.

"Unstable," "Unsuitable," "Suitable," and "Unsatisfactory" soil or aggregate items shall be defined as stated in Section 202.

802-2 MATERIALS

802-2.1 GENERAL. The pipe shall be of the type called for on the plans or in the proposal and shall be in accordance with the following appropriate requirements.

802-2.2 REINFORCED CONCRETE STORM SEWER PIPE. Reinforced concrete storm sewer pipe shall conform to the requirements of ASTM C76.

Unless otherwise specified, all pipe shall be Class III for 24-inch and smaller and Class II for 27-inch and larger in accordance with ASTM C76, Wall B.

All pipe sections shall be cast in sections 8 feet, 6 feet, or 4 feet long, with the exception of the variable length sections may be cast to match at manholes and inlets.

802-2.3 POLYVINYL CHLORIDE STORM SEWER PIPE. Polyvinyl chloride storm sewer (PVC) pipe 15 inches or smaller shall conform to the requirements of ASTM D3034 for type PSM, PVC sewer pipe and shall have an SDR of 35, all of which shall be stamped on the pipe. Polyvinyl chloride sewer pipe 18 inches or larger shall conform to the requirements of ASTM F679-PS46. PVC sewer shall have the elastomeric gasket type joint providing a watertight seal.

802-2.4 CORRUGATED STEEL STORM SEWER PIPE. Corrugated steel storm sewer pipe shall have a zinc coating weight of 2 ounces per square foot and shall conform to the requirements of AASHTO M36. This material may be used if approved by the ENGINEER.

802-2.5 HIGH DENSITY POLYETHYLENE STORM SEWER PIPE High Density polyethylene storm sewer pipe shall be of the quality to that manufactured by

ADS/Hancor, or approved equivalent. The pipe shall have a smooth interior and annular exterior corrugations. The 4-inch to 60-inch pipe with integral bell and spigot joints shall conform to ASTM F2306. The joint shall be water tight and gaskets shall conform to ASTM F477. Fittings shall conform to ASTM F2306. Bell and spigot connections shall utilize a spun-on or welded bell. Valley or saddle gaskets shall meet the soil-tight performance requirements of ASTM F477.

802-2.6 RIBBED POLYVINYL CHLORIDE STORM SEWER PIPE. Ribbed polyvinyl chloride storm sewer pipe shall be of a quality equal to that manufactured by Extrusion Technologies, Inc. Ultra-Rib, or approved equivalent. The pipe shall meet the requirements of ASTM F794 and shall have a smooth interior. The pipe shall have a bell end and a spigot end which shall be connected using elastomeric gaskets. The pipe stiffness shall be a minimum of 46 psi when tested at 5 percent deflection in accordance with ASTM D2412.

802-2.7 CORRUGATED POLYVINYL CHLORIDE STORM SEWER PIPE. Corrugated polyvinyl chloride storm sewer pipe shall be manufactured by Extrusion Technologies, Inc. Ultra-Corr, or approved equivalent. The pipe shall meet the requirements of ASTM F949 and shall have a smooth interior. The pipe shall have a bell end and a spigot end which shall be connected using elastomeric gaskets. The pipe stiffness shall be a minimum of 50 psi when tested at 5 percent deflection in accordance with ASTM D2412.

802-2.8 PERFORATED PIPE. Perforated concrete pipe in sizes 4 inches and above shall conform to the requirements of ASTM C444, Type 1 or 2. Corrugated HDPE perforated pipe in sizes 3 inches to 6 inches shall conform to ASTM F405. Corrugated HDPE perforated pipe in sizes 8 inches to 24 inches shall conform to ASTM F667. If PVC perforated pipe is selected, it must meet the requirement of Section 802-2.6, or an approved equivalent.

802-2.9 (TYPE OF PIPE) ARCH PIPE. Arch pipe shall conform to the same requirements as listed for standard pipe of like material.

802-2.10 FLARED END SECTIONS. Flared end sections shall be RCP material for the flared end section and 2 adjacent pipe sections. The 2 sections and the flared end section shall be tied together using joint ties, or an approved equivalent.

802-2.11 CONCRETE MANHOLES AND INLETS. Concrete manholes and inlets shall conform to Section 1205.

802-2.12 MORTAR. Mortar for pipe joints and connections to other drainage structures shall be composed of 1 part, by weight, of portland cement and 2 parts of mortar sand. The portland cement shall conform to the requirements of Section 501. The sand shall conform to the requirements of Section 501. Hydrated lime may be added to the

mixture of sand and cement in an amount equal to 15 percent of the weight of cement used. The hydrated lime shall meet the requirements of ASTM C6.

802-2.13 CONCRETE. Concrete for pipe cradles shall conform to the requirements of Section 501.

802-2.14 CONCRETE PIPE JOINTS. Concrete pipe joints for non-pressure pipe shall be sealed with a butyl joint mastic. Butyl joint mastic shall be equal to EZ-STIK as manufactured by the Press-Seal Gasket Corporation, or approved equivalent. The CONTRACTOR shall use 1/2-inch butyl for 12-inch to 18-inch RCP, 3/4-inch for 21-inch to 36-inch RCP, 1-inch for 42-inch to 66-inch RCP, and 1½-inch for 72-inch to 120-inch RCP. Rubber-type gaskets for concrete low-head pressure pipe shall conform to the requirements of ASTM C443 and ASTM 361. if specified.

802-2.15 GASKET JOINT FOR PVC STORM SEWER PIPE. Gaskets for PVC storm sewer pipe joints shall be of the elastomeric type providing a watertight seal.

802-2.16 BEDDING MATERIAL. The bedding material shall be defined as stated in Section 801.

802-2.17 SUBCUT GRAVEL. The subcut gravel shall be as defined in Section 801.

802-2.18 MARKING TAPE. The CONTRACTOR will be required to furnish and install marking tape located 2 feet above the top of all storm sewer mains installed under the contract. In cases where there is less than 30 inches of fill material over the top of the pipe, the tape shall be placed 12 inches from the top of the pipe. The tape shall be of the non-detectable type and shall have a minimum width of 5 inches. The tape shall be green in color with the words "CAUTION SEWER LINE BELOW" imprinted on the tape in black capital letters. The marking tape shall be manufactured by Presco standard grade or approved equivalent

Cost of marking tape and installation shall be considered incidental to other items.

802-2.19 RIPRAP. Rock shall be hard, durable, angular in shape, and free from cracks, overburden, shale, and organic material. The width and the thickness of a single stone shall each be less than one-third the length of the stone. Rock shall not sustain a loss of more than 40 percent after 500 revolutions in an abrasion test conducted in accordance with ASTM C535-69. Rock shall not sustain a loss of more than 10 percent after 12 cycles of freezing and thawing (AASHTO T103 for ledge rock, procedure A). Rock shall have a minimum specific gravity of 2.50.

CONTRACTOR shall be responsible for all costs of testing rock for compliance with these specifications. In lieu of testing proposed rock for compliance with these specifications, rock obtained from County or North Dakota Department of Transportation

approved quarries may be used. All rock materials considered for use as riprap shall have prior approval by the ENGINEER before being placed.

Gradation for riprap are as follows:

Riprap	% Smaller than Given Size by Weight	Intermediate Rock Dimension (Inches)	$^{d}50^{*}$ (Inches)
Type VL	70-100	12	–
	50-70	9	–
	35-50	6	6**
	2-10	2	–
Type L	70-100	15	–
	50-70	12	–
	35-50	9	9**
	2-10	3	–
Type M	70-100	21	–
	50-70	18	–
	35-50	12	12
	2-10	4	–
Type H	100	30	–
	50-70	24	–
	35-50	18	18
	2-10	6	–
Type VH	100	42	–
	50-70	33	–
	35-50	24	24
	2-10	9	–

*$^{d}50$ = Mean particle size
**Bury types VL and L with native topsoil and revegetate to protect from vandalism.

802-2.20 CULVERTS. Culverts installed within CITY right-of-way shall be reinforced concrete pipe (RCP) or corrugated steel storm sewer pipe.

802-2.21 STORM SEWER FORCE MAIN MATERIALS. All materials for construction of storm sewer force mains shall conform to Section 901 “Water Mains.”

802-3 CONSTRUCTION REQUIREMENTS

802-3.1 EQUIPMENT. All equipment necessary and required for the proper construction of storm sewers shall be on the project in proper working condition and approved by the ENGINEER before construction is permitted to start.

The CONTRACTOR shall provide appropriate hoisting equipment to handle the pipe while unloading and placing it in its final position without damage to the pipe.

The CONTRACTOR shall provide method and means to obtain the required compaction of the pipe bed and the backfill as specified.

The CONTRACTOR shall provide a sufficient number of watertight sewer plugs to prevent infiltration of water and any other foreign material from entering the existing sewer system and the newly constructed sewer lines.

802-3.2 EXCAVATION AND PREPARATION OF TRENCH. Excavation and preparation of the trench for storm sewer construction shall conform to Section 801 with the following additions:

HDPE sewer pipe shall have bedding material installed to 6 inches over the top of the pipe. Bedding material from the center of the pipe to 6 inches over the pipe shall be considered incidental to the pipe items.

If perforated storm drain is installed, the fine aggregate shall conform to Section 802-3.7.

802-3.3 ROCK EXCAVATION. The rock excavation shall conform to Section 801.

802-3.4 PIPE LAYING. Pipe laying shall conform to Section 801 with the following additions:

Connections between HDPE and RCP shall be made using an internal coupler spigot adaptor equivalent to the Mar-Mac Coupler manufactured by Advanced Drainage Systems per Detail No. 802-2. This work shall be incidental to the storm sewer pipe.

802-3.5 SIX-INCH CLEANOUT - IN-LINE. Where shown on the plans, storm sewer in-line cleanouts shall be constructed according to the corresponding Detail (802-3) in the City of Bismarck Construction Specifications and conform to the following criteria. The pipe shall be polyvinyl chloride sanitary sewer (PVC) pipe. Pipe that is 15 inches or smaller shall conform to the requirements of ASTM D3034 for TYPE PSM, PVC sewer pipe and fittings and shall have an SDR of 35, all of which shall be stamped on the pipe. Polyvinyl chloride sewer pipe 18 inches or larger shall conform to the requirements of ASTM F679-PS46. PVC sewer main line pipe and PVC sewer service pipe shall have the elastomeric gasket-type joint providing a watertight seal. A solvent cement-type joint will not be allowed. PVC wye branches shall be of the "factory-assembled type." The top of the pipe shall have a PVC threaded clean out adapter with a PVC threaded

plug placed under a Neenah No. R-1976 or East Jordan Iron Works No. 1578 or approved equivalent cover set in concrete as per the aforementioned detail.

802-3.6 SIX-INCH CLEANOUT - END OF RUN. Where shown on the plans, storm sewer end-of-line cleanouts shall be constructed according to the corresponding detail (802-4) in the City of Bismarck Construction Specifications and conform to the following criteria. The pipe shall be polyvinyl chloride sanitary sewer (PVC) pipe. Pipe that is 15 inches or smaller shall conform to the requirements of ASTM D3034 for type PSM, PVC sewer pipe and fittings and shall have an SDR of 35, all of which shall be stamped on the pipe. Polyvinyl chloride sewer pipe 18 inches or larger shall conform to the requirements of ASTM F679-PS46. PVC sewer main line pipe and PVC sewer service pipe shall have the elastomeric gasket-type joint providing a watertight seal. A solvent cement-type joint will not be allowed. The PVC 90 degree bend shall be a sweeping bend; right angle bends will not be accepted. The top of the pipe shall have a PVC threaded clean out adapter with a PVC threaded plug placed under a Neenah No. R-1976 or East Jordan Iron Works No. 1578 or approved equivalent cover set in concrete as per the aforementioned detail.

802-3.7 DRAINAGE AGGREGATE. The drainage aggregate shall meet either of the gradations in the following table.

NDDOT Class 43	
Sieve Size	**Percent Passing**
3/8”	100
No. 4	20-70
No. 8	0-17
No. 200	0-2
Shale	8.0%

NDDOT Class 2	
Sieve Size	**Percent Passing**
3/4”	100
3/8”	50-95
No. 10	0-15
No. 30	0-4

802-3.8 BACKFILLING OF PIPE TRENCH. Backfilling shall conform to Section 801-3.5 with the following additions:

When backfilling perforated pipe, the CONTRACTOR shall backfill with fine aggregate conforming to Section 501 to a point 2 feet below the finished surface. The remaining 2 feet shall be backfilled with existing spoil. The excess spoil shall be disposed of by the CONTRACTOR, incidental to other bid items. Care shall be taken when backfilling

around the pipe to prevent damage to the trench section surrounded by the geotextile fabric.

When backfilling flexible pipe, the CONTRACTOR shall place and compact bedding material to a point 6 inches above the top of the pipe. Care shall be used not to over-deflect the roundness of the pipe. For flexible pipe, the pipe bedding shall be considered incidental from the center of the pipe to 6 inches over the top of the outside of the pipe.

The remaining trench shall be backfilled in accordance with the specifications for the class of backfill as set forth in Section 801. The areas for each class of backfill specified shall be designated on the plans.

802-3.9 BACKFILL CLASSIFICATIONS. The backfill classifications shall be as defined in Section 801.

802-3.10 PROTECTING UNDERGROUND AND SURFACE STRUCTURES. Temporary support, adequate protection and maintenance of all underground and surface structures, drains, sewers, water mains, service connections for both sewer and water, and other obstructions encountered in the progress of the work shall be furnished by the CONTRACTOR all at its own expense as approved by the ENGINEER.

(a) Deviations Occasioned by Other Utility Structures. Wherever existing utility structures or branch connections leading to main sewer or water mains or other conduits, ducts, pipes, or structures form obstructions to the grade and alignment of the sewer to be laid, they shall be permanently supported, removed, relocated, or reconstructed by the CONTRACTOR through cooperation with the Owner of the utility, structure, or obstruction involved. In instances where their relocation or reconstruction is impracticable, a deviation from the line and grade will be ordered by the ENGINEER, and the change shall be made in the manner directed by the ENGINEER.
Wherever possible, all existing utility structures or branch connections leading therefrom will be located in advance of the excavation of the trench and properly marked. The CONTRACTOR shall not cut any existing utility lines unless it is determined by the ENGINEER that it is necessary to install the new sewer pipes. All utility lines that are cut by the CONTRACTOR with the approval of the ENGINEER shall be repaired or replaced by the CONTRACTOR as Extra Work.

All utility lines that are damaged by the CONTRACTOR shall be repaired or replaced by the CONTRACTOR at the CONTRACTOR's expense.

Wherever the ENGINEER shall determine it is necessary to remove or relocate any existing utility to properly install the new sewer pipe, the change shall be made in a

manner directed by the ENGINEER and for which extra compensation will be allowed the CONTRACTOR.

(b) Deviation Without Engineer's Consent. No deviation shall be made from the required line and grade established by the ENGINEER without the consent of the ENGINEER.

(c) Subsurface Explorations. Whenever necessary to determine the location of existing pipes, valves, or other underground structures, the CONTRACTOR, after examination of available records and upon written order from the ENGINEER, shall make all explorations and excavations for such purpose for which the ENGINEER may allow extra compensation.

802-3.11 CIRCULAR DEFLECTION TEST. All fittings and ~~plastic or HDPE~~flexible pipe of 8 inches in diameter or larger shall be tested by the CONTRACTOR to ensure that circular deflections do not exceed the maximum allowable deflection. The CONTRACTOR shall test in accordance with Section 801 "Circular Deflection Test." Any pipe requiring replacement shall be retested at the expense of the CONTRACTOR.

Deflection tests shall be performed a minimum of 30 days after the pipe has been fully backfilled or after gravel section is in place and compacted when pipe is under roadway, and received passing compaction tests per Section 801 Backfilling of Pipe Trench.

802-3.12 RIPRAP. Hand placement of riprap may be required to ensure an acceptable gradation, uniform surface, and to fill gaps between larger rocks to cover any exposed riprap fabric.

Type VL riprap and Type L riprap shall be buried with topsoil and revegetated. All items shall be considered incidental to the bid price for riprap.

Riprap fabric shall be used under the riprap as bedding. All costs for providing and installing the riprap fabric shall be incidental to the riprap.

Riprap grout shall be installed on a 4-inch thick layer of granular material. The granular material shall be in accordance with Section 801 "Bedding Material." The riprap prior to the grout placement must be as clean as practical. The grout shall be delivered to the place of final deposit by means that will ensure uniformity and prevent segregation of the grout. Placing of grout shall be obtained by pumping under pressure through a 2-inch maximum diameter hose to ensure complete penetration of the grout into the rock layer. A vibrator is to be employed near the nozzle during placement to aid the flow of the grout. The excess grout must be removed immediately by washing to leave a clean rock face exposed. Grout shall fill the voids to within approximately 4 inches of the riprap surface. The recommended minimum grout specifications include entrained air, a 28-day strength of at least 2,400 pounds per square inch, and a high slump (5-7 inches) in order to penetrate either the full depth of the riprap layer or at least 2 feet where the riprap layer s thicker than 2 feet. Concrete having maximum aggregate size of 3/4 inch may be substituted for grout when using Type M riprap or larger.

802-4 MEASUREMENT AND PAYMENT

802-4.1 thru 4.24 (X)" STORM SEWER PIPE. (X)" Storm Sewer Pipe shall be measured by the linear foot (LF) from centerline of manhole or inlet to centerline of manhole or inlet. If no inlet or manhole is installed, pipe shall be measured to end of pipe section. Flared end sections shall be paid under separate bid item. Items shall be paid for at the unit price for "(X)" Storm Sewer Pipe" complete, in place, and accepted by the ENGINEER.

802-4.25 thru 4.35 (X)" ARCHED SEWER PIPE. (X)" Arch Storm Sewer Pipe shall be measured by the linear foot (LF) from centerline of manhole or inlet to centerline of manhole or inlet and paid for at the unit price bid for "(X)" Arched (S or St) Sewer Pipe" complete, in place, and accepted by the ENGINEER.

802-4.36 thru 4.50 (X)" CORRUGATED STEEL STORM SEWER PIPE. (X)" Corrugated Steel Storm Sewer Pipe shall be measured by the linear foot (LF) from centerline of manhole or inlet to centerline of manhole or inlet and paid for at the unit price bid for "(X)" Corr Steel St Sewer Pipe" complete, in place, and accepted by the ENGINEER.

802-4.51 thru 4.79 (X)" FLARED END SECTION. (X) " Flared End Sections shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "(X)" Flared End Section" complete, in place, and accepted by the ENGINEER.

802-4.80 thru 4.89 (X)" PERFORATED PIPE. "(X)" Perforated Pipe" shall be measured and paid by the linear foot (LF) in place and accepted by the ENGINEER. Bends, tees, caps and coupling bands shall be considered incidental to the unit price bid.

802-4.90 BEDDING MATERIAL. Bedding Material shall be measured and paid for under Section 801-4.60, with the following revision:

When no pipe material is specified, any additional bedding material required due to installing pipe material other than RCP shall be considered incidental to the pipe and no additional compensation shall be made. If pipe material other than RCP is specified, material shall be measured and paid as per Section 801-4.60.

802-4.91 SUBCUT GRAVEL. Subcut Gravel shall be measured and paid for under Section 801-4.61.

802-4.92 ROCK EXCAVATION. Rock Excavation shall be measured and paid for under Section 801-4.62.

802-4.94 thru 802-4.98 RIPRAP - TYPE ( ). Riprap - Type ( ) shall be measured by the ton and paid for at the unit price bid for "Riprap-Type ( )" complete, in place, and accepted by the ENGINEER.

802-4.99 RIPRAP GROUT. “Riprap Grout” shall be paid for by the cubic yard (CY) of riprap grout installed and accepted by the ENGINEER. Granular bedding material and installation shall be considered incidental to the price bid for grouted riprap.

802-4.100 SIX INCH CLEANOUT - IN LINE. ”6” Cleanout – In-Line” shall be paid for by (EA) in place and accepted by the ENGINEER.

802-4.101 SIX INCH CLEANOUT - END OF RUN. ”6” Cleanout - End of Run” shall be paid for by each (EA) in place and accepted by the ENGINEER.

802-4.102 DRAINAGE AGGREGATE. Drainage Aggregate shall be paid by the ton for “Drainage Aggregate” in place and accepted by the ENGINEER .

DIVISION 900

WATER DISTRIBUTION

SECTION 901 – WATERMAIN

901-1 DESCRIPTION

This item shall consist of watermain pipe and related items of the types, classes, sizes, and dimensions required on the plans, furnished and installed at the places designated on the plans and profiles, or by the ENGINEER in accordance with these specifications and with the lines and grades given.

The bid price per linear foot of pipe in place shall include the cost of excavation and backfill, the cost of furnishing and installing all trench bracing, concrete bases, and concrete thrust blocking, and the material for and the making of all joints, including all connections to existing watermain.

"Unstable," "Unsuitable," "Suitable," and "Unsatisfactory" soil or aggregate items shall be defined as stated in Section 202.

901-2 MATERIALS

901-2.1 GENERAL. All materials that may come into contact with water intended for use in a public water system shall meet the American National Standards Institute (ANSI) / National Sanitary Foundation International (NSF) Standard 61. A product will be considered as meeting this standard if so certified by NSF, the Underwriters Laboratories, or other organization accredited by ANSI to test and certify such products. The materials shall be of the type selected by the CONTRACTOR and in accordance with the following appropriate requirements unless otherwise specified.

901-2.2 POLYVINYL CHLORIDE (PVC) PIPE. Polyvinyl chloride pipe (PVC) or molecularly oriented PVC (PVCO) shall meet the requirements of AWWA C900 or C905 or C909, or the latest revision thereof, and shall be furnished in ductile iron pipe (DIP) equivalent outside diameters with elastomeric joints. The pressure class of PVC pipe shall be PC235 with a DR of 18. PVCO pipe shall be pressure class PC235 (AWWA C900 DR18 equivalent).

Where shown on the plans, restrained joint pipe and fittings shall be used. Restrained jointing systems require approval of the ENGINEER. Preapproved restraining systems include Certa-Lok, Yelomine, and EBAA Iron, Inc (MEGALUG). The CONTRACTOR shall note that the standard mechanical joint is not a restrained joint.

901-2.3 HIGH DENSITY POLYETHYLENE (HDPE) PIPE. Pipe shall be manufactured in accordance with PE4710 conforming to the latest ANSI/AWWA C906 and NSF/ANSI 61. For potable water applications, PE4710 compound shall conform to ASTM D3350 minimum cell classification 445574C-CC3. The minimum wall thickness of the high-density polyethylene pipe shall meet the requirements of DR 11 pipe with ductile iron pipe outside diameters. HDPE pipe shall conform to the requirements of ASTM F714 for Polyethylene (PE) Plastic Pipe.

All pipes shall be made of virgin material. No rework except that obtained from the manufacturer of the same formulation shall be used. The pipe shall be homogeneous throughout and be free of faults such as visible cracks, holes, foreign material, and blisters.

901-2.4 DUCTILE IRON PIPE. Ductile iron pipe shall be manufactured in accordance with the requirements of AWWA/ANSI C151/A21.51. Push-on joints and mechanical joints shall be manufactured in accordance with AWWA/ANSI C111/A21.11. Pipe thickness shall be designated in accordance with AWWA/ANSI C150/A21.50. All pipe under 16 inches shall use pressure Class 350. All 16-inch to 20-inch pipe shall use pressure class 250 or higher. All 24-inch pipe shall be pressure Class 200 or higher. All 30-inch pipe or larger shall be pressure Class 150 or higher. All pipe shall be supplied with a cement mortar lining in accordance with AWWA/ANSI C104/A21.4. All pipe shall have a bituminous exterior coating in accordance with AWWA/ANSI C110/A21.10.

All pipe material suppliers shall be ISO 9001 or 9002 registered or provide the services of an independent inspection agency. Prior to the start of manufacturing, any manufacturer not meeting the ISO registration requirements shall submit to Owner or Owner's engineer the name of an independent inspection agency for approval. The independent inspection agency shall be responsible for sample monitoring of chemical and mechanical tests, and sample visual inspection of quality assurance tests performed on in-process pipe and fittings, and a sample visual and dimensional inspection report from the independent inspection agency of all witnessed tests shall be supplied to Owner or Owner's engineer within 10 days of completion of pipe manufacturing.

Chemical samples shall be taken from each ladle of iron, and the manufacturers' chemical control limits shall be maintained for at least the following elements: carbon, sulfur, phosphorus, silicon, magnesium, chromium, manganese, tin, aluminum, cerium, copper, and lead. When chemical values fall outside the manufacturers' control limits, additional mechanical property tests shall be performed to assure minimum mechanical properties are met.

Where called out on the plans, restrained joint pipe and fittings shall be used. All restrained jointing systems require approval of the ENGINEER. Preapproved restraining systems include Griffin Pipe Product Co. Snap-Lok, US Pipe TR Flex, Sigma One-Lok or American Cast Iron Pipe Co. Flex-Ring. The CONTRACTOR shall note that

the standard mechanical joint is not a restrained joint and offers no practical resistance against joint separations.

901-2.5 DUCTILE IRON FITTINGS. Ductile Iron fittings shall be manufactured in accordance with AWWA/ANSI C110/A21.10 and shall be furnished with either Standardized Mechanical Joints or Push-On Joints in accordance with AWWA/ANSI C111/A21.11. Ductile Iron fittings shall be manufactured in accordance with AWWA/ANSI C153/A21.53 or AWWA/ANSI C110/A21.10. Ductile iron fittings shall have a working pressure of 350 pounds per square inch conforming to AWWA/ANSI C153/A21.53 or AWWA/ANSI C110/A21.10. All ductile iron fittings shall contain an interior and exterior bituminous seal conforming to AWWA/ANSI C104/A21.4. All ductile iron fittings shall be considered incidental to the price bid for watermain.

901-2.6 COUPLINGS. All pipe couplings 12-inch diameter and smaller shall be epoxy coated ductile iron meeting the requirements of ASTM A 536, grade 65-45-12 and AWWA C219. Couplings shall have a minimum working pressure of 250 psi and have end rings that are segmented and joined with a pinless hinge. Gaskets shall be for water and sewer as per ASTM D2000. Fasteners shall be stainless steel.

Couplings larger than 12-inch diameter shall be Romac Macro HP, Hymax High Pressure or approved equivalent. Couplings larger than 12-inch diameter shall be mechanical joint long body sleeves (min. 15" length) meeting AWWA C153.

901-2.7 GATE VALVE. The gate valve furnished shall be manufactured by American Flow Control or American AVK Company or approved equivalent, under the minimum requirements in design, material, and workmanship conforming to the latest AWWA Standard C515. The metals used shall be in accordance with AWWA and ASTM Standards. Unless otherwise designated, all gate valves shall have a non-rising stem, O-ring stem seals, 2-inch operating nuts, and open counterclockwise. If a stem extension is specified, it shall be fastened to the operating nut with a set screw. The operating nut shall be drilled or otherwise indented to accept the set screw and provide a secure connection that will prevent an extension from coming loose during operation. The gate valve shall have a resilient synthetic rubber coating seat attached to the wedge, manufactured and designed in accordance with the latest AWWA Standard C515. Resilient-seated gate valve body and bonnet shall be coated, inside and out, with a fusion bonded epoxy in accordance with AWWA C550. The waterway shall have a full unobstructed flow without recesses in the bottom. All bonnet bolts shall be stainless steel.

All valves shall be placed on a minimum 6 inch thick concrete pad of sufficient dimensions for valve size.

901-2.8 VALVE BOXES. The valve boxes furnished shall be manufactured by Tyler Pipe Model 6860, Sigma Type "G" or Star Pipe Products Cast Iron Heavy Duty Model "G" or approved equivalent, with bases and dimensions of each section to be as follows:

No. 6 round base for 16-inch and smaller gate valves.

No. 160 oval base for 24-inch or larger.
No. 6 round base for all butterfly valves.
Covers marked "Water."
Top Section 25½ inches long.
Extension pieces as required.

Valve box debris plugs as manufactured by Infact Corporation or approved equivalent shall be furnished and installed into new valve boxes.

All valve boxes shall be capable of a minimum 6 inch top adjustment in either direction, up or down, to or from, the finished grades shown in the plans.

Valve box debris plugs and valve box extension pieces required to make the above-mentioned adjustment shall be considered incidental to the price bid for “Gate Valve and Box.”

901-2.9 GATE VALVE ADAPTOR. Gate valve adaptor shall be as manufactured by Adaptor, Inc. or approved equivalent. The adaptor shall be ¼-inch steel with a UV protective coating and a ¾-inch gasket attached to the adaptor. The adaptor shall be considered incidental to the price bid for “Gate Valve and Box”.

901-2.10 HYDRANTS. Hydrants shall be manufactured in accordance with the requirements of AWWA C502. The hydrants shall be equipped with break-a-way type traffic flanges and two 2½-inch hose connections with National Standard Threads and one 4½ -inch pumper connection with National Standard Threads. All 6-inch and 8-inch hydrants shall be 5¼-inch Waterous Pacer Model WB-67-250 as manufactured by American Flow Control or 5¼-inch American Darling Model B-62-B as manufactured by American Flow Control, or approved equivalent. New fire hydrants shall have a minimum of 24 inches between the 2½-inch hose connection and the nominal ground line groove and have a bury depth of 8½ feet unless otherwise called for per plans. All metal internal moving parts below ground will be brass, Class 304 or 316 stainless steel, or have an epoxy coating as such to prevent corrosion for the life of the fire hydrant. All washers and barrel bolts below ground level shall be stainless steel. The hydrant lower rod shall be Class 304 or 316 stainless steel or have an epoxy coating as such to prevent corrosion for the life of the fire hydrant. The hydrants shall be surrounded by ½ cubic yards of subcut gravel so placed that it will readily take up all water from the drip valves. The hydrants shall be set on a concrete pad 6 inches thick and 18 inches square.

All fire hydrant leads will have a gate valve installed on the lead. The valve shall be restrained to the tee with a city-approved method. For those fire hydrant leads 4 feet or less, a special fitting such as a Foster Adapter will be acceptable. No valve shall be installed closer than 2 feet from the fire hydrant.

Fire hydrants shall be installed with a 48-inch Red FH800 American Series Fire Hydrant Marker manufactured by Flexstake, Inc. of Fort Myers, Florida, or an equivalent

approved by the ENGINEER. All costs to furnish and install marker shall be incidental to hydrant.

When a hydrant extension is required, extension shall be as required for the make and model of the hydrant per manufacturer's recommendations.

901-2.11 RESET HYDRANT. Hydrants to be reset shall be either furnished by the CITY OF BISMARCK or an existing hydrant salvaged during construction. Hydrants shall be set at the location shown on the plans. Care shall be taken by the CONTRACTOR not to damage existing watermain, connections, or valves while removing existing hydrants. Care shall also be taken not to damage the hydrant to be reset during transportation or storage by the CONTRACTOR.

The depth of earth cover over the connecting pipe shall be no less than 8 feet. The hydrants shall be surrounded by ½ cubic yards of subcut gravel so placed that it will readily take up all water from the drip valves. The hydrants shall be set on a concrete pad 6 inches thick and 18 inches square.

Reset fire hydrants shall be installed with a 48-inch Red FH800 American Series Fire Hydrant Marker manufactured by Flexstake, Inc. of Fort Myers, Florida, or an equivalent approved by the ENGINEER. All costs to furnish and install marker shall be included with the cost to reset the hydrant.

901-2.12 TAPPING SLEEVE WITH TAPPING VALVE. For pipe sizes of 6 inches to 24 inches, the tapping sleeve shall be stainless steel with a stainless steel flange and bolts and shall conform to the "Smith Blair" Type 663, "Romac" Type SST,"PowerSeal" 3480AS, or approved equivalent. For pipe sizes of 24 inches or larger, the tapping sleeve shall be epoxy- lined and coated with stainless steel bolts and shall conform to the "Smith Blair" Type 622 Split Sleeve with O-Ring Seal. The tapping valve shall conform to City of Bismarck Standard Specification 901-2.7 for Gate Valves. Tapping saddles shall be installed according to manufacturer's installation instructions. The tapping saddle bolts shall be torqued using a calibrated torque wrench with a handle at least 12 inches in length. The CONTRACTOR should be prepared to show certification of torque wrench calibration at the request of the ENGINEER.

901-2.13 CONCRETE. Concrete for pipe cradles, anchors, and thrust blocking shall conform to the requirements of Section 501.

901-2.14 BEDDING MATERIAL. The bedding material shall be defined as stated in Section 801.

901-2.15 SUBCUT GRAVEL. The subcut gravel shall be as defined in Section 801.

901-2.16 SALVAGE MATERIAL. All existing pipe, gate valves, fittings, etc., removed during construction, when requested by the ENGINEER, shall be salvaged and delivered to the City of Bismarck Water Department as directed. No extra compensation will be allowed for this work.

901-2.17 MARKING TAPE. The CONTRACTOR will be required to furnish and install marking tape located 2 feet above the top of all watermain installed under the contract. The tape shall be of the non-detectable type and shall have a minimum width of 5 inches. The tape shall be blue in color with the words "CAUTION WATER LINE BELOW" imprinted on the tape in black capital letters. The marking tape shall be equivalent to that manufactured by Presco standard grade.

Cost of marking tape and installation shall be considered incidental to other items.

901-2.18 POLYETHYLENE ENCASEMENTS. All ductile iron and cast iron pipe, valves, valve boxes, fittings, couplers, and hydrants shall be encased with 8-mil linear low-density (LLD) polyethylene film in accordance with ANSI/AWWA C105/A21.5. All encasements shall be considered incidental. Care shall be taken so as not to damage epoxy coating or painted surfaces. Damaged pipe and fittings shall be replaced at the expense of the CONTRACTOR.

901-2.19 MECHANICAL JOINT BOLT REQUIREMENTS. Bolts for mechanical joints for fittings, valves, and hydrants shall be alternated with one-half stainless steel and one-half low alloy steel. All stainless steel bolts shall be Grade 304.

901-2.20 RESTRAINED JOINT BOLT REQUIREMENTS. Bolts for restrained joints shall be stainless steel. All stainless steel bolts shall be Grade 304.

901-2.21 INSULATION BOARDS.
Polystyrene insulation board shall have a thermal conductivity of not more than 0.28 BTU per hour per square foot per degree Fahrenheit per inch of thickness as tested in accordance with ASTM C177. The insulation shall not absorb moisture to an extent greater than 2.5 percent by volume as tested in accordance with ASTM D2842. The compression strength of the insulation shall be greater than 20 psi as tested in accordance with ASTM D1621. The density of the insulation shall be between 0.9 and 1.3 pounds per cubic feet as tested in accordance with ASTM D-1622. The insulation shall be specifically designed for protection of underground utilities and shall be installed in accordance with the manufacturer's recommendations.

901-3 CONSTRUCTION REQUIREMENTS

901-3.1 EQUIPMENT. All equipment necessary and required for the proper construction of watermain shall be on the project, in first-class working condition, and approved by the ENGINEER before construction is permitted to start.

The CONTRACTOR shall provide appropriate hoisting equipment to handle the pipe while unloading and placing it in its final position without damage to the pipe.

The CONTRACTOR shall provide methods and means to obtain the required compaction of the pipe bed and the backfill, as specified.

901-3.2 EXCAVATION AND PREPARATION OF TRENCH. Excavation and preparation of the trench for watermain construction shall conform to Section 801 with the following additions:

The CONTRACTOR shall notify the City of Bismarck Fire Department of any loss of service of a fire hydrant or ability to use a fire hydrant 1 day before the occurrence. The CONTRACTOR shall also notify the City of Bismarck Fire Department when each hydrant is back in service. Any existing hydrants and valve boxes to be removed and not replaced shall be cut off 2 feet below the surface, and the void shall be filled with granular material, up to 2 feet below the surface. All hydrant heads shall be salvaged and delivered to the City of Bismarck Public Works Department at 601 South 26th Street.

Existing gate valves shall only be operated by City of Bismarck representatives. The CONTRACTOR will operate its newly installed valves until the project is accepted. Existing valves may not close tight enough to get a watertight closure. The CONTRACTOR may have to do work without a total water shut off with no extra charge to the City of Bismarck. In the event extra valves have to be shut down to slow the flow of water, there shall be no extra charge to the City of Bismarck by the CONTRACTOR for the time, up to 2 hours, to accomplish the water shutdown.

901-3.3 ROCK EXCAVATION. The rock excavation shall be as defined in Section 801.

901-3.4 PIPE LAYING. All pipe laying shall conform to Section 801 with the following additions:

Before lowering and while suspended, pipe shall be inspected for defects. Any defective, damaged, or unsound pipe shall be rejected. All foreign matter or dirt shall be removed from the inside of the pipe before it is lowered into its position in the trench, and it shall be kept clean by approved means during and after laying. Care shall be taken to prevent dirt from entering the joint space. At times when pipe laying is not in progress, the open ends of the pipe shall be closed by approved means, and no trench water shall be permitted to enter the pipe.

Cutting pipe for inserting valves, fittings, or closure pieces shall be done in a neat and workmanlike manner without damage to the pipe. Unless otherwise directed, pipe shall be laid with the bell ends facing the direction of laying. For lines on an appreciable slope, bells shall face upgrade, if directed by the ENGINEER. Whenever necessary to deflect the pipe from straight line, whether in the vertical or horizontal plane to avoid obstructions, to plumb stems, or other reasons, the degree of deflection shall not exceed manufacturer's recommendations and shall be approved by the ENGINEER. When any railroad is crossed, all precautionary construction measures required by the railroad officials shall be followed. No pipe shall be laid in water or when the trench condition or the weather is unsuitable for such work except by permission of the ENGINEER.

The CONTRACTOR shall place a 16-inch by 16-inch or larger concrete block, as directed by the ENGINEER, under all valves. A larger block will be required for larger valves. The block shall be considered incidental to the price bid for the valve.

When installing watermain 12 inches and larger, on either side of each fitting and valve, a minimum of 50 lineal feet and 2 uncut sections of pipe shall be installed with restrained joints.

Restrained joint systems require approval by the ENGINEER. Preapproved restraining systems include Certa-Lok, Yellowmine, Romac 600 Series, Ford uniflange, Sigma PV-Lok and EBAA Iron, Inc C100. (Megalug). The CONTRACTOR shall note that the standard mechanical joint is not a restrained joint and offers no practical resistance against joint separations. All nuts, bolts and threaded rod shall be stainless steel. Restraining systems shall be incidental to watermain. All restrained joint systems shall be wrapped with 8mm poly.

All bolted fittings and service saddles shall be installed according to the manufacturer's recommendations. All bolts shall be tightened with a torque wrench according to the manufacturer's recommendations. The CONTRACTOR shall have a copy of the installation guide on site.

The CONTRACTOR shall furnish and install temporary watertight plugs in any opening left in the main line or lead off the main line, during construction, which would allow water or other debris to enter the newly constructed pipe or any existing pipe.

901-3.5 TESTS. Inspection and tests must be made by the manufacturer on all pipe and component parts before shipment. Such tests shall be made by a testing laboratory satisfactory to the ENGINEER, and such tests shall be made in accordance with the requirements of the American Society for Testing Materials. Documentary evidence that the materials have been passed such inspection and tests must be furnished to the ENGINEER before the delivery of the materials on the job. Any materials which do not prove satisfactory after being placed must be removed from the premises and replaced with satisfactory material. The cost of foundry inspection shall be paid for by the CONTRACTOR. After the pipe has been laid, all new pipe, including pipe for water services or any valve section thereof, shall be subject to a hydrostatic pressure test under the supervision of the ENGINEER. The test section shall be filled with water, and the pressure shall be gradually increased. If defects are found, the CONTRACTOR shall immediately make the necessary repairs at its own expense. The final pressure test shall be 150 pounds per square inch and shall be held at least 2 hours. The CONTRACTOR shall furnish all tools, equipment, and material necessary to perform the pressure test. The CITY OF BISMARCK will provide the water for filling the pipe.

901-3.6 DISINFECTION AND BACTERIOLOGICAL TESTING. After the new mains, replacement mains, service lines, and valved extensions have been hydrostatic pressure tested, they shall be flushed at a minimum scour rate of 3.0 fps until all foreign material has been removed. Chlorination applications shall be made under supervision

of the ENGINEER in accordance with AWWA C651-14 Disinfecting Water Mains, with the exception that 5.1.1.1 Option B will not be allowed. Water shall be fed into the new line with chlorine applied in amounts to maintain a chlorine residual of 50 milligrams per liter for 24 hours or chlorine residual of 200 milligrams per liter for three hours. All valves and hydrants in the section treated shall be operated during this time in order to disinfect the appurtenance. Heavily chlorinated water should not remain in prolonged contact (maximum of 48 hours) with the watermain pipe. The chlorine shall be flushed from the main through hydrants and taps until all excess chlorine has been removed. The CONTRACTOR shall be responsible for repairing all grass, new or existing, damaged by the chlorination and flushing process. No chlorination water will be permitted in the watermain trench. The CONTRACTOR shall furnish all tools, equipment, materials, and chlorine to complete the chlorination process, incidental to other bid items. Prior to discharging chlorinated water into any drainage way, the CONTRACTOR shall obtain the permission of the ENGINEER. Taps are to be provided so at least one set of samples may be collected from every 1,200 feet of the new watermain, with one set from the end of the line and at least one set from each branch exceeding ~~50~~ 20 feet in length. If the new watermain is less than 1,200 feet but more than 400 feet the CONTRACTOR shall collect two sets of samples, with one set from the end of the line and one set from location as determined by the ENGINEER. If the new watermain is less than 400 feet, one set of samples will be acceptable.

After final flushing two consecutive sets of acceptable samples, taken at least 16 hours apart, shall be collected from the new main. Any other option will only be allowed with the approval of the ENGINEER. The CONTRACTOR or testing laboratory, in the presence of the ENGINEER, shall perform the sampling. The CONTRACTOR shall record the locations, by street, station and date, the samples were taken. Sampling shall be performed with due care to prevent contamination using sterile bottles provided by the testing laboratory. It is not recommended that samples be collected from hoses or fire hydrants. The testing of the samples shall be performed by a State of North Dakota certified testing laboratory selected by the CONTRACTOR. All samples shall be tested for bacteriological quality, chlorine residual, and shall show the absence of coliform organisms. All super chlorinated water from the disinfection of a potable distribution system shall not reach waters of the state until the total residual chlorine level has become non-detectable. Any sample result less than 0.05 mg/l will be considered “non-detectable.”

Written records of all test results shall be supplied to the ENGINEER and the CONTRACTOR by the testing laboratory as soon as possible. If trench water has entered the new main during construction or if, in the opinion of the ENGINEER, excessive quantities of dirt or debris have entered the new main, bacteriological samples shall be taken at intervals of approximately 200 feet and shall be identified by location. Samples shall be taken of water that has stood in the new main for at least 16 hours after final flushing has been completed.

The testing laboratory shall test for coliforms and e-coli using the "Colilert" or other ENGINEER approved equivalent test. The "Colilert" test is a pass/fail test that does not

quantify the amount of bacteria. Any presence of coliforms or e-coli shall qualify as a failed test.

If the initial disinfection fails to produce satisfactory bacteriological results, the new main may be reflushed and shall be resampled. If check samples also fail to produce acceptable results, the main shall be rechlorinated by the continuous-feed or slug methods of chlorination until satisfactory results are obtained.

Disinfection of any repair or short connection shall be in accordance with AWWA C651-14 Disinfecting Water Mains.

All disinfection and bacteriological testing shall be incidental to other items.

901-3.7 HANDLING PIPE AND ACCESSORIES. Pipe, fittings, valves, hydrants, and other accessories shall, unless otherwise directed, be unloaded at the point of delivery, and hauled to and distributed at the site of the project by the CONTRACTOR. They shall at all times be handled with care to avoid damage. In loading and unloading, they shall be lifted by hoists or slid or rolled on skidways in such a manner as to avoid shock. Under no circumstances shall they be dropped. Pipe handled on skidways must not be skidded or rolled against pipe already on the ground. In distributing the material at the site of the work, each piece shall be unloaded opposite or near the place where it is to be laid in the trench. Pipe shall be handled in such a manner that a minimum of damage to the coating will result. Damaged coating shall be repaired in a manner satisfactory to the ENGINEER. Pipe shall be placed on the site of work parallel with the trench alignment and with bell ends facing the direction in which the work will proceed unless otherwise directed. The interior of all pipe fittings and other accessories shall be kept free from dirt and foreign matter at all times. Valves and hydrants, before installation, shall be drained and stored in a manner that will protect them from damage by freezing.

901-3.8 BACKFILLING OF PIPE TRENCH. Excavation and preparation of the trench for watermain construction shall conform to Section 801 with the following revision:

After the pipe has been laid to line and grade, the trench shall be backfilled under and along the sides of the pipe up to 2 inches over the top of the pipe by thoroughly compacting bedding material into place so as to form a uniform bed for the pipe. See Standard Detail 900-3.

901-3.9 BACKFILL CLASSIFICATIONS. The backfill classifications shall be as defined in Section 801.

901-3.10 PROTECTING UNDERGROUND AND SURFACE STRUCTURES. Protection shall conform to Subsection 801.

901-3.11A BLOCKING HYDRANTS AND FITTINGS. All hydrants, tees, and bends 22½ degrees and larger, and tapping saddles 3 inches and larger, shall be provided with suitable reaction blocking of concrete blocks of adequate size or poured in place

concrete to prevent movement of fittings and hydrants when the pipe is under pressure. Precast concrete blocks shall be allowed for pipe sizes 12 inches and smaller. Thrust blocks for pipe sizes larger than 12 inches shall be poured in place. The blocks shall be placed in a manner acceptable to the ENGINEER and shall allow pipe and fitting joints to be accessible for repair. The concrete blocks may be poured in place if sufficient time is allowed for curing.

901-3.11B HYDRANT EXTENSIONS. Hydrant extensions shall be furnished and installed by CONTRACTOR per manufacturer's recommendations. Maximum extension length shall be three vertical feet. CONTRACTOR shall not stack multiple extensions.

901-3.12 GATE VALVE ADAPTORS. All gate valve boxes shall be installed upon the valve with the use of a gate valve adaptor. The adaptor shall be considered incidental to the price bid for "Gate Valve and Box".

901-3.13 MARKING VALVE BOX LOCATIONS. The CONTRACTOR will be required to furnish and install a steel fence post by each valve box unless directed not to by the ENGINEER. Steel fence posts to be used for valve locations shall be a "Tee" or "U" post having a minimum length of 5½ feet. The post shall be located within 2 feet from the valve box in a direction toward the street.

The cost of the steel fence post and the installation shall be considered incidental to other bid items.

901-3.14 INSULATE WATERMAIN. The CONTRACTOR shall furnish and install the insulation required to insulate the watermain as shown on the plans. The insulation shall be at least 4 inches thick by 8 feet wide centered on the watermain. The material between the top of the watermain bedding and the insulation shall consist of a concrete sand.

901-3.15 TEMPORARY WATER SUPPLY. If the CONTRACTOR elects to use a temporary water supply, the CONTRACTOR must provide a continuous water supply to the affected properties. The CONTRACTOR must use a polyethylene or PVC pipe. Rubberized garden hoses may not be used. The method and type of material shall be approved by the ENGINEER prior to setting up the temporary water supply. Any damage that may occur from the temporary water supply shall be the responsibility of the CONTRACTOR. All materials, labor, and equipment necessary to provide the temporary water supply shall be considered incidental to other bid items.

All temporary water mains and services shall be disinfected as per section 901-3.6 after installation or relocation and prior to putting into use. One water sample shall be taken per 1000 lineal feet of mainline and each individual service over 100 lineal feet at the end of a service connection after the temporary water is flushed. The sample shall show the absence of bacteria before connections are allowed. All mains and services shall be flushed prior to being put in service. No additional time or compensation shall be made for failure to pass bacteria test.

901-3.16 ABANDONED WATERMAIN. The CONTRACTOR shall plug all exposed ends of the watermain to be abandoned with concrete and remove all existing valve boxes and hydrant heads on the abandoned line, incidental to other bid items. CONTRACTOR shall confirm all existing valves to be abandoned are closed prior to removal of the box.

901-3.17 TAPPING SLEEVE WITH TAPPING VALVE. Tapping saddles with valves shall be hydrostatically pressure tested on the main prior to requesting a tap. The test shall be minimum 125 pounds per square inch for a duration of 30 minutes.

The City of Bismarck Public Works Department will tap the watermain at a charge to the CONTRACTOR. The CONTRACTOR shall be responsible for all other work connected with installation of the tapping sleeve and valve, including the necessary space around the watermain required for the tapping machine and assisting the Public Works Department in positioning the tapping machine.

901-4 MEASUREMENT AND PAYMENT

MISCELLANEOUS ITEMS.
Ductile Iron Fittings shall be considered incidental to the price bid for “(X)” Watermain.”

Air Release Valve and Manholes shall be measured and paid for under Section 1205-4.4.

Bedding Material shall be measured and paid for under Section 801-4.60.

Subcut Gravel shall be measured and paid for under Section 801-4.61.

Rock Excavation shall be measured and paid for under Section 801-4.62.

901-4.10 thru 4.25 (X)” WATERMAIN. The watermain pipe shall be measured by the linear foot (LF) through fittings and from centerline of pipe to centerline of pipe as shown in Standard Detail 900-2 and shall be paid for at the unit price bid for "(X)” Watermain" complete, in place, and accepted by the ENGINEER.

901-4.50 thru 4.69 (X)” GATE VALVE AND BOX. (X)” Gate Valve and Box shall be measured on an individual unit basis (EA) and shall be paid for at the unit price bid for "(X)” Gate Valve & Box" complete, in place, and accepted by the ENGINEER.

901-4.70 SIX INCH HYDRANT. 6” Hydrant shall be measured on an individual basis (EA) and paid for at the unit price bid for "6” Hydrant" complete, in place, and accepted by the ENGINEER.

901-4.71 EIGHT INCH HYDRANT. 8” Hydrant shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "8” Hydrant" complete, in place, and accepted by the ENGINEER.

901-4.73 thru 4.75 (X)” HYDRANT EXTENSION. (X)” Hydrant Extension shall be measured on an individual basis (EA) and paid for at the unit bid price for “(X)” Hydrant Extension” complete, in place, and accepted by the ENGINEER.

901-4.77 RESET HYDRANT. Reset Hydrant shall be measured on an individual basis (EA) and paid for at the unit price bid for "Reset Hydrant" complete, in place, and accepted by the ENGINEER.

901-4.78 INSULATE WATERMAIN. Insulate Watermain shall be measured by the linear foot (LF) of watermain to be insulated and paid for at the unit price bid for "Insulate Watermain" complete, in place, and accepted by the ENGINEER.

901-4.79 thru 4.99 (SIZE) TAPPING SLEEVE, VALVE AND BOX. (Size) Tapping Sleeve with and Valve and Box shall be measured on an individual basis (EA) and paid for at the unit bid price for “(Size) Tapping Sleeve & Valve” complete, in place, and accepted by the ENGINEER.

SECTION 910 – WATER MAIN PIPE BURSTING

910-1 DESCRIPTION

This specification shall cover the rehabilitation of the existing water mains by the pipe bursting method.

910-2 MATERIALS

910-2.1 WATER MAIN. PVC C900 water main piping shall include: C900 and C905 which shall meet all requirements as per section 901 of the City of Bismarck Construction Specifications. Pipe shall have a DR18 rating with ductile iron pipe outside diameters. Pipe shall meet the product standards of AWWA C900 and C905. Pipe compound classification shall have a minimum cell classification of 12454 as defined in ASTM D1784.

High density polyethylene (HDPE) pipe shall ~~be manufactured in accordance with PE4710 conforming to the latest ANSI/AWWA C906 and NSF/ANSI 61. For potable water applications, PE4710 compound shall conform to ASTM D3350 minimum cell classification 445574C-CC3. The minimum wall thickness of the high-density polyethylene pipe shall meet the requirements of DR 11 pipe with ductile iron pipe outside diameters. HDPE pipe shall conform to the requirements of ASTM F714 for Polyethylene (PE) Plastic Pipe.~~

~~All pipes shall be made of virgin material. No rework except that obtained from the manufacturer of the same formulation shall be used. The pipe shall be homogeneous throughout and be free of faults such as visible cracks, holes, foreign material, and blisters.~~be as per Section 901.

910-2.2 VALVES, HYDRANTS AND FITTINGS. All valves, hydrants, fittings, and related materials shall meet the requirements of section 900 of the City of Bismarck Construction Specifications.

All hydrant leads shall be PVC C900 from the hydrant to the mainline tee.

Where High-Density Polyethylene pipe is connected to mechanical joint fittings and valves, a HDPE mechanical joint adaptor shall be fused to the end of the pipe when a connection will be made to an existing pipe. Electrofusion fittings shall be PE4710 HDPE, Cell Classification of 345464C as determined by ASTM D3350-99, manufacturing standard ASTM D3261 and shall have a pressure rating equal to or greater than the pipe rating.

910-2.3 SERVICE CONNECTIONS. All water service and sanitary sewer service connection fittings and materials shall meet the requirements of section 1209 of the City of Bismarck Construction Specifications with the exception: saddles for HDPE water

main shall be mechanical saddles type Ford FS 313. No Belleville washer style mechanical saddles or electorfused saddles will be allowed.

910-3 CONSTRUCTION REQUIREMENTS

910-3.1 CONSTRUCTION MEETING. The CONTRACTOR shall attend a weekly construction update meeting, with the day and time to be determined at the preconstruction meeting, to discuss project status, upcoming work items, areas, traffic control and coordination with other projects.

910-3.2 HANDLING AND STORAGE. All pipe, fittings, and other materials shall be transported, handled, and stored as recommended by the manufacturer to prevent any damage. Any section of pipe having defects such as concentrated ridges, discoloration, excessive spot roughness, pitting, variable wall thickness, or any other defect in manufacturing shall be rejected by the ENGINEER and must be removed from the project site. If new pipe, fittings, and other materials become damaged during installation, they shall be rejected by the ENGINEER and replaced by the CONTRACTOR at his expense before proceeding further.

910-3.3 EQUIPMENT. The methods approved for rehabilitation of existing water mains are T.T. Technologies, Inc., Grundoburst Systems, Vermeer Bursting Systems, or approved equivalent. The bursting tool shall be capable of forcing its way through the existing water main by fragmenting the pipe and compressing the existing pipe into the surrounding soil and creating a void in which the new pipe, attached to the expander, can be pulled simultaneously. The unit shall maintain automatic thrust and pull back. A static unit must be capable of bursting in two directions from the same excavation.

The pipe bursting method shall be static with a minimum of 80 ton pull force. The existing pipe size and new pipe size shall be as noted on the construction plans. Any pipe bursting that introduces any lubricants into the pipe shall not be allowed.

All water main and related work, not covered in the Special Provisions shall be done in accordance with the City of Bismarck Construction Specifications.

910-3.4 PIPE FUSION. The pipe shall be assembled and joined on the site using the butt fusion method to provide a leak proof joint. Solvent cement or threaded joints will not be allowed. All equipment and procedures used shall be in strict compliance with the manufacturer's recommendations. A butt fused joint shall be true in alignment and have uniform rollback beads resulting from the use of proper temperature and pressure. The fused joint shall be watertight and have a tensile strength equal to or greater than that of the pipe.

Data loggers shall be used to record length of heating, fusing and cooling time, temperature, and pressure of each pipe joint and electrofused fitting. Resultant Data shall be submitted by the CONTRACTOR to the ENGINEER upon request. ~~Data~~

~~recording documentation of pipe fusion may be requested at times and shall be supplied by the CONTRACTOR to the ENGINEER in a printed document.~~

Fused joints shall be subject to acceptance by the ENGINEER prior to installation. All defective joints shall be cut out and replaced at no cost to the OWNER. Any section of the pipe with a gash, blister, abrasion, nick, scar, or other fault greater in depth than 10 percent of the wall thickness shall be rejected and removed from the project site. A defective area of the pipe may be cut out and the joint fused in accordance with the procedures stated above.

910-3.5 EXISTING WATER MAIN SHUTOFF. Existing water main shall not be shut off to proceed with construction until the following procedures or items are complete and in place:

a. Traffic control
b. Sidewalk closures
c. Construction and water disruption notifications to water users
d. Hydrants out of order approved by ENGINEER
e. Temporary water in place and ready for switchover
    1. Passing bacterial test results submitted to ENGINEER
f. Other procedures required for preparing water system for disruption as directed by ENGINEER

910-3.6 INSTALLATION. The CONTRACTOR shall follow all manufacturer's procedures and specifications for the water main bursting equipment being used.

Water main pull lengths, bend radii, and entrance ramps shall be as per manufacturer's specifications and shall not exceed the limitations for the pipe type being installed.

For all water main piping, manufacturer's recommendations shall be followed for sufficient time to allow the pipe to relax from thermal expansion and pull stress after being pulled into place and prior to being cut or connected.

Water main installation may not be accepted by the ENGINEER if manufacturer's recommendations are not followed.

Water mains to be burst shall be no closer than 10 ft from a live water main. If closer than 10 ft, the live water main shall be isolated at the next valve going back on the system.

The number of excavation pits shall be kept to a minimum, as necessary to perform the work efficiently. The CONTRACTOR shall consider using an area that will have to be excavated for a valve insertion, water service connection, or connections to existing water mains.

Connections to existing water mains shall be included in the price bid for water main.

Where subcut gravel is used to set up or support the pipe bursting equipment, it shall be included in the price bid for water main and there will be no extra compensation.

The invert profile of most water mains is assumed. The CONTRACTOR shall verify the existing elevations of all connections and crossings prior to making the connections. The grade of the water main shall be adjusted gradually minimizing the need for vertical bends.

Wrap all existing ductile and cast iron water main and fittings that are exposed during construction. Include within the price bid for water main.

The CONTRACTOR shall plug all exposed ends of the water main to be abandoned with concrete and remove all top sections of gate valve boxes, hydrant heads and curb stop boxes below ground level. Fill the remaining hydrant barrel or gate valve box with sand and add a minimum of 1 foot of concrete at the top of the hydrant barrel or gate valve box section and confirm that all valves to be abandoned are closed. All hydrant heads shall be scheduled and delivered to City of Bismarck, Public Works scrap metal pile at 601 S. 26th Street. This work shall be included in the price bid for water main items.

Removal of existing thrust blocking behind elbows, tees, hydrants and valves shall be included in the price bid for water main items.

The CONTRACTOR shall be responsible for any damage to utilities result from bursting or excavation operations. The CONTRACTOR shall be responsible for any damage to properties and such damage shall be repaired and the property restored to its original condition at the CONTRACTOR's expense.

The CONTRACTOR shall provide verification to the ENGINEER by means of potholing or excavation of the water main for areas that are within 12 inches of another utility or as called out on the plans. The verification at utility crossings shall be excavated to relieve loading and/or prevent damage during the bursting process. All verifications shall be included in the price bid for Surface Restoration.

The CONTRACTOR may encounter abandoned utilities while excavating. Extra time to verify and removal of the abandoned utility shall be included in the price bid for water main items.

The CONTRACTOR shall notify the ENGINEER if material of existing pipe is found to differ from what is stated in the plans.

910-3.7 BACKFILLING. Backfill shall be done in accordance with section 801 of the City of Bismarck Construction Specifications.

All imported backfill shall be Class A.

910-3.8 TESTING. The hydrostatic pressure test, disinfection and bacteriological testing shall be done in accordance with section 901 of the City of Bismarck Construction Specifications.

Pressure testing of the new water main shall be completed only after the water main has been installed and connected.

910-4 MEASUREMENT AND PAYMENT

Water main shall be paid by the Linear Foot (LF) for each size of the water main specified on the plans, regardless of pipe type, in place and accepted by the ENGINEER. Water main pay quantities are based on actual length of water main, not stationing.

Hydrants and valves shall be paid per Each in place and accepted by the ENGINEER.

All fittings, restraints, foster adaptors, and couplings shall be included in the price bid for water main.

All other items shall be paid per Section 901 of the City of Bismarck Construction Specifications or per the Special Provision for that item.

SECTION 1003 – STREETLIGHTING FEEDPOINT

1003-1 DESCRIPTION

This item consists of installation of streetlighting feedpoint, pad or pole mounted, including cabinet, cabinet foundation, relays, photocell, load center and appurtenances in accordance with these specifications and standard details at the locations shown on the plans or as directed by the ENGINEER.

1003-2 MATERIALS

1003-2.1 GENERAL. CONTRACTOR shall furnish all materials required to install streetlighting feedpoint in place as shown on the plans complete and ready for operation.

All materials and equipment furnished shall be new and shall be approved by the Underwriter's Laboratories, Inc. as conforming to its standards in every case where such a standard has been established for the item in question.

It is the intent of the plans and specifications to comply with the requirements set forth by the National Electric Code, the North Dakota State Electrical Board, the local utility company, and the ordinances established by the CITY. It shall be the responsibility of the CONTRACTOR to ensure that the above requirements are met. All electrical work outlined in this section shall be done under the supervision of a master electrician.

Should the plans or the specifications not meet these minimum established standards, either through omission or specification of equipment, material, and installation methods installation, the CONTRACTOR shall immediately notify the ENGINEER.

Requests for approved equivalents, alternates or substitutions shall be in accordance with Section 102 for all items included in this section. Equivalents, alternates or substitutions will be provided to plan holders in advance of opening of bids.

The CONTRACTOR shall submit shop drawings or product data in accordance with Section 104 for all items included in this section. Drawings or product data shall be marked with bid item designation and submitted within 30 days after contract award. No equipment shall be ordered until shop drawings and product data have been approved by the ENGINEER.

The CONTRACTOR shall keep one set of plans at the construction site as work is occurring to record any deviations from the plans and specifications including, but not limited to, relocated light units, feed points, junction boxes and changes in the cable location. This red-lined plan set shall be turned over to the CITY prior to the close of the project.

When the installation is complete and at such time as may be specified by the ENGINEER, the CONTRACTOR shall conduct an operating test for approval. The equipment shall be demonstrated to operate in accordance with the requirement of the specifications, the plans, and to the satisfaction of the ENGINEER. The CONTRACTOR shall furnish all instruments and personnel required for all tests. All test results shall be recorded. The CONTRACTOR shall be present during all tests and inspections unless so informed by the ENGINEER.

1003-2.2 FEEDPOINT ENCLOSURES.

a. Pad-mounted feedpoint enclosure having the following features:
   1. Minimum 1/8-inch aluminum, with a brushed aluminum finish, NEMA 3R with a domed roof and NEMA 3R drip shield. ETL or UL listed in accordance with UL 50.
   2. Dimensions of the enclosure: 42” W X 12” D X 51” H.
   3. Base frame: minimum 2” along each side and ¼” thick
   4. Two doors with continuous stainless steel or aluminum piano-style hinges, a neoprene gasket, and a stainless steel 3-point latch capable of being padlocked.
   5. The enclosure shall be equipped with back panel rails or unistrut such that equipment may be mounted in the cabinet with no penetrations to the exterior of the cabinet. The back panel shall be galvanized steel. All hardware shall be non-corrosive.
   6. Pre-approved manufacturers:
      - Povolny Specialties, Inver Grove Heights, MN
      - States Manufacturing, Minneapolis, MN

      Other manufacturers as approved by special provision or addendum.

b. Pole-mounted feedpoint enclosure having the following features:
   1. Minimum 1/8-inch aluminum, with a brushed aluminum finish with an exterior mounting plate, NEMA 3R. ETL or UL listed in accordance with UL 50.
   2. Dimensions of the enclosure: 30” W X 8” D X 36” H (minimum).
   3. One door with three lift off hinges, a neoprene gasket, and a stainless steel 3-point latch capable of being padlocked.
   4. The enclosure shall be equipped with back panel rails such that equipment may be mounted in the cabinet with no penetrations to exterior of the cabinet. The back panel shall be galvanized steel. All hardware shall be non-corrosive.
   5. Pre-approved manufacturers:
      - Hoffman Enclosures
      - Povolny Specialties, Inver Grove Heights, MN
      - States Manufacturing, Minneapolis, MN

      Other manufacturers as approved by special provision or addendum.

1003-2.3 LIGHTING RELAYS/CONTACTORS. Lighting relays/contactors shall be Square D-Type S as manufactured by Schneider Electric, ~~or~~ Eaton-Type CN 35 as manufactured by Cutler-Hammer or approved equivalent and shall be electrically held,

rated at 60 A, UL listed, with normally open contacts, and housed in a NEMA 1 enclosure ~~or approved equivalent~~.

1003-2.4 PANEL AND CIRCUIT BREAKERS.

a. For a pad mounted feedpoint, the electric panel shall be a single-phase load center with a NEMA 1 rated enclosure with minimum ~~12~~ 8 two-pole spaces (~~24~~ 16 single-pole circuits), rated 120/240 volt, 100-amp two-pole main breaker, copper bus, and a minimum 22,000-amp IR. The load center shall be Square D, Model ~~QO124M100~~ QO116M100 with QO breakers or equivalent.

b. For a pole mounted feedpoint, the electric panel shall be a single-phase load center with a NEMA 1 rated enclosure with minimum 6 two-pole spaces (12 single-pole circuits), rated 120/240 volt, 100-amp two-pole main breaker, copper bus, and a minimum 22,000-amp IR. The load center shall be Square D, Model QO112M100 with QO breakers or equivalent.

1003-2.5 SWITCHES AND OUTLETS.

a. A single pole test switch shall be provided to test the lights and bypass the photocell. The switch shall be mounted on a metal box with raised cover.

b. A 20 A, GFI duplex receptacle shall be provided. The outlet shall be mounted on a metal box with a raise cover.

1003-2.6 PHOTOCELL. Photocell for control of relays shall be Hubbell PBT-1, Intermatic K4021C, or approved equivalent.

1003-2.7 METER TRIM Meter trim shall be in a NEMA 3R enclosure, galvanized steel with a gray powder coat finish. Meter trim shall be ringless, UL listed, rated for 600V, 200A and shall have a lever bypass. Meter trim as manufactured by Milbank or approved equivalent.

1003-3 CONSTRUCTION REQUIREMENTS

1003-3.1 FEEDPOINT-PAD MOUNTED -TYPE I, II, III and IV.

The new pad-mounted feedpoint shall be installed per standard details 1003-1, 1003-2, 1003-4, 1003-5. Each Feedpoint-Pad Mounted Type refers to the number of 2-wire or 3-wire streetlight circuits.

The concrete to be used in the construction of the concrete foundations shall conform to the City of Bismarck Specifications, Section 600 for Sidewalks, Driveways, Curb, and Combined Curb and Gutter. Install two 8” X 4’ concrete piers, anchored with rebar as shown in the detail. The concrete pad shall be set on 12-inch of drainage aggregate subbase. Provide 1-inch chamfer around all edges.

Provide a minimum of seven 2-inch PVC stub outs through concrete base and extending a minimum of 12 inches beyond edge of the base. Point one conduit towards power company transformer and six towards direction of outgoing circuits. Provide two 1-inch conduits for ground rod conductors through the base only. Notify ENGINEER a minimum of 24 hours prior to pouring concrete base such that the form and cable entrances may be inspected. All unused conduits shall be sealed with a duct plug as manufactured by Tyco, Series JM. All used conduits shall be sealed with duct seal.

Install a concrete housekeeping pad between the feedpoint foundation and the edge of the sidewalk when sidewalks are already in place or as specified in the plans. The housekeeping pad shall run the length of the feedpoint foundation and shall be 4 inches thick and meet the requirements of City sidewalks as detailed in Section 601 of the standard specifications.

The pad-mounted feedpoint enclosure shall be anchored to the concrete foundation, at a minimum, at each corner, with minimum 3/8" diameter x 8" bolts. No more than 2" of the anchor bolt shall be exposed. Seal the external interface between all feedpoint cabinets and foundations to prevent infiltration of dust and moisture. Sealant shall be a silicone or polyurethane product rated for exterior use, readily bond to both metal and concrete, and be grey or clear in color.

Provide one lighting relay or contactor for each 3-wire streetlight circuit (2-120V) or one lighting relay or contactor for each 2-wire streetlight circuit (1-240V). Provide one 40-amp two-pole breaker for each 2-wire or 3-wire circuit, a 15-amp one-pole breaker for control circuit and a 20-amp one-pole breaker for convenience outlet. Control circuit includes installing the switch to bypass the photocell for testing of streetlight circuits. Install GFI outlet.

Provide 1-1/4" Sealtight flexible plastic~~PVC~~ conduit and min. 4"x4"~~x36~~x30" wire trough with tool-less access covers to enclose all exposed conductors between the load center, lighting contactors and the field conduits, as per standard detail 1003-1.Terminal blocks shall be provided by the CONTRACTOR for use for any connection within the feedpoint. The terminal blocks(s) shall be installed within the wire trough. Wire nuts shall not be permitted in the feedpoint or wire trough to make any connection. Terminal blocks will not be required when connections within the feedpoint are not required. The Contractor shall reserve the space for future terminal blocks in the layout of the feedpoint.

Photocell shall be mounted on the side of enclosure as per the detail; direct photocell to north or as directed by the Engineer.

Provide two 5/8-inch by 10-foot copper ground rods. The two ground rods shall be a minimum of 6 feet apart and shall be looped. Bond all circuits, relay cabinets, electric panel cabinet, enclosure, and neutral per NEC.

Install 120/240-volt single-phase service from power company transformer. Service conductors, rated at 200 amp, shall be installed in 2-inch, minimum, conduit with three

Type RHW-USE conductors sized for voltage drop and power utility company requirements.

Underground conductors to provide power to the feedpoint shall be 36 inches deep and shall be included in the cost of the feedpoint. Conduit above grade and not otherwise enclosed shall be galvanized rigid steel conduit. Rigid steel conduit ends shall be carefully reamed to provide a smooth surface for conductors. Plastic bushings shall be placed on rigid steel conduit ends. Route conduit through meter. Meter location shall be as shown on standard feedpoint detail. Size and type of incoming power conductors must meet NEC and utility company requirements. All unfused conductors within the feedpoint enclosure shall be placed in conduit.

Verify location of pad-mounted feedpoint with the utility company based on transformer location and space requirements. Notify ENGINEER if feedpoint cannot be served at the location shown on the plans.

1003-3.2 FEEDPOINT-POLE MOUNTED -TYPE I AND II. The new pole-mounted feedpoint shall be installed per standard details 1003-3, 1003-4 and 1003-5. Each Feedpoint-Pole Mounted Type refers to the number of 2-wire or 3-wire streetlight circuits.

Incoming service shall be fed from below grade unless otherwise shown on the plans. Underground conductors to provide power to the feedpoint shall be 36 inches deep and shall be included in the cost of the feedpoint. Route conduit through meter. Meter location shall be as shown on standard feedpoint detail. Size and type of incoming power conductors must meet NEC and utility company requirements. All unfused conductors within the feedpoint enclosure shall be placed in conduit.

Provide one 1½-inch galvanized rigid steel conduit stub outs for each 2-wire or 3-wire streetlight circuits and conduit as required by utility company for power conductors. Rigid steel conduit ends shall be carefully reamed to provide a smooth surface for conductors. Plastic bushings shall be placed on rigid steel conduit ends. Point one conduit towards power company transformer and all other conduits in the direction of the outgoing circuits. Provide two 1-inch conduits for ground rod conductors. All conduits shall be sealed with duct seal.

Provide one lighting relay or contactor for each 3-wire streetlight circuit (2-120V) or one lighting relay or contactor for each 2-wire streetlight circuit (1-240V).

Provide two 5/8-inch by 10-foot copper ground rod at bottom of pole as shown on detail drawings; ground enclosure and service. The two ground rods shall be a minimum of 6 feet apart and shall be looped. Bond all circuits, relay cabinets, electric panel cabinet, enclosure, and neutral per NEC.

Terminal blocks shall be provided by the CONTRACTOR for use for any connection within the feedpoint. The terminal blocks(s) shall be installed within the wire trough. Wire nuts shall not be permitted in the feedpoint or wire trough to make any connection.

Terminal blocks will not be required when connections within the feedpoint are not required. The Contractor shall reserve the space for future terminal blocks in the layout of the feedpoint.

Verify location of pole-mounted feedpoint with the utility company based on transformer location and space requirements. Notify ENGINEER if feedpoint cannot be served at the location shown on the plans.

1003-3.3 NAMEPLATES. Provide nameplates for all feedpoint cabinets consisting of letters and/or numbers, printed on a thermosetting laminated plastic consisting of melamine or phenolic core and melamine surface.

Mount nameplates on the front of the feedpoint with a combination of stainless steel or aluminum rivets and 3M adhesive similar to Type EC-847.

Provide nameplates with a black background and white letters and/or numbers unless noted otherwise. Provide one 1½-inch X 6-inch nameplate (minimum length) and one 1½-inch X 3-inch nameplate (minimum length) for each new feedpoint to label the front of the enclosure. The feedpoint number shall be as designated in standard detail 1003-5.

Provide two 1/2-inch by 1½-inch nameplates (minimum) for the test switch. The switch options shall be marked as “Test” and “Auto” with two 1/2-inch by 1½-inch nameplate(minimum).

Provide one 1-inch by 1½-inch nameplate (minimum) for each lighting relay or contactor. Label relays as shown on standard detail 1003-1. Marker as a means of labeling will not be acceptable.

1003-4 MEASUREMENT AND PAYMENT

1003-4.1 FEEDPOINT-PAD MTD-TYPE I
1003-4.2 FEEDPOINT-PAD MTD-TYPE II
1003-4.3 FEEDPOINT-PAD MTD-TYPE III
1003-4.4 FEEDPOINT-PAD MTD-TYPE IV

Pad mounted feedpoint include the following:

a. Feedpoint cabinet, lighting relays, load center, photocell, ground rods, and appurtenances as specified such that the feedpoint is complete and operational.
b. Concrete foundation pad with reinforced concrete piers
c. All conduit leading from the feedpoint to one foot beyond the edge of the concrete pad.
d. Conduits and conductors to provide utility power to the feedpoint meter, including the meter, all connections and coordination with the utility company along with any fees to provide service to the feedpoint.

Measurement for payment shall be on a per each (EA) unit basis for each complete unit installed and ready for operation.

1003-4.5 FEEDPOINT-POLE MTD-TYPE I
1003-4.6 FEEDPOINT-POLE MTD-TYPE II

Pole mounted feedpoints include the following:

a. Feedpoint cabinet, lighting relays, load center, photocell, ground rods, and appurtenances as specified such that the feedpoint is complete and operational.
b. Wood pole, mounting hardware and appurtenances to mount feedpoint cabinet, meter trim, and conduit.
c. All conduit leading from the feedpoint to one foot beyond the edge of the concrete pad.
d. Conduits and conductors to provide utility power to the feedpoint meter, including the meter, all connections and coordination with the utility company along with any fees to provide service to the feedpoint.

Measurement for payment shall be on a per each (EA) unit basis for each complete unit installed and ready for operation.

SECTION 1004 – STREETLIGHT UNITS

1004-1 DESCRIPTION

This work shall consist of the installation of streetlights, streetlight foundations and appurtenances in accordance with these specifications and standard details as shown on the plans or as directed by the ENGINEER.

1004-2 MATERIALS

1004-2.1 GENERAL. Materials to be furnished by the CONTRACTOR shall be all materials required to install roadway streetlighting in place as shown on the plans complete and ready for operation.

All materials and equipment furnished shall be new and shall be approved by the Underwriter's Laboratories, Inc. as conforming to its standards in every case where such a standard has been established for the item in question.

It is the intent of the plans and specifications to comply in every respect to the requirements set forth by the National Electric Code, the North Dakota State Electrical Board, the local utility company, and the ordinances established by the CITY, and it shall be the responsibility of the CONTRACTOR to ensure that the above requirements are met in every respect. All electrical work outlined in this section shall be done under the supervision of a master electrician.

Should the plans or the specifications not meet these minimum established standards, either through omission of equipment, material, and method of installation, or by specification of material, equipment, or installation methods, the CONTRACTOR shall immediately notify the ENGINEER.

Approved equivalents for equipment and materials in this section will be considered annually. Submittals must be received on or before the first business day after December 1 to be considered for projects for the next year. Equivalency for any luminaire will be based on the following minimum requirements based on the type of fixture specified:

a. Type III photometrics as defined by Illuminating Engineering Society (IES) per ANSI/IES RP-8-18 complete with IES files for each fixture.
b. Specified lumen output, driver current and color temperature.
c. Input watts not to exceed specifications.
d. Fixture housing requirements including general appearance, and specified features.
e. BUG rating.
f. Surge protection per ANSI C136.2~~.62.41~~ with a surge rating of 20kV/10kA.
g. UL listed or CSA Certified to UL Standards.
h. 10-year manufacturer's warranty with supporting documentation.

All submittals must contain the following information:

a. Digital IES files
b. Full product specification detailing how the product meets the ~~specifictions~~specifications per the standard specifications and details.
c. Applicable structural test data for light standards.
d. A minimum of three references on comparable installations.

Incomplete submittals will not be considered. Approved equivalents will be at the discretion of the ENGINEER and will be included in the project special provisions in advance of opening of bids.

The CONTRACTOR shall submit shop drawings or product data, in accordance with Section 104 for all items included in this section. Drawings or product data shall be marked with bid item designation and submitted within 30 days after contract award. No equipment shall be ordered until shop drawings and product data have been approved by the ENGINEER.

The CITY reserves the right to order additional light standards and/or luminaires along with the CONTRACTOR's shipment for the specific project for the following types of light units: BL, L, L1, C, C1 and DL1, as detailed in these specifications. Materials are to be billed to the CITY at the CONTRACTOR's invoice cost plus 15 percent; the CITY is exempt from paying sales tax. The CITY will be responsible for unloading and storing additional materials ordered by the CITY. The CONTRACTOR shall contact the CITY, by letter or email, prior to placing the order for light standards and luminaires. The CITY shall state quantities of additional materials, per item desired, in a letter or email addressed to the CONTRACTOR.

The CONTRACTOR shall keep one set of plans at the construction site as work is occurring to record any deviations from the plans and specifications including, but not limited to, relocated light units, feed points, junction boxes and changes in the cable location. This red-lined plan set shall be turned over to the CITY prior to the close of the project.

When the installation is complete and at such time as may be specified by the ENGINEER, the CONTRACTOR shall conduct an operating test for approval. The equipment shall be demonstrated to operate in accordance with the requirement of the specifications, the plans, and to the satisfaction of the ENGINEER. The CONTRACTOR shall furnish all instruments and personnel required for all tests. All test results shall be recorded. The CONTRACTOR shall be present during all tests and inspections unless so informed by the ENGINEER.

1004-2.2 TYPE BL STREETLIGHT UNIT

a. Type BL light standard, concrete having the following features:
    1. Precast, pre-stressed spun concrete, direct embedded, octagonal, tapered, 13’ nominal mounting height.

2. 2 7/8-inch cast aluminum tenon
3. Mix color and finish: Equivalent to Ameron color No.112-sky gray, or Stresscrete~~/Traditional Concrete~~ color salt & pepper with gloss acrylic graffiti-resistant coating.
4. Handhole opening, placed at 180º to curb side, minimum 1 5/8”X9” opening (consistent through the depth of the pole wall) with cast aluminum frame and cover with stainless steel screws.
5. Cable entrances, placed at 90º and 270º to curb side, minimum 1 5/8” X 8” (consistent through the depth of the pole wall).
6. Grounding lead to bond the pole to the grounding system.
7. Manufacturers and model number:
   Ameron SEO04SPL
   Stresscrete E16’4”-APO-G-S30-AG C/W 140(30/30)
   ~~Traditional Concrete, Inc. D413-SG-PA-3T~~
   Other manufactures and models as approved by special provision.

b. Type BL Luminaire, post top fixture having the following features:
   1. Type and style of fixture to equal to Holophane, model PTUE3 series.
   2. Heavy grade cast aluminum, non-photocell type housing with tool-less access to electrical components, integral slip-fitter for 3-inch OD tenon for mounting to the pole having a total of 6 tenon set screws, black polyester powder coat finish to meet a 5000 hour salt spray test.
   3. High CRI LEDs with IES Type III photometrics with a semi cutoff distribution glass refractor, BUG rating not to exceed B3-U3-G3.
   4. Luminaire to operate LED’s at 4000K with minimum lumen output of 7000 lumens,a maximum rated power of 60W, and a maximum driver current of 1075 mA.
   5. The fixture shall be L70 rated, have a full 10-year manufacturer’s warranty with supporting documentation, and UL Listed or CSA Certified to UL Standards.
   6. Surge protection ~~shall~~ per ANSI C136.2 with a surge rating of 20kV/10kA. ~~protect against a 22kA 8/20 microSec wave with clamping voltage of 320V and surge rating of 273J per ANSI C136.2C.62.~~
   7. Manufacturer:
      Holophane, model number per special provisions.
      Other manufactures and models as approved by special provision.

1004-2.3 TYPE L STREETLIGHT UNIT

a. Type L light standard, concrete having the following features:
   1. Precast, pre-stressed spun concrete, direct embedded, octagonal, tapered, 28-foot nominal mounting height.
   2. 6-foot galvanized steel mast arm with 4-bolt arm clamp with scroll assembly, Ameron Part No. C6ARMA.
   3. Mix color and finish: equivalent to Ameron color No.112-sky gray, or Stresscrete~~/Traditional Concrete~~ color salt & pepper with gloss acrylic graffiti-resistant coating.
   4. Bolt on cast aluminum top cap with two 3/8” X 1” cap screws.

5. Handhole opening, placed at 180º to curb side, minimum 2 1/4” X 7” opening (consistent through the depth of the pole wall) with cast aluminum frame and cover with stainless steel screws.
6. Cable entrances, placed at 90º and 270º to curb side, minimum 2 1/4” X 9” (consistent through the depth of the pole wall).
7. Grounding lead to bond the pole to the grounding system.
8. Manufacturers and model number:
   Ameron MEO-8.5-C6-Brace
   Stresscrete E330-BPO-G-S30-AG C/W
   ~~Traditional Concrete, Inc. D128-SG-PA-FI~~
   Other manufactures and models as approved by special provision.

b. Type L Luminaire, mast arm mounted fixture having the following features:
1. Heavy grade diecast aluminum, non-photocell type housing with tool-less access to electrical components, bubble level, 2” slip-fitter for horizontal mounting and 3G vibration design, gray polyester powder coat finish to meet a 5000 hour salt spray test.
2. High CRI LEDs with IES Type III photometrics, BUG rating not to exceed B2-U0-G2.
3. Luminaire shall deliver a minimum of 11,000 lumens at 4000K with a maximum rated power of 90W, a maximum driver current of 1,000 mA, and a rated life of 100,000 hours. The light engine and assembly shall have a minimum IP65 rating.
4. The fixture shall be L70 rated, have a full 10-year manufacturer’s warranty with supporting documentation and UL Listed or CSA Certified to UL Standards.
5. Surge protection per ANSI C136.2 with a surge rating of 20kV/10kA~~shall protect against a 22kA 8/20 microSec wave with clamping voltage of 320V and surge rating of 273J per ANSI C.62.41~~.
6. Manufacturer:
   American Electric Lighting or GE Current, model numbers per special provision.
   Other manufacturers and models as approved by special provision.

1004-2.4 TYPE L1 STREETLIGHT UNIT

a. Type L1 light standard, concrete having the following features:
1. Precast, pre-stressed spun concrete, direct embedded, octagonal, tapered, 28-foot nominal mounting height.
2. 6-foot galvanized steel mast arm with 4-bolt arm clamp with scroll assembly brace, Ameron Part No. C6ARMA.
3. Mix color and finish: equivalent to Ameron color No.112-sky gray, or Stresscrete~~/Traditional Concrete~~ color salt & pepper with gloss acrylic graffiti-resistant coating.
4. Bolt on cast aluminum top cap with two 3/8” X 1” cap screws.
5. Handhole opening, placed at 180º to curb side, minimum 2 1/4” X 7” opening (consistent through the depth of the pole wall) with cast aluminum frame and cover with stainless steel screws.

6. Cable entrances, placed at 90º and 270º to curb side, minimum 2 1/4” X 9” (consistent through the depth of the pole wall).
7. Grounding lead to bond the pole to the grounding system.
8. Manufacturers and model number:
   Ameron MEO-8.5-C6-Brace
   Stresscrete E330-BPO-G-S30-AG C/W
   ~~Traditional Concrete, Inc. D128-SG-PA-FI~~
   Other manufactures and models as approved by special provision.

b. Type L1 Luminaire, mast arm mounted fixture having the following features:
   1. Heavy grade diecast aluminum, non-photocell type housing with tool-less access to electrical components, bubble level, 2” slip-fitter for horizontal mounting and 3G vibration design, gray polyester powder coat finish to meet a 5000 hour salt spray test.
   2. High CRI LEDs with IES Type III photometrics, BUG rating not to exceed B3-U0-G3.
   3. Luminaire shall deliver a minimum of 18,000 lumens at 4000K with a maximum rated power of 130W, a maximum driver current of 1,000 mA, and a rated life of 100,000 hours. The light engine and assembly shall have a minimum IP65 rating.
   4. The fixture shall be L70 rated, have a full 10-year manufacturer’s warranty with supporting documentation and UL Listed or CSA Certified to UL Standards.
   5. Surge protection per ANSI C136.2 with a surge rating of 20kV/10kA~~shall protect against a 22kA 8/20 microSec wave with clamping voltage of 320V and surge rating of 273J per ANSI C.62.41~~.
   6. Manufacturer:
      American Electric Lighting or GE Current, model numbers per special provision.
      Other manufactures and models as approved by special provision.

1004-2.5 TYPE DL1 STREETLIGHT UNIT

a. Type DL1 light standards, concrete having the following features:
   1. Precast, pre-stressed spun concrete, direct embedded, octagonal, tapered, 19.5’ nominal mounting height.
   2. 2 7/8-inch cast aluminum tenon
   3. Mix color and finish: Equivalent to Ameron color No. 6P3A, or Traditional Concrete Eclipse Black with gloss acrylic graffiti-resistant coating.
   4. Handhole opening, placed at 180º to curb side, minimum ~~2 1/4~~2” X ~~8~~7” opening (consistent through the depth of the pole wall) with cast aluminum frame and cover with stainless steel screws.
   5. Cable entrances, placed at 90º and 270º to curb side, minimum 2~~ 1/4~~” X 9” (consistent through the depth of the pole wall).
   6. Grounding lead to bond the pole to the grounding system.
   7. Manufacturers and model number:
      Ameron MEO06
      Stresscrete E23’10”-BPO-G-E11-AG C/W 140(30/30)

~~Traditional Concrete, Inc. D120-EB-EA-3T~~
Other manufactures and models as approved by special provision.

b. Type DL1 luminaires, post top mounted having the following features:
   1. Type and style of fixture to equal to Holophane, model WAE3 STS series complete with ornate trim including spike finial, bands, ribs and medallions,
   2. Heavy grade cast aluminum, non-photocell type housing with tool-less access to electrical components, integral slip-fitter for 3-inch OD tenon for mounting to the pole, black polyester powder coat finish to meet a 5000 hour salt spray test.
   3. High CRI LEDs with IES Type III photometrics with a semi cutoff distribution glass refractor, BUG rating not to exceed B3-U5-G5.
   4. Luminaire to operate LED's at 4000K with a minimum lumen output of 13,300 lumens, a maximum rated power of 95W, and a maximum driver current of 525 mA.
   5. The fixture shall be L70 rated, have a full 10-year manufacturer's warranty with supporting documentation and UL Listed or CSA Certified to UL Standards.
   6. Surge protection per ANSI C136.2 with a surge rating of 20kV/10kA.~~shall protect against a 22kA 8/20 microSec wave with clamping voltage of 320V and surge rating of 273J per ANSI C.62.41~~
   7. Manufacturer:
      Holophane-Washington Postlite Series, model number per special provision.
      Other manufactures and models as approved by special provision.

1004-2.6 TYPE CL STREETLIGHT UNIT

a. Type CL light standards, galvanized steel having the following features:
   1. 40' nominal mounting height, davit type pole with a 6' mast arm and aluminum transformer base.
   2. The pole shaft, a one-piece assembly, conforming to ASTM A572 Grade 55 and davit arm conforming to ASTM A595 Grade A.
   3. Anchor bolts conforming to ASTM F1554 Grade 55. Bolts have an L bend on one end and are galvanized a minimum of 12" on the threaded end.
   4. Aluminum transformer base, nominal height 17", complete with connecting bolts, flat washers, bearing washers and hex nuts. Access door minimum 8.5" X 11". As manufactured by Akron Foundry or Valmont, type TB1-17 or approved equivalent.
   5. Reinforced handhole opening and cover, placed at 180º to curb side, minimum 4" X 6.5" and 18" above grade. A grounding lug to be provided inside handhole.
   6. Manufacturers and model number:
      Millerbernd Manufacturing, RLDA6-400ND
      Valmont Industries, Inc. DS90 RTS
      Other manufactures and models as approved by special provision.

b. Type CL Luminaire, mast arm mounted fixture having the following features:
   1. Heavy grade diecast aluminum, non-photocell type housing with tool-less access to electrical components, bubble level, 2" slip-fitter for horizontal

mounting and 3G vibration design, gray polyester powder coat finish to meet a 5000 hour salt spray test.
2. High CRI LEDs with IES Type III photometrics, BUG rating not to exceed B3-U0-G3.
3. Luminaire shall deliver a minimum of 18,000 lumens at 4000K with maximum rated power of 130W, a maximum driver current of 1,000 mA, and a rated life of 100,000 hours. The light engine and assembly shall have a minimum IP65 rating.
4. The fixture shall be L70 rated, have a full 10-year manufacturer's warranty with supporting documentation and UL Listed or CSA Certified to UL Standards.
5. Surge protection per ANSI C136.2 with a surge rating of 20kV/10kA~~shall protect against a 22kA 8/20 microSec wave with clamping voltage of 320V and surge rating of 273J per ANSI C.62.41~~.
6. Manufacturer:
   American Electric Lighting or GE Current, model numbers per special provision.
   Other manufactures and models as approved by special provision.

1004-2.7 TYPE CL1 STREETLIGHT UNIT

a. Type CL1 light standards, galvanized steel having the following features:
   1. 40' nominal mounting height, davit type pole with a 6' mast arm and aluminum transformer base.
   2. The pole shaft, a one-piece assembly, conforming to ASTM A572 Grade 55 and davit arm conforming to ASTM A595 Grade A.
   3. Anchor bolts conforming to ASTM F1554 Grade 55. Bolts have an L bend on one end and are galvanized a minimum of 12" on the threaded end.
   4. Aluminum transformer base, nominal height 17", complete with connecting bolts, flat washers, bearing washers and hex nuts. Access door minimum 8.5" X 11". As manufactured by Akron Foundry or Valmont, type TB1-17 or approved equivalent.
   5. Reinforced handhole opening and cover, placed at 180º to curb side, minimum 4" X 6.5" and 18" above grade. A grounding lug to be provided inside handhole.
   6. Manufacturers and model number:
      Millerbernd Manufacturing, RLDA6-400ND
      Valmont Industries, Inc. DS90 RTS
      Other manufactures and models as approved by special provision.

b. Type CL1 Luminaire, mast arm mounted fixture having the following features:
   1. Heavy grade diecast aluminum, non-photocell type housing with tool-less access to electrical components, bubble level, 4-bolt. 2" slip-fitter for horizontal mounting and 3G vibration design, gray polyester powder coat finish to meet a 5000 hour salt spray test.
   2. High CRI LEDs with IES Type III photometrics, BUG rating not to exceed B3-U0-G4.
   3. Luminaire shall deliver a minimum of 27,000 lumens at 4000K with maximum rated power of 200W, a maximum driver current of 1,100 mA, and a rated life

of 100,000 hours. The light engine and assembly shall have a minimum IP65 rating.

4. The fixture shall be L70 rated, have a full 10-year manufacturer's warranty with supporting documentation and UL Listed or CSA Certified to UL Standards.
5. Surge protection per ANSI C136.2 with a surge rating of 20kV/10kA~~shall protect against a 22kA 8/20 microSec wave with clamping voltage of 320V and surge rating of 273J per ANSI C.62.41~~.
6. Manufacturer:
   American Electric Lighting or GE Current, model numbers per special provision.
   Other manufactures and models as approved by special provision.

1004-2.8 WIRING SPLICE CONNECTORS AT LIGHT UNIT. Splice connectors at pole handhole shall be Penn-Union PBNA2/0X, or approved equivalent.

1004-2.9 IN-LINE FUSE AND FUSE HOLDER. The fuse holder shall be type FEB, Ferraz Shawmut, rated for 600Vac and 30 amps or approved equivalent. The fuse shall be a fast-acting midget fuse with and interrupt rating of 10kA at 600Vac, 5 amp as manufactured by Bussmann, Littelfuse or equivalent.

1004-3 CONSTRUCTION REQUIREMENTS

1004-3.1 STREETLIGHT STANDARD – GENERAL REQUIREMENTS

Streetlight standards shall be set as shown on the plans. Install 1/2-inch by 10-foot ground rod at each streetlight. All streetlight standards shall be grounded. Bond ground conductor, streetlight standard, and ground rod.

Pole wiring shall be No. 10 AWG stranded copper, Type THHN/THWN 600-volt cable, three conductors minimum (power, neutral, and ground), and shall be continuous from the luminaire to the fuse holder in the light standard base. Wire nuts shall not be permitted.

Fuse each luminaire in the base of the lighting standard at the handhole. Tape fuse kits with a 1/2-inch lapped layer for a distance of 1½ inches on each side of joint with conductor. Fuse holders to be complete with proper fuse to protect luminaire. The neutral conductor shall be solidly connected and unfused throughout system.

The fuse holder shall be supported by the conductors at the level of the hand hole. Sufficient excess conductor length shall be provided to permit withdrawal of the fuse holder through the hand hole a minimum of 6 inches outside of the hand hole for purposes of installation and inspection.

All luminaires shall be leveled after the pole is standing or in place, ready for operation. For fixtures with mast arms, mast arms to be oriented perpendicular to the face of the

curb unless otherwise noted. For post top mounted streetlight units, the fixture shall be orientated so the optics are perpendicular to the face of the curb unless otherwise noted.

1004-3.2 STREETLIGHT UNITS - DIRECT EMBEDDED (TYPES, BL, L, L1 and DL1)

A concrete-bearing base pad, 6 inches thick and a minimum 16 inches diameter, shall be provided under the bottom of the pole as shown in the standard detail. Provide roofing tar paper around poles between pole and concrete pad. In sidewalks, provide 3/4-inch expansion joint around concrete pad between concrete pad and sidewalk.

A concrete housekeeping pad of dimensions shown in the standard detail shall be constructed around the base of the direct buried concrete standard.

The concrete to be used in the construction of the concrete housekeeping pads and bearing base pads shall be a minimum of 4,000 psi strength at 28 days with a minimum of six bags of cement per cubic yard of concrete and shall conform in all respects to the City of Bismarck Specifications, Section 600 for Sidewalks, Driveways, Curb, and Combined Curb and Gutter.

1004-3.3 STREETLIGHT UNITS WITH CONCRETE FOUNDATIONS (TYPES CL and CL1)

Concrete foundations shall be installed as per standard detail. Foundations shall be completed with anchor bolts, rebar, and conduit stub-in. Anchor bolt spacing to accommodate poles shall be verified in the field prior to construction.

The concrete to be used in the construction of the concrete foundations shall be a minimum of 4,000 psi strength at 28 days with a minimum of six bags of cement per cubic yard of concrete and shall conform in all respects to the City of Bismarck Specifications, Section 600 for Sidewalks, Driveways, Curb, and Combined Curb and Gutter.

The CONTRACTOR shall notify the ENGINEER at least 24 hours prior to pouring a concrete foundation such that the form with the anchor bolt placement, rebar, conduit stub-ins, and ground rod can be inspected. The CONTRACTOR shall provide concrete tests in conformance with City of Bismarck Specifications, a minimum of one test per day or a minimum of one test per five light standard foundations or as directed by the ENGINEER.

1004-3.4 REMOVE STREETLIGHT STANDARD. The standard shall be removed from the site shown on the plans, salvaged, transported, and stored (by blocking and supporting at three points) in the CITY storage yard located at the Municipal Solid Waste Facility on North 52nd Street. Where plans indicate that the light unit shall not be salvage, the CONTRACTOR will be responsible to dispose of the light unit.

The luminaire wiring within the pole shall be disconnected at the fuse. The luminaire shall be removed from the mast arm, salvaged, and delivered to CITY storage at 601 South 26th Street.

Where the plans call for salvaging the conductors and re-splicing the underground conductors, the standards shall be removed carefully to prevent damage to the conductors. Splices shall be made as detailed in these specifications.

The hole where the standard was removed shall be filled with earth supplied by the CONTRACTOR and tamped to the density of the surrounding soil. The surface shall be returned to that of the adjoining area.

1004-3.5 RELOCATE STREETLIGHT STANDARD. This item shall consist of removing a light unit from its present location and reinstalling at either the same location or a new location as shown on the plans including reconnecting to the new or existing streetlight system conductors as indicated in the plans.

The standard shall be removed, salvaged, transported, and stored (by blocking and supporting at three points) in the CITY storage yard located at the Municipal Solid Waste Facility on North 52nd Street or at a location approved by the ENGINEER. The CONTRACTOR shall take care in the removal, relocation and storage of the light standard to prevent damage to the standard or luminaire. Any damage that occurs to the light as part of the relocation will be the responsibility of the CONTRACTOR.

Where the plan calls for relocating the pole, the CONTRACTOR shall fill the existing hole with earth supplied by the CONTRACTOR and tamped to a density of the surrounding soil. The surface shall be returned to that of the adjoining area.

1004-4 MEASUREMENT AND PAYMENT

1004-4.1 TYPE BL STREETLIGHT UNIT
1004-4.2 TYPE DL1 STREETLIGHT UNIT

Type BL and DL1 consists of the following:

a. Designated LED luminaire.
b. Concrete direct buried poles.
c. Wiring and connections to underground circuits.
d. Ground rod with connection.
e. Fuse holder, fuses and internal wiring.
f. Concrete housekeeping pads and concrete-bearing base pads.
g. Unit set in place, ready for operation.

Measurement for payment shall be on a per each unit basis (EA) for each complete unit installed and ready for operation.

1004-4.3 TYPE L STREETLIGHT UNIT
1004-4.4 TYPE L1 STREETLIGHT UNIT

Type L and L1 consists of the following:

a. Designated LED luminaire.
b. Concrete direct buried poles with bracket (mast arm).
c. Wiring and connections to underground circuits.
d. Ground rod with connection.
e. Fuse holder, fuses and internal wiring.
f. Concrete housekeeping pads and concrete-bearing base pads.
g. Unit set in place, ready for operation.

Measurement for payment shall be on a per each unit basis (EA) for each complete unit installed and ready for operation.

1004-4.5 TYPE CL STREETLIGHT UNIT
1004-4.6 TYPE CL1 STREETLIGHT UNIT

Type CL and CL1 Streetlight Units consist of:

a. Designated LED luminaire.
b. Galvanized steel, davit type anchor base pole with mast arm.
c. Wiring and connections to underground circuits.
d. Fuse holder, fuses and internal wiring.
e. Reinforced concrete base, anchor bolts, anchor bolt covers, ground rod, and conduit.
f. Unit set in place and ready for operation.

Measurement for payment shall be on a per each unit basis (EA) for each complete unit installed and ready for operation.

1004-4.7 REMOVE STREETLIGHT STANDARD. This item shall consist of removal, transport, storage or disposal of streetlight standards and restoration of the ground to that of the surrounding area.

Measurement for payment shall be per each (EA) street light standard removed and accepted by the ENGINEER.

1004-4.8 RELOCATE STREETLIGHT STANDARD. This item shall consist of removal, transport, storage, relocating/resetting and reconnecting conductors as required. Where Plans call for the relocation of a type CL or CL1 streetlight unit, the cost of the concrete foundation will be paid under 1004-4.9 Streetlight Base.

Measurement for payment shall be per each (EA) streetlight standard removed, stored, relocated, reconnected and accepted by the ENGINEER.

1004-4.9 STREETLIGHT BASE. This item consists of constructing concrete base per specification and standard details for type CL or CL1 streetlight units.

Streetlight base shall be measured per each (EA) and paid for at the unit price bid for streetlight base complete, in place, and accepted by the ENGINEER.

SECTION 1205 – MANHOLES AND INLETS

1205-1 DESCRIPTION

These items shall consist of the construction or installation of manholes and inlets, in accordance with these specifications, at the specified locations and Standard Details and conforming to the lines, grades, and dimensions shown on the plans or required by the ENGINEER.

1205-2 MATERIALS

1205-2.1 CONCRETE. Plain and reinforced concrete used in this work shall conform to the requirements of Section 501 "Portland Cement Concrete Pavement."

1205-2.2 MORTAR. Mortar shall be a compound of 1 portland cement to 2 parts of sand by volume to which lime may be added not to exceed 10 percent of the cement by weight.

1205-2.3 PRECAST REINFORCED CONCRETE PIPE MANHOLE. Precast reinforced concrete manhole risers and top sections shall conform to ASTM C478.

All barrel-to-barrel joints shall be sealed using a P2 gasketed joint for 48-inch manholes, a CX-4 joint for all other sizes of manholes, and an exterior seal by Press-Seal Gasket Corporation EZ Wrap and EZ Stik No. 4 primer, for all sizes of manholes, or approved equivalent. The height of the manhole shall be shown on the plans, and the diameter shall be 48 inches minimum or larger if recommended by the manhole fabricator and approved by the ENGINEER.

Steps shall not be placed in sanitary sewer, storm sewer, or air release manholes or inlets unless specified. If specified, the manhole steps to be furnished and installed shall be rubber-coated over steel reinforcing of the type manufactured by the Delta Products (Delta-Surefoot Company), or approved equivalent.

1205-2.4 PRECAST REINFORCED CONCRETE PIPE MANHOLE WITH MONOLITHIC BASE. Precast reinforced concrete risers and top sections shall conform to ASTM C478. Manhole bases shall extend a minimum of 6 inches past the exterior manhole wall, except bases below M.S.L. elevation 1626 (NGVD29) or manholes 18 feet (from rim to top of pipe) or greater in depth, which shall extend 12 inches past the exterior manhole wall. The base and the bottom section shall be cast monolithically with precast flow lines.

The pipe connections to the manhole shall be sealed with Press-Seal Gasket Corporation Model PSX-Boot with 2 stainless steel bands or approved equivalent. Connection boots and bands are not required on storm sewer manholes. All barrel-to-barrel joints shall be sealed using a P2 gasketed joint for 48-inch manholes, a CX-4 joint

for all other sizes of manholes, and an exterior seal by Press-Seal Gasket Corporation EZ Wrap and EZ Stik No. 4 primer, Infi-Shield External Gator Wrap for all sizes of manholes, or approved equivalent. All barrel sections below M.S.L. elevation 1626 (NGVD29) or 18 feet or greater in depth (from rim to top of pipe) shall be restrained using three outside pipe joint ties equally spaced or approved equivalent. The height of the manhole shall be shown on the Plans, and the diameter shall be 48 inches minimum or larger if recommended by the manhole fabricator and approved by the ENGINEER.

Steps shall not be placed in sanitary sewer, storm sewer, or air release manholes or inlets unless specified. If specified, the manhole steps to be furnished and installed shall be rubber-coated over steel reinforcing of the type manufactured by the Delta Products (Delta-Surefoot Company), or approved equivalent.

1205-2.5 MANHOLE CASTINGS. Materials shall conform to the requirements of Section 1206.

~~(a) Sanitary Sewer, Storm Sewer, and Water Main Manhole Castings. Manhole frames and covers shall be of the type manufactured by the Neenah Foundry Company Number R-1733, East Jordan Iron Works Number 1205, or Municipal Castings, Inc. Number 301 with concealed pick holes and self-sealing platen lid, or approved equivalent.~~

~~(b) Sanitary Sewer, Storm Sewer, and Water Main Floating Manhole Castings. Floating manhole frames and covers shall be of the type manufactured by Neenah Foundry Company Number R-1955-1 or East Jordan Iron Works Number 3025 with concealed pick holes and self-sealing platen lid, or approved equivalent. See Standard Details 1206-3 and 1206-4.~~

1205-2.6 INLET CASTINGS. Materials shall conform to the requirements of Section 1206. ~~Inlet castings shall be of the type manufactured by Neenah Foundry Company with Type V Grates and NDDOT Style Backs, East Jordan Iron Works with M6 Type Grate and Type T2 Back for Type 24-inch and with type M4 Grate and Type T5 Back for Type 36-inch or larger, or approved equivalent. All bolts to be temper finish, double heat-treated 1038 S.A.E., Grade 5, Cad-Dichromate plated.~~

~~(a) Type 24" Castings. Type 24-Inch Castings shall be Neenah Foundry Number R-3030, East Jordan Iron Works Number 7010 with round base, or approved equivalent.~~

~~(b) Type 36" Castings. Type 36-Inch Inlet Castings shall be a Neenah Foundry Number R-3295, East Jordan Iron Works Number 7030, or approved equivalent.~~

~~(c) Type 72" Castings. Type 72-Inch Castings shall be Neenah Foundry Number R-3295-2, East Jordan Iron Works Number 7031, or approved equivalent.~~

~~(d) Type 108" or Larger Castings. Type 108-Inch or Larger Castings shall be Neenah Foundry Number R-3295-3, or East Jordan Iron Works Number 7032 with additional inner sections, or approved equivalent.~~

~~(e) Catch Basin Castings. Catch Basin Castings shall be Neenah Foundry Number R-2573 with concave grate, Neenah Foundry Number R-2573 with "beehive" grate, or approved equivalent.~~

1205-2.7 SLOTTED DRAIN INLET. Slotted drain inlets shall be of a quality equal to the type manufactured by Contech Construction Products with the modified hugger band under the minimum requirements in design, material, and workmanship conforming to the latest standards of AASHTO M36 and AASHTO M111.

1205-2.8 REINFORCING STEEL. Reinforcing steel used in this work shall conform to Section 501.

1205-2.9 AIR RELEASE VALVE. All air release valve taps, made into all sizes and classes of PVC and ductile iron water mains, shall be reinforced with a tapping saddle. Tapping saddles shall be a minimum of 2-bolt stainless steel skirted or complete gasket type. An O-ring single bolt stainless steel saddle is not acceptable.

The automatic air release valve shall be a ~~1~~2-inch APCO No. 200, ~~or~~ Valmatic Model 38, or H-TEC Model 986-01, or approved equivalent for water. Connection type shall be threaded FNTP.

The automatic air release valve shall be a 2-inch H-TEC Model 986-01 or approved equivalent for sewer. Connection type shall be threaded FNTP.

~~and APCO No. 400 or Valmatic Model 48 for sewer, or approved equivalent.~~ The corporation stop shall be a Mueller No. H-15000 for copper water pipe, or approved equivalent. Any fittings required for connection to the outlet pipe shall be brass or stainless steel. See Standard Detail 1205-11.

1205-2.10 PRECAST REINFORCED CONCRETE MANHOLE BASES. Precast reinforced concrete manhole bases shall conform to ASTM C478. The bases shall extend a minimum of 6 inches past the exterior manhole wall, except bases below M.S.L. elevation 1626 (NGVD29), which shall extend 12 inches past the exterior manhole wall. Base thickness shall be as follows: Manholes with inside diameters up to and including 48 inch – 6 inch thick, 54 inch through 102 inch – 8 inch thick, 108 inch and 120 inch – 12 inch thick. Precast air release manhole bases shall be 2 inches thicker than the base thicknesses listed above.

1205-2.11 PRECAST REINFORCED CONCRETE MANHOLE COVERS. Precast reinforced concrete manhole covers shall conform to ASTM C478. Cover thickness shall be as follows: Manholes with inside diameters up to and including 48 inch – 6 inch thick, 54 inch through 102 inch- 8 inch thick, and 108 inch and 120 inch – 12 inch thick.

1205-2.12 CONCRETE ADJUSTMENT RINGS. Concrete adjustment rings shall have a minimum compressive strength of 4000 psi, steel reinforced and meet AASHTO H-20 loading requirements.

Concrete adjusting rings shall have an internal dimension that does not restrict the opening size of the manhole or inlet casting.

Rings shall be 2 inches thick unless otherwise approved by Engineer.

1205-2.13 POLYMER ADJUSTMENT RINGS. Polymer adjustment rings shall be injection molded High Density Polyethylene (HDPE) as manufactured by Ladtech, Inc or IPEX Inc conforming to ASTM D4976, Expanded Polypropylene as manufactured by Cretex Specialty Products, or approved equal.

Polymer adjusting rings shall have an internal dimension that does not restrict the opening size of the manhole or inlet casting.

Rings shall be 2 to 6 inches thick, cross slope adjusting rings may be allowed with approval by Engineer.

1205-3 CONSTRUCTION REQUIREMENTS.

1205-3.1 EXCAVATION. Excavation for catch basins, manholes, inlets, and pipe junctions shall be done in a manner to provide adequate room for the construction of the item according to details shown on the plans. When necessary the excavation shall be adequately shored or sheeted to ensure safe and satisfactory construction and backfilling.

1205-3.2 PRECAST REINFORCED CONCRETE PIPE MANHOLES AND INLETS. Unless otherwise specified, standard reinforced concrete sewer pipe shall be used for this purpose. When this type of construction is used, the bottom precast section shall be set in a full mortar bed, and the joints between sections and around pipes shall be filled with mortar.

1205-3.3 CONCRETE CONSTRUCTION (CAST IN PLACE). The composition, consistency, placing, form work, curing, and protection of the concrete shall conform to the plans. No finishing of the concrete will be required except the filling of honeycombed areas.

1205-3.4 CONCRETE BASE. The bottoms of all manholes and inlets shall be of concrete. The thickness and other dimensions of the base shall be as specified on the plans. The invert channel shall be the true shape of the lower half of the pipe or sewer. Pipe or tile placed in concrete for inlet or outlet connections shall extend through the walls a sufficient distance to allow for connections, and the concrete shall be carefully constructed around them so as to prevent leakage along their outer surface. The inside ends shall be flush with the inside walls, and the pipe shall be of the same size and kind as those with which they connect on the outside.

1205-3.5 CASTINGS. The methods of construction shall conform insofar as applicable to the requirements of Section 1206.

~~All manhole, inlet, and catch basin castings shall be placed with a minimum of 1/2 inch of grout between the manhole inlet or catch basin, but not adjusted to grade unless specified on the plans. Total allowance for adjustment shall be from 0 to 6 inches. Castings requiring adjustment to grade shall be paid for under Section 1206 "Castings and Adjustment." All inlet castings shall be placed on the inlet facing the roadway with bolts, washers, and nuts installed in accordance with Standard Detail 1206-1.~~

~~All new and existing manholes located in concrete pavement surfaces shall have floating manhole castings installed as shown in standard details 1206-3 and 1206-4.~~

1205-3.6 SLOTTED DRAIN INLET. Slotted drain inlets shall be constructed in compliance with Standard Details 1205-9 and 1205-10. The CONTRACTOR shall furnish all equipment, labor, and materials, including the connection to the inlet or manhole, flowable fill for bedding and curb and gutter, all of which shall be considered incidental to the price for slotted drain.

1205-3.7 BACKFILL. Backfill shall be deposited in horizontal layers not over 6 inches in depth (loose) and each layer compacted, this process being repeated to the elevation of the finished grade as designated on the plans. Compaction shall be secured by watering each layer if dry (the water content of the material used shall not exceed the optimum moisture content) and tamping with approved mechanical rammers. The backfill shall be compacted to a density equal to the requirements specified for the pipe trench common to the manhole or inlet.

1205-3.8 CLEANING. All manholes and inlets shall be thoroughly cleaned of any accumulation of silt, debris, or foreign matter of any kind, and shall be free from such accumulations at the time of the final inspection.

1205-3.9 MARKING MANHOLES AND INLETS. The CONTRACTOR will be required to furnish and install a "Tee" or "U" steel fence post having a minimum length of 5½ feet located 1 foot from the edge of the casting on a line perpendicular to the face of the curb.

The cost of the steel fence post and installation shall be considered incidental to other bid items.

1205-3.10 POLYVINYL CHLORIDE PIPE TO MANHOLE ADAPTERS. All connections shall be made using manhole connector (rubber boot) or a PVC to MH adapter (sand collar) in the wall of any manhole connected to PVC sewer pipe. Manhole connectors shall be PSX Press Boot as manufactured by Press-Seal Gasket Corporation, or approved equivalent, and shall be connected with 2 pipe bands.

The cost of the PVC manhole adaptor and the installation shall be considered incidental to the bid item for "Concrete Manhole."

1205-3.11 INLET STAKES. Section 104 “Monuments, Bench Marks, Witness and Grade Stakes” shall be strictly followed. The same line and grade stakes set by the ENGINEER for the construction of each inlet shall be saved and used by the CONTRACTOR to set the inlet castings to line and grade.

1205-4 MEASUREMENT AND PAYMENT

1205-4.1 CONCRETE MANHOLE. Concrete Manhole, including casting, shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Concrete Manhole" complete, in place, and accepted by the ENGINEER.

1205-4.2 CONCRETE MANHOLE WITH MONOLITHIC BASE. Concrete Manholes with Monolithic Base, including casting, shall be measured on an individual basis (EA) and paid for at the unit price bid for "Concrete MH w/Monolithic Base" complete, in place, and accepted by the ENGINEER.

1205-4.3 CONCRETE DROP MANHOLE. Concrete Drop Manhole, including casting, shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Concrete Drop Manhole" complete, in place, and accepted by the ENGINEER. This item to include all items shown on Standard Detail 1205-2.

1205-4.4 AIR RELEASE VALVE AND MANHOLE. Air Release Valve and Manhole, including casting, shall be measured as a complete unit on an individual unit basis (EA) and paid for at the unit price bid for "Air Release Valve & Manhole" complete, in place, and accepted by the ENGINEER. This item to include all items shown on Standard Detail 1205-11.

1205-4.5 TYPE 24" INLET. Type 24" Inlet, including casting, shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Type 24" Inlet" complete, in place, and accepted by the ENGINEER. This item to include all items shown on Standard Detail 1205-5.

1205-4.6 TYPE 24” INLET/MANHOLE. Type 24-Inch Inlet/Manhole, including casting, shall be measured on an individual unit basis (EA) and be paid for at the unit price bid for "Type 24” Inlet/Manhole" complete, in place, and accepted by the ENGINEER. This item to include all items shown on Standard Detail 1205-5.

1205-4.7 TYPE 36” INLET. Type 36-Inch Inlet, including casting, shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Type 36” Inlet" complete, in place, and accepted by the ENGINEER. This item to include all items shown on Standard Detail 1205-6 and 1205-7.

1205-4.8 TYPE 72” INLET. Type 72-Inch Inlet, including casting, shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Type 72” Inlet" complete in place and accepted by the ENGINEER. This item to include all items shown on Standard Detail 1205-6 and 1205-7.

1205-4.9 TYPE 108” OR LARGER INLET. Type 108-Inch or Larger Inlet, including casting, shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Type 108” or Larger Inlet" complete, in place, and accepted by the ENGINEER.

1205-4.10 CATCH BASIN. Catch Basins including castings shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Catch Basin" complete, in place, and accepted by the ENGINEER.

1205-4.11 REMOVE EXISTING CATCH BASIN OR INLET. Remove Existing Catch Basin or Inlet shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Remove Catch Basin or Inlet" complete and accepted by the ENGINEER. This item is to include removing entire concrete structure and backfilling excavation with bedding material. The casting shall be taken to the salvage yard at the City of Bismarck Department of Public Works at 601 South 26th Street.

1205-4.12 thru 4.18 (X)” SLOTTED DRAIN. (X)-Inch Slotted Drain shall be measured by the linear foot (LF) basis for "(X)” Slotted Drain" complete, in place, and accepted by the ENGINEER. This item to include all items shown on Standard Detail, 1205-9 and 1205-10.

SECTION 1206 – CASTINGS AND ADJUSTMENT

1206-1 DESCRIPTION

This item shall consist of furnishing and adjusting new castings on existing manholes in accordance with this section, the details and plans at the locations indicated on the plans, or as directed by the ENGINEER. This item includes adjusting new castings installed within the same project.

This item shall include the resetting of manhole frames and covers, inlet frames and covers, City water works valve boxes, or other accessories requiring adjustment to new lines and grades where such accessories are public property. Unless otherwise indicated on the plans, adjustments, replacements, and repairs to private property shall be exempt from this item.

This item shall include the furnishing of new castings, grating, I & I barriers, or covers specifically indicated on the plans. The CONTRACTOR, however, will be required to replace, at its own expense, any damaged parts resulting from its operations.

1206-2 MATERIALS

1206-2.1 Materials shall conform insofar as applicable to the requirements of Section 1205.

1206-2.2 MANHOLE CASTINGS. Sanitary Sewer, Storm Sewer, and Water Main Manhole Castings shall conform to the following:

(a) MANHOLE CASTINGS. Manhole frames and covers shall be of the type manufactured by the Neenah Foundry Company Number R-1733, East Jordan Iron Works Number 1205, or Municipal Castings, Inc. Number 301 with concealed pick holes and self-sealing platen lid, or approved equivalent.

(b) FLOATING MANHOLE CASTINGS. Floating manhole frames and covers shall be of the type manufactured by Neenah Foundry Company Number R-1955-1 or East Jordan Iron Works Number 3025 with concealed pick holes and self-sealing platen lid, or approved equivalent. See Standard Details 1206-3 and 1206-4.

Type (b) shall be used with all new and existing manholes located in concrete pavement surfaces unless noted in plans, type (a) shall be used in all other instances unless noted in plans.

Manhole lids shall be labeled, with applicable utility type, “SANITARY SEWER”, “STORM SEWER” or “WATER” in two-inch raised lettering.

1205-2.3 INLET CASTINGS. Inlet castings shall be of the type manufactured by Neenah Foundry Company with Type V Grates and NDDOT Style Backs, East Jordan Iron Works with M6 Type Grate and Type T2 Back for Type 24-inch and with type M4 Grate and Type T5 Back for Type 36-inch or larger, or approved equivalent. All bolts to be temper finish, double heat-treated 1038 S.A.E., Grade 5, Cad-Dichromate plated.

(a) Type 24" Castings. Type 24-Inch Castings shall be Neenah Foundry Number R-3030, East Jordan Iron Works Number 7010 with round base, or approved equivalent.

(b) Type 36" Castings. Type 36-Inch Inlet Castings shall be a Neenah Foundry Number R-3295, East Jordan Iron Works Number 7030, or approved equivalent.

(c) Type 72" Castings. Type 72-Inch Castings shall be Neenah Foundry Number R-3295-2, East Jordan Iron Works Number 7031, or approved equivalent.

(d) Type 108" or Larger Castings. Type 108-Inch or Larger Castings shall be Neenah Foundry Number R-3295-3, or East Jordan Iron Works Number 7032 with additional inner sections, or approved equivalent.

(e) Catch Basin Castings. Catch Basin Castings shall be Neenah Foundry Number R-2573 with concave grate, Neenah Foundry Number R-2573 with "beehive" grate, or approved equivalent.

1206-2.42 Flexible foam expansion joint materials shall meet the requirements of ASTM D5249, TYPE 2, and ASTM D1752, Sections 5.1 through 5.4 with the compression required modified to 10 psi and 25 psi maximum. This material shall be non-gassing and shall be compatible with hot pour joint sealants.

1206-3 CONSTRUCTION REQUIREMENTS

1206-3.1 GENERAL. The methods of construction shall conform insofar as applicable to the requirements of Section 1205.

Existing manholes, inlets, and valve boxes shall be adjusted to the elevation, grade, or dimensions as indicated on the plans and standard details, or as ordered by the ENGINEER. The structures are assumed to be clean prior to the beginning of the adjustment construction unless otherwise agreed to by the CONTRACTOR and the ENGINEER. Castings shall be carefully removed and reinstalled by the CONTRACTOR as indicated. If the height of a rectangular casting is to be increased, the addition may be of solid concrete block or concrete as specified in Section 501. Solid concrete block shall not be used to increase the height of circular castings. In the event the top part of the existing structure is weak and faulty, it shall be replaced as directed by the ENGINEER and the extension completed. Where the casting, grating, I & I barrier, or cover is to be lowered, the masonry or concrete shall be removed to sufficient depth so that a seat of proper dimensions may be reconstructed to receive the casting, grating, I & I barrier, or cover at the new grade. Castings shall be set in full mortar beds or

otherwise secured as shown on the plans. Mortar shall be in accordance with Section 1205. Casting shall be set accurately to correct elevation and line so that no subsequent adjustment will be necessary. If necessary, the CONTRACTOR shall use tapered or sloped adjusting risers.

Upon completion of the adjustment, all surplus material shall be removed, and the structure and the site of the work shall be left in a neat and clean condition. The entire structure shall be thoroughly cleaned of any accumulation of silt, debris, or foreign matter of any kind and shall be free from such accumulations at the time of final inspection.

1206-3.2 WATER STOP BOX EXTENSION. Water service stop boxes are found within the area of construction very frequently. Adjustments in elevation that can be accomplished within the range of the adjustment sleeve of the stop box shall be considered incidental to the contract bid items. The CONTRACTOR is required to use due care in making these adjustments.

If the stop box cannot be extended to the proper grade within the above limits, it shall be adjusted by removing the lid and adding the required length and diameter of standard weight pipe with a standard pipe coupling and replacing the lid. The adjustment shall be paid under Bid Item ”Water Stop Box Extension”.

1206-3.3 WRAPPED UTILITY BOXES. Utility structures, excluding manholes, encased in concrete sidewalks and pavements, shall be wrapped with a flexible foam expansion joint. Wrapped structures include valve boxes, hydrants, curb stop boxes, street light poles and foundations, traffic signal foundations, pedestrian signal pole foundations, and street signs.

Minimum thickness of the flexible joint will be 1/2 inch used on curb stop boxes, hydrants, street signs, pedestrian signal foundations, and valve boxes. Minimum thickness for larger structures shall be 3/4 inch to 1 inch maximum. Wrapped utility boxes shall be considered incidental.

1206-3.4 CASTINGS. All manhole, inlet, and catch basin castings shall be placed with a minimum of 1/2 inch of grout between the manhole inlet or catch basin, but not adjusted to grade unless specified on the plans. Total allowance for adjustment shall be from 0 to 6 inches. Castings requiring adjustment to grade shall be paid for under Section 1206 "Castings and Adjustment." All inlet castings shall be placed on the inlet facing the roadway with bolts, washers, and nuts installed in accordance with Standard Detail 1206-1.

1206-3.5 FLOATING MANHOLE CASTINGS. All new and existing manholes located in concrete pavement surfaces shall have floating manhole castings installed as shown in standard details 1206-3 and 1206-4.

1206-3.6 ADJUSTING MANHOLE CASTING WITH RING AND I/I BARRIER. I/I barriers shall be installed on all new manholes outside of pavement areas. Manholes

installed in future paved areas shall have I/I barriers installed unless the roadway is scheduled to be paved within the same construction season. I/I barriers will not be required when casting, all rings and top of cone are encased in concrete and located in a paved roadway, including roadways scheduled to be paved in the same construction season.

All newly installed manholes requiring adjustment under this bid item shall use the precast concrete adjusting rings with the AP/M PERMAFORM I/I BARRIER as manufactured by Strike Products, or approved equivalent, installed and field tested according to the manufacturers' specifications. This item shall be paid for under Section 1206 "Castings and Adjustment." All inlet castings shall be placed on the inlet facing the roadway with bolts, washers, and nuts installed in accordance with Standard Detail 1206-1.

The I/I Barrier shall have a watertight seal to the top of manhole barrel using butyl sealant, or approved equivalent, as specified by the manufacturer of the I/I Barrier. The top of manhole barrel shall be free of dust and debris before applying the sealant and the I/I Barrier. Sufficient quantity of sealant must be used to accommodate any flaws in the top of manhole barrel.

If deemed necessary by the ENGINEER, to check the seal of the I/I Barrier, the excavated area around the I/I Barrier shall be filled with water to a level above the joint between the I/I Barrier and the top of manhole barrel. If any leakage or moisture is present in the area inside the manhole around the seal, the CONTRACTOR shall remove the I/I Barrier and reseal at no additional cost.

The bottom ring placed on the I/I Barrier shall not be sealed to the I/I Barrier to allow infiltrated water to escape. All successive rings above the bottom ring shall be sealed together per manufacturer's recommendations. The I/I Barrier shall extend a minimum of 2 inches above the top ring. If a floating manhole casting is used, the I/I Barrier extending above the top ring shall be trimmed so that the I/I Barrier does not interfere with the manhole casting's ability to function. All pressure-reducing valve and air release valve manholes shall include a Cap 'N Seal as manufactured by Strike Products.

All existing manholes outside the roadway surface that require adjustment shall be paid for under Section 1206-4.17 "Adjust Manhole Casting in Unpaved Area." All existing manholes outside the roadway surface that require a new manhole casting shall have standard castings and shall be paid for under Section 1206-4.18 "Furnish and Adjust Manhole Casting in Unpaved Area."

1206-3.~~6~~ 7 CASTING ADJUSTMENTS. All manhole and inlet castings shall be adjusted to final grade with concrete or poly adjustment rings. Rings shall be installed per manufacturer's recommendations and City of Bismarck Standard Details. Use of other methods to adjust castings to final grade may be allowed with approval by Engineer in Field.

When concrete adjustment rings are used, installation shall include using a concrete glue between all rings.

1206-4 MEASUREMENT AND PAYMENT

1206-4.1 ADJUST MANHOLE CASTING IN ASPHALT PAVEMENT. This item shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Adj Manhole Casting-Asph Pvmt" complete as detailed and accepted by the ENGINEER.

1206-4.2 FURNISH AND ADJUST MANHOLE CASTING IN ASPHALT PAVEMENT. This item shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Furn &Adj. M.H. Cast-Asph Pvmt" complete as detailed and accepted by the ENGINEER.

1206-4.3 ADJUST TYPE 24" INLET CASTING. This item shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Adjust Type 24" Inlet Casting" complete as detailed and accepted by the ENGINEER.

1206-4.4 FURNISH AND ADJUST TYPE 24" INLET CASTING. This item shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Furn & Adj Type 24" Inlet Cast" complete as detailed and accepted by the ENGINEER.

1206-4.5 ADJUST TYPE 36" INLET CASTING. This item shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Adjust Type 36" Inlet Casting" complete as detailed and accepted by the ENGINEER.

1206-4.6 FURNISH AND ADJUST TYPE 36" INLET CASTING. This item shall be measured on an individual unit basis (EA) and paid for at the unit price bid for " Furn & Adj Type 36" Inlet Cast" complete as detailed and accepted by the ENGINEER.

1206-4.7 ADJUST TYPE 72" INLET CASTING. This item shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Adjust Type 72" Inlet Casting" complete as detailed and accepted by the ENGINEER.

1206-4.8 FURNISH AND ADJUST TYPE 72" INLET CASTING. This item shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Furn & Adj Type 72" Inlet Cast" complete as detailed and accepted by the ENGINEER.

1206-4.9 ADJUST TYPE 108" OR LARGER INLET CASTING. This item shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Adj Type 108" or LGR Inlet Casting" complete as detailed and accepted by the ENGINEER.

1206-4.10 FURNISH AND ADJUST TYPE 108" OR LARGER INLET CASTING. This item shall be measured on an individual unit basis (EA) and paid for at the unit price bid

for "Furn&Adj Type 108" or LGR Inlet Cast" complete as detailed and accepted by the ENGINEER.

1206-4.11 ADJUST VALVE BOX IN ASPHALT PAVEMENT. This item shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Adjust Valve Box-Asph Pvmt" complete as detailed and accepted by the ENGINEER.

Valve boxes contracted under this bid item which are located outside of concrete or paved areas, such as in street boulevards, shall be measured and paid for at one-half the unit price bid for "Adjust Valve Box-Asph Pvmt"

1206-4.12 WATER STOP BOX EXTENSION. This item shall be measured on an individual basis (EA) and paid for at the unit price bid for "Water Stop Box Extension" complete, in place and accepted by the ENGINEER.

1206-4.13 WRAPPED UTILITY BOXES. This item shall be measured and paid at the unit price bid per each (EA) "Wrapped Utility Box" complete, in place, as detailed and accepted by the ENGINEER.

1206-4.14 ADJUST VALVE BOX IN CONCRETE. This item shall be measured on an individual basis (EA) and paid for at the unit price bid for "Adjust Valve Box in Concrete" complete as detailed and accepted by the ENGINEER.

Valve boxes contracted under this bid item which are located outside of concrete or paved areas, such as in street boulevards, shall be measured and paid for at one-half the unit price bid for "Adjust Valve Box In Concrete."

1206-4.15 ADJUST MANHOLE CASTING IN CONCRETE. This item shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Adjust MH Casting-Concrete" complete as detailed and accepted by the ENGINEER.

1206-4.16 FURNISH AND ADJUST MANHOLE CASTING IN CONCRETE. This item shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Furn & Adj. M.H. Cast-Concrete" complete as detailed and accepted by the ENGINEER.

1206-4.17 ADJUST MANHOLE CASTING IN UNPAVED AREA. This item shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Adj M.H. Casting-Unpaved Area" complete as detailed and accepted by the ENGINEER.

1206-4.18 FURNISH AND ADJUST MANHOLE CASTING IN UNPAVED AREA. This item shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Furn &Adj MH Cast-Unpaved Area" complete as detailed and accepted by the ENGINEER.

1206-4.19 ADJUST VALVE BOX IN UNPAVED AREA. This item shall be measured on an individual unit basis (EA) and paid for at the unit price bid for “Adj Valve Box-Unpaved Area” complete as detailed and accepted by the ENGINEER.

SECTION 1209 – SANITARY SEWER AND WATER SERVICE CONNECTIONS

1209-1 DESCRIPTION

This item shall consist of furnishing and installing sanitary sewer and water service connections from the main lines located in public easements or rights-of-way, such as streets and alleys, up to within five feet of a building foundation or meter pit . The materials, equipment, and construction methods shall be in full compliance with the ordinances of the City of Bismarck, the North Dakota State Plumbing Code, regulations set forth by the North Dakota Department of Environmental Quality (NDDEQ), and in accordance with these specifications and standard details.

1209-2 MATERIALS

1209-2.1 POLYVINYL CHLORIDE SEWER PIPE. PVC sewer pipe and fittings shall conform to the requirements of ASTM D3034 for type PSM, PVC sewer pipe and fittings and shall have an SDR of 35 which shall be stamped on the pipe. PVC sewer pipe with a bury depth of equal to or greater than 18 feet shall have an SDR of 26. Gasketed type joints on PVC pipe and fittings are preferred. Use of PVC sewer pipe joint cement must be approved by the ENGINEER prior to construction. The polyvinyl chloride sewer pipe joint cement shall consist of a viscous brushable solution of polyvinyl chloride in suitable active solvents. The cement shall be purchased from the pipe manufacturer and used in accordance with the manufacturer's instructions. It shall produce a joint of sufficient strength to permit normal installation handling within 5 minutes after jointing when exercising reasonable care.

1209-2.2 JOINT MATERIALS. Joint materials for sewer pipe shall conform to Section 801.

1209-2.3 COPPER WATER PIPE. Copper water pipe shall be 1” to 2” only and conform to ASTM B88, Type K Soft copper. Copper water service connections shall be flared or compression type Pack Joint coupling as manufactured by Mueller, Ford or approved equal.

1209-2.4 POLYETHYLENE WATER PIPE. Polyethelene (PE) water service pipe shall be 1” to 2” only. PE pipe shall be manufactured from ultra-high molecular weight PE of virgin materials and shall meet the requirements of Type III class “C” category 5-P34 PE as defined in ASTM D1248 with a working pressure of 250 psi.

PE pipe shall conform to ASTM D2239 with a SIDR of 7 or conform to ASTM D2737 with a SIDR of 9. If PE pipe with SIDR of 9 is used, pipe shall be 1¼” CTS in lieu of 1” IPS nominal size.

PE water service connections shall be compression type Pack Joint couplings as manufactured by Mueller, Ford or approved equivalent and shall include the installation of a stainless steel insert stiffener at all connections. The use of other couplings shall not be permitted.

1209-2.5 CORPORATION STOP. Corporation stops shall be Mueller, McDonald, Ford, or approved equivalent and shall be a ball valve with AWWA tapered threads. No plug or key style corporation stops will be allowed.

1209-2.6 CURB STOP. Curb stops shall be Mueller, McDonald, Ford, or approved equivalent and shall be a straight ball valve, without drain, having a Minneapolis Pattern. Curb stops shall be installed on a 6(six)-inch square by 4(four)-inch thick concrete pad.

1209-2.7 CURB BOX. Curb boxes shall be McDonald No. 5622 or Mueller No. H- 10302 (1½-inch diameter upper section) for all size curb stops, or approved equivalent. Stationary rods shall not be installed with curb stop boxes. The length of the curb box extended shall be 8 feet. The curb stop box cap shall have a pentagon cast iron threaded plug.

All curb boxes shall be encased with 8-mil linear low-density (LLD) polyethylene film in accordance with ANSI/AWWA C105/A21.5. All encasements shall be considered incidental.

1209-2.8 CONCRETE. Concrete for pipe cradles and saddles shall conform to the requirements of Section 501.

1209-2.9 TAPPING SLEEVE WITH TAPPING VALVE. For pipe sizes 6 inches to 24 inches, the tapping sleeve shall be stainless steel with a stainless steel flange and bolts and shall conform to the “Smith Blair” Type 663 or “Romac” Type SST, or approved equivalent. For pipe sizes 24 inches or larger, the tapping sleeve shall be epoxy lined and coated with stainless steel bolts and shall conform to the “Smith Blair” Type 622 Split Sleeve with O-Ring Seal. The tapping valve shall conform to City of Bismarck Standard Specification 901-2.7 for Gate Valves.

The City of Bismarck Public Works Department will tap the water main at a charge to the CONTRACTOR. The CONTRACTOR shall be responsible for all other work connected with installation of the tapping sleeve and valve, including the necessary space around the water main required for the tapping machine and assisting the Public Works Department in positioning the tapping machine.

All corporation taps made into all sizes and classes of asbestos cement, PVC, sandcast iron, cast iron, ductile iron, and prestressed concrete water mains shall be reinforced with a tapping saddle.

Tapping saddles used on PVC water main shall provide full support around the circumference of the pipe and provide a bearing area of sufficient width along the axis of the pipe 2 inches minimum, ensuring that the pipe will not be distorted when a saddle is tightened.

Tapping saddles for PVC, ductile iron, cast iron, and sand cast iron water main up to 12 inches in diameter shall be one of the following: (a) Romac Style 306, (b) PowerSeal Model 3412, (c) Smith Blair Series 370, (d) Ford FS313 or (e) approved equivalent.

Tapping saddles for PVC, ductile iron, cast iron, and sand cast iron water main over 12 inches in diameter shall be Romac Style 305 or approved equivalent.

Tapping saddles for asbestos cement water main shall be a double strap bronze with an O-ring gasket cemented in body groove as manufactured by the Mueller Company or approved equivalent.

Tapping saddles for prestressed concrete water mains shall be approved by the ENGINEER.

Tapping saddles shall be installed according to manufacturer's installation instructions. The tapping saddle bolts shall be torqued using a calibrated torque wrench with a handle at least 12 inches in length. The CONTRACTOR should be prepared to show certification of torque wrench calibration at the request of the ENGINEER.

1209-3 CONSTRUCTION REQUIREMENTS

1209-3.1 GENERAL. Construction requirements shall conform to Section 801 for sewer service connections and Section 901 for water service connections. All pipe and fittings shall be installed in accordance with the manufacturer's recommendations unless otherwise specified herein. All copper water service lines shall be constructed "snaked" within the trench.

On new construction, for each sewer stubout, a 2x2 inch wood marker shall be placed a minimum of one foot from the end of the sewer stubout, shall extend vertically and plumb to not less than two feet above the existing surrounding ground, and painted green.

On new construction, for each water curb stop , a metal T-Post marker shall be placed a maximum of one foot from the curb stop box, extended vertically to a minimum of three feet above the existing surrounding ground, and painted blue.

The CONTRACTOR shall be responsible for maintaining the markers until the project has been accepted by the ENGINEER. The cost of the stubout markers shall be considered incidental to other bid items.

When connecting to a sewer main and a wye is not available, the connection shall be made using an Inserta Tee manufactured by Inserta Fittings Co., or approved equivalent. A factory-assembled wye may be cut in using gasketed repair couplers. When connecting to VC sewer main, Shear Guard Indiana Seal (GPK) repair couplers manufactured by Fernco, Inc., or approved equivalent, may be used.

Bedding material in accordance with Section 801 shall be placed in the trench, prior to laying any type of sewer pipe, 2 inches below bottom of pipe up to 6 inches or smaller, 4 inches when pipe used is 8 inches or larger. Bedding material shall be installed to the centerline of the pipe and the full width of the excavating trench.

1209-3.2 SANITARY SEWER RISERS. When the depth of the sewer exceeds 12 feet, risers shall be installed. Risers shall be sufficiently long to reach within 10 feet of the top of curb elevation. When risers are five feet or greater, 1¼-inch crushed rock shall be used to encase the wye and support the vertical bend. Risers shall be laid on a slope not to exceed 2:1 vertical to horizontal.

1209-4 MEASUREMENT AND PAYMENT

1209-4.1 thru 4.5 (X)" SEWER SERVICE PIPE. On new construction, the sewer service pipe shall be measured by the linear foot (LF) from centerline of sewer main to plugged end of service connection. On reconstruction projects, the sewer service pipe shall be measured by the linear foot (LF) from end of the wye to the end of the existing sewer service pipe. Sewer service pipe shall be paid for at the unit price bid for "(X)" Sewer Service Pipe" complete, in place, and accepted by the ENGINEER.

1209-4.6 thru 1209-4.10 (X)" SEWER PIPE BEND. The angle of the bend shall be compatible with the type of sewer service pipe and wye branch selected to provide a 90 degree angle between the sewer mainline and sewer service line. The sewer pipe bend shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "(X)" 45-Degree Bend" complete, in place, and accepted by the ENGINEER.

1209-4.11 thru 1209-4.23 (X)" (MATERIAL) WATER SERVICE PIPE. On new construction, the water service pipe shall be measured on an in-line basis by the linear foot (LF) from the centerline of the water main at the water service connection to the end of the water service pipe. On reconstruction projects, the water service pipe shall be measured on an in-line basis by the linear foot (LF) from the water service connection directly to the end of the existing water service pipe. Water service lines shall be paid for at the unit price bid for "(X)" (MATERIAL) Water Service Pipe" complete, in place, and accepted by the ENGINEER.

1209-4.40 thru 4.49 (X)" WATER SERVICE CONNECTION. This connection shall include one tapping sleeve, one tap to the water main, and one corporation stop. The connection shall be measured as a combined unit on an individual unit basis (EA) and paid for at the unit price bid for "(X)" Water Service Connection" complete, in place, and accepted by the ENGINEER.

1209-4.50 thru 4.54 (X)" CURB STOP AND (X)" CURB BOX. The curb stop and curb box shall be measured as a combined unit on an individual unit basis (EA) and paid for at the unit price bid for "(X)" CB Stop & (X)" CB Box" complete, in place, and accepted by the ENGINEER.

1209-4.55 DISCONNECT WATER SERVICE LINE. Disconnecting a water service line shall consist of turning off the corporation stop at the main and disconnecting the pipe after the corporation stop. Disconnect Water Service Line shall be measured on an individual unit basis (EA) and paid for at the unit price bid for "Disconnect Water Service Line" complete, in place, backfilled, and accepted by the ENGINEER.

SECTION 1211 – TRAFFIC CONTROL

1211-1 DESCRIPTION

This work consists of furnishing, installing, documentation, and maintaining all required traffic control devices according to an approved traffic control plan or details shown on the plans. This includes specifications providing for watch persons, flaggers, pilot cars, and necessary precautions for protecting the public, the workers, and the work.

The CONTRACTOR must submit a traffic control plan to the ENGINEER for approval at least two weeks prior to setting up the detour closing a roadway.

The CONTRACTOR shall provide, prior to construction, all proposed haul routes to the ENGINEER for approval. The ENGINEER may require a haul road inspection at his discretion.

Press releases shall be submitted to the ENGINEER for review a minimum of three days prior to each change in operation or phase. The CONTRACTOR shall notify the Traffic ENGINEER a minimum of 48-hours prior to implementation of the traffic control plan for a road closure, detour or lane reduction for the issuance of a press release. A press release is required to announce the reopening of a roadway with a detour when not otherwise notified.

The CONTRACTOR is responsible for the placement and maintenance of all work zone signs and barricades during construction. All traffic control devices shall be installed and maintained in a safe and orderly manner complying with the provisions of Chapter 6 of the most recent update of the *Manual on Uniform Traffic Control Devices for Streets and Highways*, U.S. Department of Transportation.

The CONTRACTOR is responsible for maintaining and protecting traffic, the public and the work during a temporary suspension of work.

The CONTRACTOR shall designate a superintendent and an alternate for emergency repair service to traffic control devices. Telephone numbers for these personnel shall be provided to the ENGINEER at the preconstruction meeting. These personnel shall be available at all times to respond to an emergency.

When an emergency occurs and the superintendent and alternate are not available to take protective measures, the CITY may authorize others to do the necessary work and deduct the cost of the work from the CONTRACTOR.

1211-2 MATERIALS AND EQUIPMENT

All materials and construction details not specifically addressed in the Plans, Special provisions, and Construction Specifications for Municipal Public Works, Bismarck, North Dakota shall be in conformance with Section 704 of the most recent edition and supplements of the Standard Specifications for Road and Bridge Construction, North Dakota Department of Transportation, NDDOT CAD Standard Drawings, and the provisions of Chapter 6 of the most recent update of the *Manual on Uniform Traffic Control Devices for Streets and Highways*.

Traffic control devices used on the project will be rated according to the American Traffic Safety Services Association (ATSSA) *Quality Standards for Work Zone Traffic Control Devices.* The definitions of "acceptable," "marginal," and "unacceptable" and the evaluation guidelines shall be as defined in ATSSA's *Quality Standards for Work Zone Traffic Control Devices.*

The ENGINEER retains the right to reject any traffic control device that is considered "marginal" or "unacceptable". All traffic control devices that are rejected shall be immediately removed from the public right-of-way and replaced with "acceptable" devices.

Payment for traffic control devices, labor, plans, and maintenance shall be measured and paid by the lump sum as "Traffic Control" for each unit.

1211-3 CONSTRUCTION REQUIREMENTS

1211-3.1 GENERAL. The CONTRACTOR shall furnish, install, and maintain all required traffic control devices, and shall provide watchpersons and flaggers as necessary to protect the work and to ensure public and workers' safety. All required traffic control devices shall be available for installation when needed and shall be maintained, relocated, covered, or removed as necessary.

If the CONTRACTOR has not furnished, installed, located, maintained, or removed traffic control devices as required, the ENGINEER may direct work to cease until the deficiencies have been corrected. The ENGINEER may authorize others to do the necessary work and deduct the cost of the work from the CONTRACTOR, if deficiencies are not corrected in a timely manner and pose a risk to public safety.

If the CONTRACTOR's construction operations or sequence requires additional traffic control devices, such as signing, flashing arrow boards, barricades, channelizing devices or flaggers, they shall be furnished at the CONTRACTOR's expense and construction operations shall be suspended in that area until the condition is corrected and the required traffic control devices have been installed.

Traffic control devices shall be operated only as long as they are needed. Only those devices that apply to existing conditions shall be in place.

1211-3.2 WORK AREA SIGNING. Appropriate traffic control devices as shown on the traffic control plan drawings shall be erected and maintained for each type of work area required by the operations. When no details are provided for the particular type of construction situation involved, traffic control devices shall be installed according to the *MUTCD* or as directed by the ENGINEER. No construction work shall be started until the proper traffic control devices for the work area are in place.

When traffic is carried through the construction area, two-way traffic shall be maintained when practicable or when required per the project plans or special provisions. One-way traffic shall be directed by flag persons or maintained under control of an approved traffic signal system. All signs and other traffic control devices shall indicate actual conditions and shall be relocated, removed, or changed, as conditions require. Traffic Control Devices necessary only during hours when work is actually being performed shall be removed or completely covered when no work is in progress.

All channelizing devices shall be reflectorized and be a minimum of 36 inches in height. Non-reflectorized channelizing devices shall not be allowed.

When pedestrian facilities are impacted due to maintenance or construction, signs mounted on pedestrian barricades shall be placed to allow pedestrian traffic to route around the work zone per detail 1211-1. Pedestrians shall be rerouted to pedestrian facilities when feasible. In residential areas the ENGINEER will determine what type of pedestrian guidance is required.

The cost to remove and reset existing traffic signs to accommodate construction shall be included in the price bid for other items.

1211-3.3 TEMPORARY SUSPENSION. During a temporary suspension of work, the CONTRACTOR is responsible for maintaining and protecting traffic. When work is suspended for winter and roadways are reopened for normal operations, all traffic control devices shall be removed from the public right-of-way. Resetting of signs removed due to temporary suspension will not be measured for payment.

1211-3.4 TRAFFIC CONTROL SUPERVISOR. The CONTRACTOR shall designate a qualified traffic control supervisor. This supervisor shall be in addition to the watchperson. If this traffic control supervisor becomes unavailable on the project, the CONTRACTOR shall designate a qualified replacement supervisor.

a. Qualifications. The traffic control supervisor shall:

1. Have completed a NDDOT-approved comprehensive course of study based on Part VI of the *MUTCD* or have completed certification by American Traffic Safety Services Association (ATSSA) as a Traffic Control Supervisor and furnish proof hereof.
2. Be familiar with the requirements traffic control plans and specifications.

3. Have a total of at least 12 months field experience with traffic control plans, layouts, and maintenance.
4. Be competent to supervise personnel in traffic control operations.

b. Duties. The traffic control supervisor shall:

1. Provide traffic control as required by the plans, specifications, *MUTCD*, or as directed by the ENGINEER.
2. Supervise the installation, operation, inspection, maintenance, and removal of the traffic control system.
3. Correct traffic control devices to reduce potential for conditions that cause erratic vehicle movements, unexpected braking, etc.
4. Monitor and propose changes to improve traffic flow through the work zone.
5. Be accessible to the job site within one hour of notification and be "on call" on a 24-hour basis.
6. Provide the ENGINEER with daily documentation of all traffic control activities.
7. Function as watchperson in his/her absence.

1211-3.5 WATCHPERSONS. Watchpersons shall be provided to patrol the project to assure that the traffic control devices are properly placed in accordance with the traffic control plans and standards. The project shall be patrolled a minimum of every 4 hours, including once before 9 a.m. and at least once after 6 p.m. during construction activities. and twice daily, before 10 am and after 4pm on weekends and days when no work is in progress. The CONTRACTOR shall provide written documentation (log or diary) to the ENGINEER of the watchperson's hours and activities.

The CONTRACTOR shall immediately assist the watchperson, whenever needed, to correct conditions that cause erratic traffic movement, unexpected braking, etc., and erect, repair, replace, or relocate the required traffic control devices. Emergency assistance shall be provided to motorists, when needed, due to roadway conditions. Suspension of watchperson service may be permitted during periods of authorized suspension or after substantial completion of the work, provided the job site is in safe condition.

1211-3.6 EMERGENCY CONTROL. Written notification shall be provided to the ENGINEER, the State Police, and local law enforcement agencies of the names, addresses, and telephone numbers of the CONTRACTOR's Superintendent and an alternate at the preconstruction meeting. Either the Superintendent or the alternate shall be on call for notification of any emergencies that may arise during periods when construction operations are not in progress. Changes in the designation of the superintendent or the alternate shall immediately be made known, in writing, to the ENGINEER and the law enforcement agencies.

The CONTRACTOR's superintendent or alternate, or traffic control foreman shall meet with the ENGINEER before work commences to review traffic control plans, and shall be available at all reasonable times to periodically discuss modifications to the traffic control plan with the ENGINEER or his representative.

When an emergency occurs and the superintendent or alternate are not available to take protective or corrective measures, the department will authorize others to do the necessary work and deduct the cost of the work from the CONTRACTOR.

1211-3.7 MAINTENANCE OF TRAFFIC CONTROL DEVICES. At the time of initial set up and major phase changes, 100 percent of each type of device (signs, barricades, vertical panels, drums, cones, tubular markers, warning lights, arrow panels, etc.) shall be classified as acceptable. The CONTRACTOR shall certify in writing to the ENGINEER that all traffic control devices installed are classified as acceptable.

For signs, barricades, vertical panels, drums, cones, tubular markers, and arrow panels, the number of acceptable devices of each type may decrease to 75 percent of the initial quantity as a result of damage or deterioration during the course of work. The remaining 25 percent of each type of devices may be in the marginal category. Warning lights shall be “acceptable” or “marginal” at the limits defined in the ATSSA Standards. All unacceptable devices found on the job site shall be replaced within 12 hours.

Traffic control devices not covered by the evaluation guidelines shall be maintained to operate effectively and be in good repair.

Traffic control devices shall be cleaned as necessary to remove dirt, mud, or other foreign material which reduces the brightness of the reflectorized sheeting or warning lights.

1211-3.8 NON-CONFORMANCE. Any devices found not meeting ATSSA or MUTCD standards shall be removed from site within 24 hours and replaced with proper devices.

Should traffic control be found insufficient or not meeting MUTCD or ATSSA Standards by ENGINEER, the CONTRACTOR shall, upon notification, have 24 hours on collector and local streets and four hours on arterial streets to remedy deficiencies as per the ENGINEER. If deficiencies are not corrected within this time, the ENGINEER may authorize others to perform the necessary work and deduct the invoiced cost of the work plus 20 percent from moneys owed to the CONTRACTOR.

1211-3.9 FLAGGING. The garments worn by flaggers shall comply with the American National Standard for High-Visibility Safety Apparel and Headwear ANSW/ISEA 107-2020.

Flaggers shall not be assigned other duties while working as authorized flaggers.

The CONTRACTOR is responsible for providing certified flaggers. The CONTRACTOR will acknowledge in writing, before any flagging work begins on the project, that all flaggers are certified before performing flagging on the project. Certification shall be based on a written examination found at ndflaggertraining.com.

1211-3.10 GARMENT REQUIREMENTS FOR ALL PERSONNEL. Garments complying with the standard ANSI/ISEA 107-2010 shall be worn by everyone working within city rights-of-way or work zones.

1211-4 MEASUREMENT AND PAYMENT

1211-4.1 TRAFFIC CONTROL. Traffic Control includes furnishing, installing, and maintaining the required signs, barricades, channelizing devices, flashing arrow boards, and other warning devices, relocating or removing devices as dictated by the work progress, and providing watchpersons/traffic control supervisor to patrol the work. These items shall be included in the pay item "Traffic Control."

Payment (over the lump sum bid for "Traffic Control") may be authorized for additional traffic control devices if the type or number of such devices requested by the ENGINEER exceeds the requirements indicated by the traffic control plan or when the need for additional traffic control devices is created as a result of contract revisions.

No additional payment will be authorized for additional traffic control devices required as a result of the CONTRACTOR's method or sequence of operation, whether or not the type of operation is included in the typical work area layouts shown on the traffic control plan sheets.

Traffic Control shall be measured as a lump sum, and graduated payment for the contract lump sum bid will be made according to the following schedule:

Percent of Bid Price Paid	Contract Requirement
40%	All initial traffic control devices required to start construction have been installed.
50%	Contract is 25% complete.
75%	Contract is 50% complete.
90%	Contract is 75% complete.
100%	Contract is complete.

When a project contains multiple units with individual traffic control bid items for each unit, "Contract" in the above table shall refer to the contract total for the unit to which the bid item applies and the traffic control bid item for each unit shall be paid according to the amount complete within that unit.

When additional traffic control devices are requested by the ENGINEER and qualify for payment, the cost for furnishing and installing such devices will be agreed upon by the ENGINEER and CONTRACTOR prior to installation. If no agreement can be made, the ENGINEER may authorize others to complete the additional work.

The above payments for installation include the cost of removing or relocating the traffic control devices. No additional payment will be made when traffic control devices are covered up, or temporarily taken out of service, and then returned to use.

All standard traffic control devices furnished by the CONTRACTOR shall remain the property of the CONTRACTOR.

Flagging shall be included in the pay item “Traffic Control.”

If the CONTRACTOR is required to furnish special non-standard signs not shown on the Plans, payment will be made at invoice price plus 15 percent, and the sign will become the City’s property after it has been removed from service. Payment for sign supports and installation of special signs will be as per payment for additional traffic control above.

SECTION 1212 – HIGHWAY POSTS AND SIGNS

Note: This section replaces prior versions

1212-1 DESCRIPTION

This item shall consist of furnishing, fabricating, and installing highway signs, delineators, and supporting structures.

1212-2 MATERIALS

All materials furnished and installed in this work shall be new and shall be per the plans and comply with the following:

a) NDDOT "Standard Specifications for Road and Bridge Construction", Current Edition, as revised.
b) Manual on Uniform Traffic Control Devices (MUTCD), Current Edition, as revised.

All sign faces shall be fabricated as per the plans and the alphabets shown in the MUTCD.

1212-2.1 CONCRETE. Concrete shall be as per Section 501.

1212-2.2 SIGN MATERIAL. All sign backing material shall be a minimum of 0.1 inches thick unless otherwise noted on the plans.

1212-3 CONSTRUCTION REQUIREMENTS

All installation shall be per the plans and comply with the following:

a) NDDOT "Standard Specifications for Road and Bridge Construction", Current Edition, as revised.
b) Manual on Uniform Traffic Control Devices (MUTCD), Current Edition, as revised, published by the FHWA.

1212-3.1 SIGN POST INSTALLATION. When sign installation occurs in a permanent surface, a 4-inch PVC sleeve shall be installed around the post for the thickness of the permanent surface. The sleeve shall be backfilled with loose, clean sand. The cost shall be incidental to sign bid items

Drive anchors for telescoping perforated tubes supports 2" to 4" above finish grade. The cost shall be incidental to sign bid items.

1212-3.2 LOCATING AND POSITIONING OF SIGNS. Signs shall be located according to the plans. Installed signs will be inspected at night, if any sign is ineffective at night, the sign shall be replaced at the CONTRACTOR's expense.

1212-3.3 ROAD CLOSED – TYPE III SINGLE BARRICADE. Shall consist of installing a single Type III barricade with a Road Closed Sign, R11-2-48, meeting ASTM D4956 Type XI, per Detail 1212-1.

1212-3.4 ROAD CLOSED – TYPE III TRIPLE BARRICADE. Shall consist of installing 3 Type III barricades, 2 without road closed signs and the center barricade a Road Closed Sign, R11-2-48, meeting ASTM D4956 Type XI, per Detail 1212-1.

1212-4 MEASUREMENT AND PAYMENT

All costs to properly complete the work specified herein and/or shown on the plans shall be included in the prices bid for these or other items unless applicable bid items are included in the contract.

1212-4.1 ROAD CLOSED – TYPE III. Shall be measured on a per each (EA) basis and paid for at the unit price for "ROAD CLOSED-TYPE III" complete, in place, and accepted by the ENGINEER.

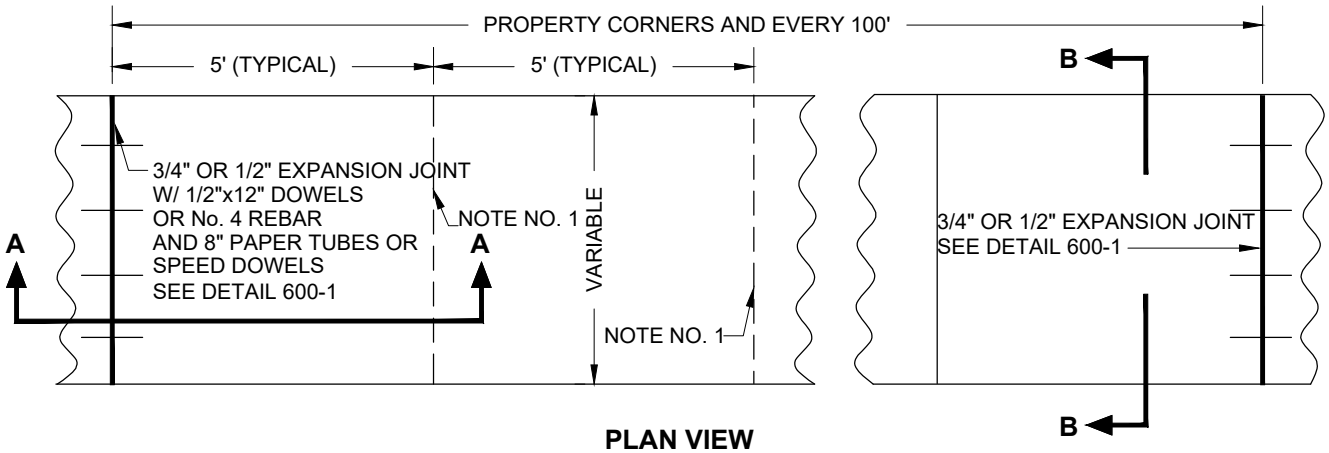
1212-4.2 RELOCATE ROAD CLOSED. Shall be measured on a per each (EA) basis and paid for at the unit price for "RELOCATE ROAD CLOSED" complete, in place, and accepted by the ENGINEER.

NOTE NO. 1 CONTRACTION JOINT SCORED 1" DEEP

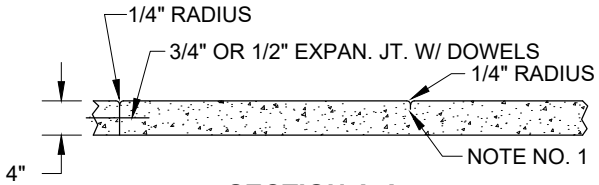
NOTE NO. 2 LONGITUDINAL CONTRACTION JOINTS SHALL BE SPACED EQUIDISTANT, WITH 7' MAX. SPACING, SCORED 1/3 DEPTH OF CONCRETE.

NOTE NO. 3 SIGN & FLAG POLE HOLDERS, WHERE DESIGNATED BY THE ENGINEER, SHALL BE PLACED 2' BEHIND THE FACE OF THE CURB. ALL GATE VALVES, CURB STOP BOXES, HYDRANTS, TRAFFIC CONTROL FOUNDATIONS AND SIGN POSTS SHALL BE WRAPPED WITH APPROVED EXPANSION JOINT, OR APPROVED BOND BREAKER MATERIAL.

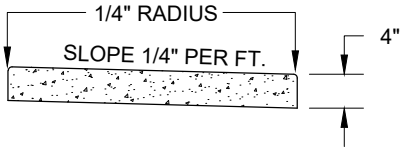
NOTE NO. 4 TRANSITIONS BETWEEN 4.5' & 6' SIDEWALKS SHALL BE TAPERED. TAPERS SHALL OCCUR IN NO LESS THAN 10' UNLESS APPROVED BY ENGINEER.



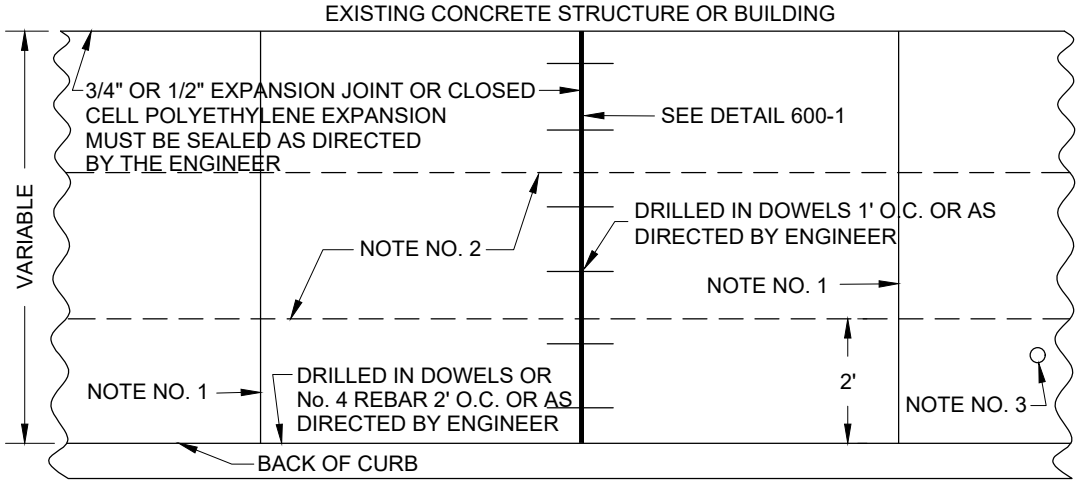
PLAN VIEW



SECTION A-A



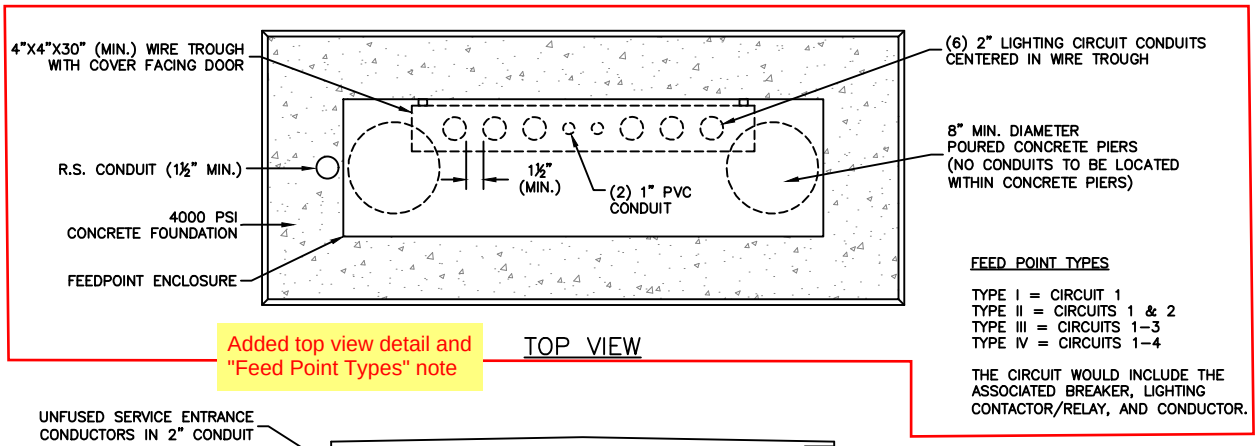
SECTION B-B



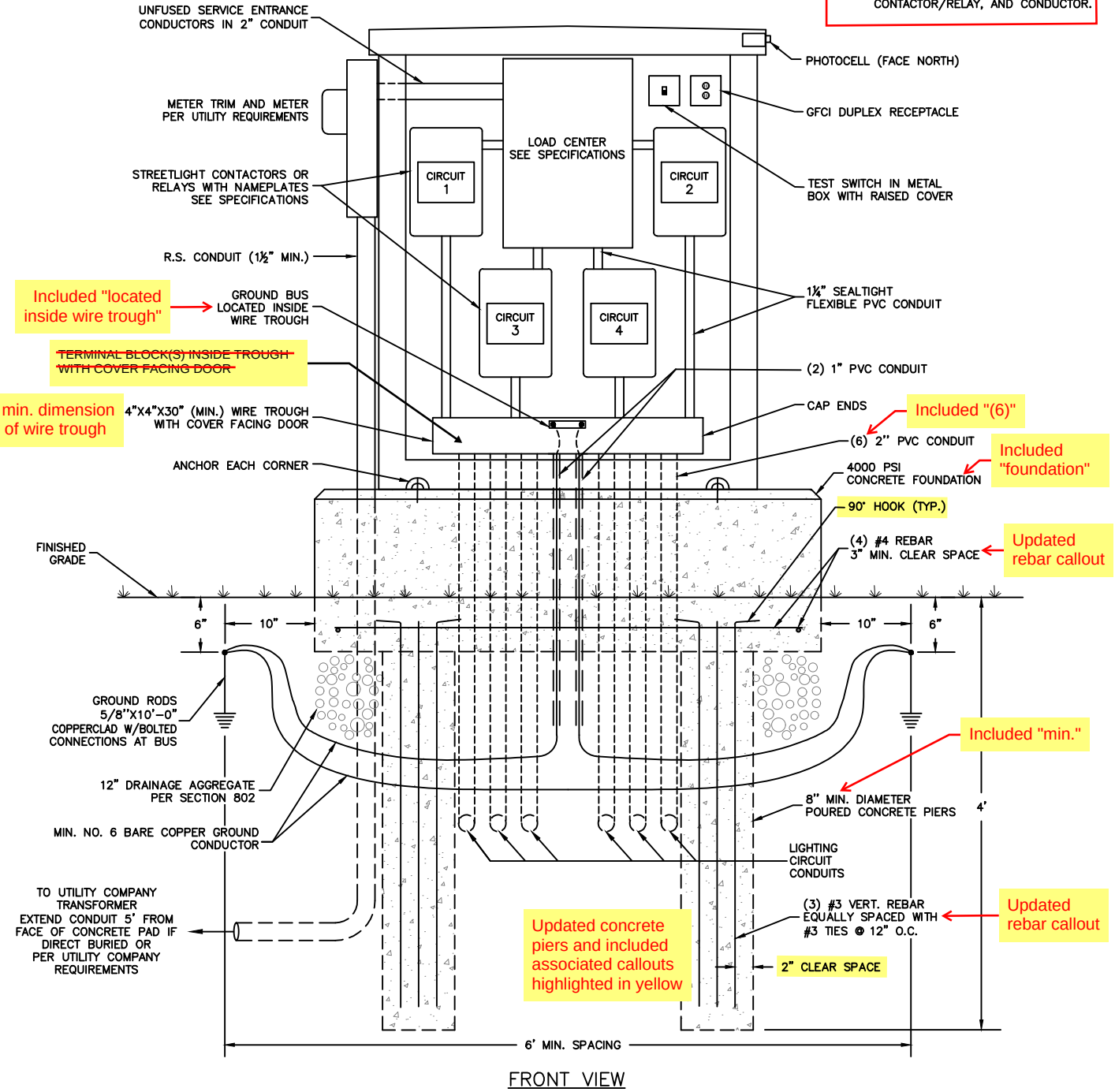
COMMERCIAL SIDEWALK
PLAN VIEW

NOTE: AGGREGATE BASE AS PER SECTION 601

Bismarck ENGINEERING	**COMMERCIAL CONCRETE SIDEWALK**	SCALE: Not to Scale DATE: 12/2023	STANDARD DETAIL NO. **600-2**



Added top view detail and "Feed Point Types" note



Included "located inside wire trough"

Updated min. dimension and view of wire trough

Included "(6)"

Included "foundation"

Updated rebar callout

Included "min."

Updated rebar callout

Updated concrete piers and included associated callouts highlighted in yellow

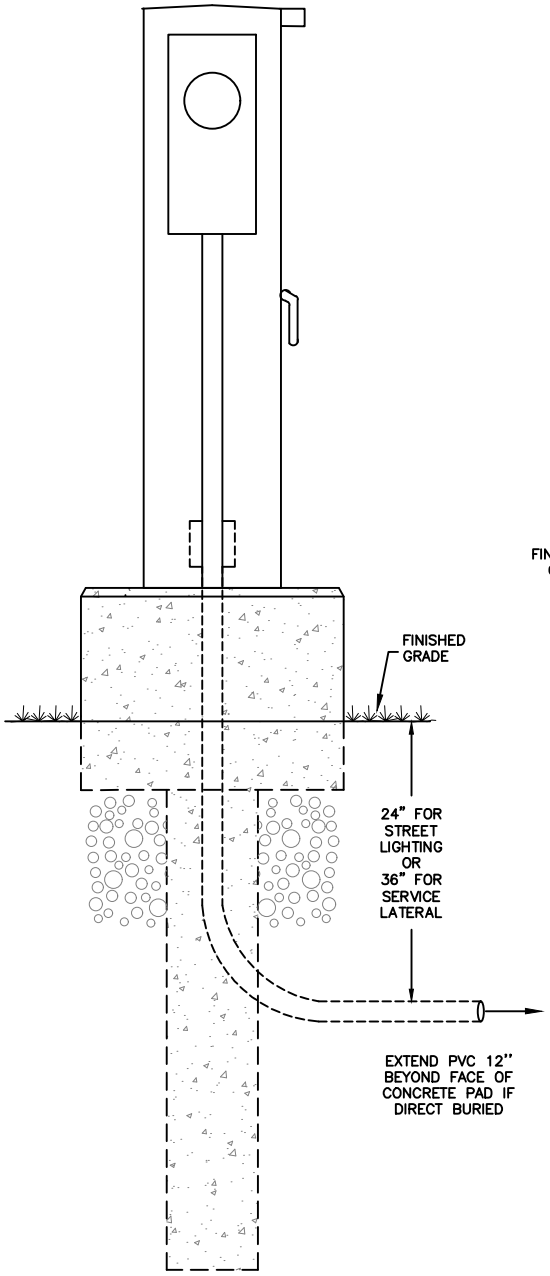
Bismarck ENGINEERING

PAD MOUNTED FEED POINT DETAIL (SHEET 1 OF 2)

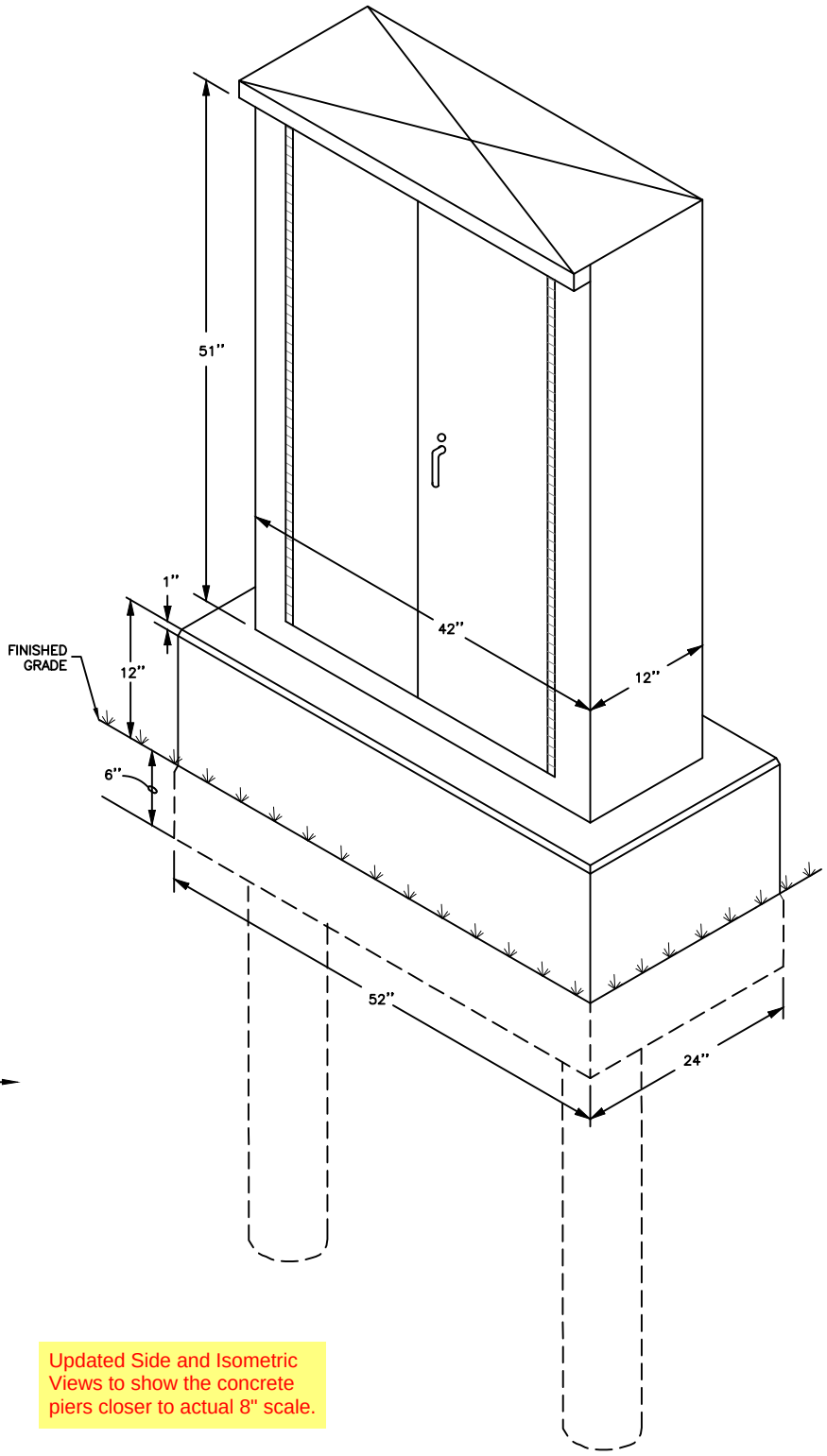
SCALE: Not to Scale

DATE: 01/2024

STANDARD DETAIL NO. 1003-1



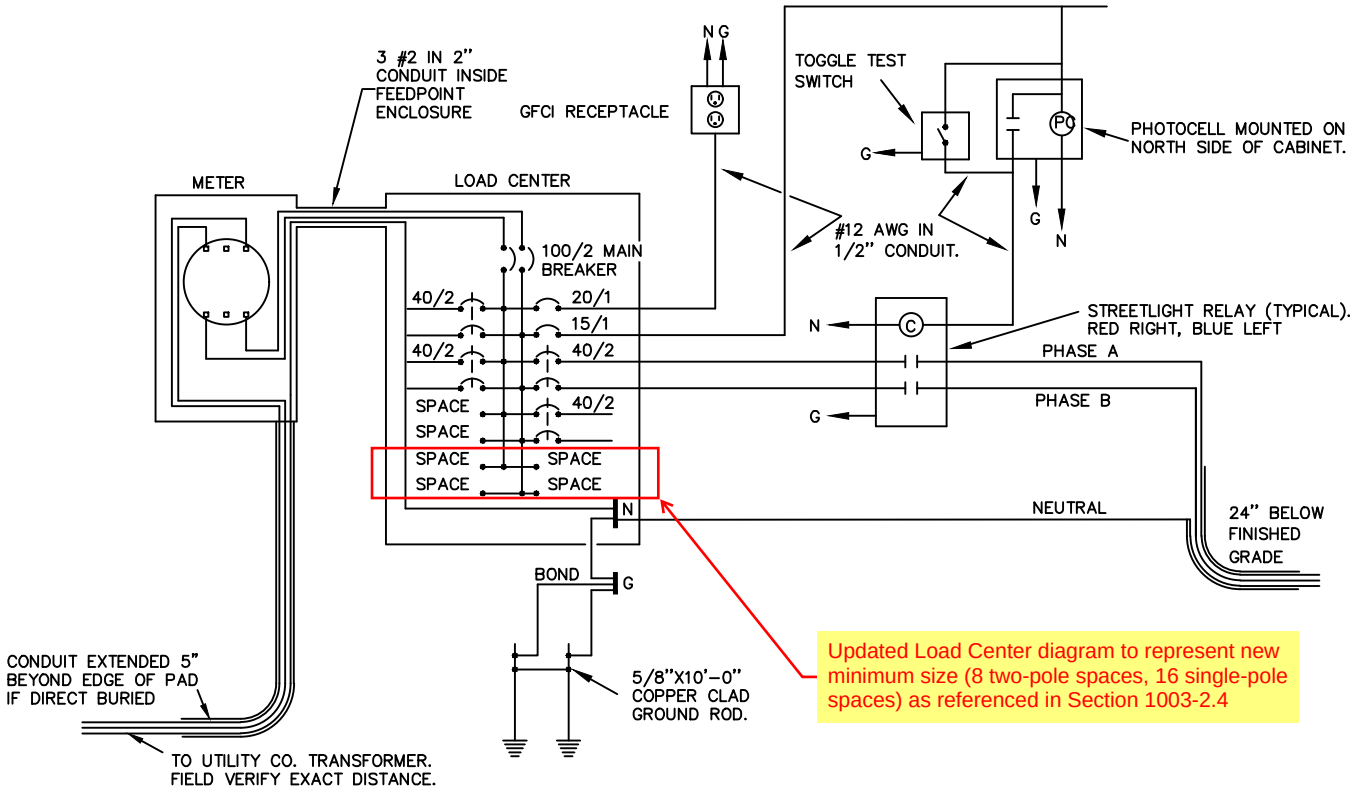
SIDE VIEW



Updated Side and Isometric Views to show the concrete piers closer to actual 8" scale.

ISOMETRIC VIEW

Bismarck ENGINEERING	**PAD MOUNTED FEED POINT DETAIL (SHEET 2 OF 2)**	SCALE: Not to Scale DATE: 01/2024	STANDARD DETAIL NO. **1003-2**



Included Notes

NOTE NO.1 - MAIN BREAKER TO BE ON TOP OF PANEL BOARD
NOTE NO. 2 - PROVIDE ONE RELAY/CONTACTOR FOR EACH STREETLIGHT CIRCUIT

Bismarck ENGINEERING	FEED POINT WIRING DETAIL	SCALE: Not to Scale DATE: 01/2024	STANDARD DETAIL NO. 1003-4

NOTE NO.1 - MANHOLE CASTING AND COVER SHALL BE AS DEFINED IN SECTION 1205.

NOTE NO.2 - ALLOWANCE FOR ADJUSTMENT - 0" TO 6" - SEE STANDARD DETAIL NO. 1206-1.

NOTE NO.3 - P2 GASKETED JOINT FOR 48" MANHOLES, CX-4 JOINT FOR ALL OTHER SIZES OF MANHOLES AND EXTERIOR SEAL BY PRESS-SEAL GASKET CORP. EZ WRAP AND EZ STIK NO. 4 PRIMER, SPECIALTY PRODUCTS "MAC WRAP", INFI-SHIELD EXTERNAL GATOR WRAP, OR AN APPROVED EQUAL.

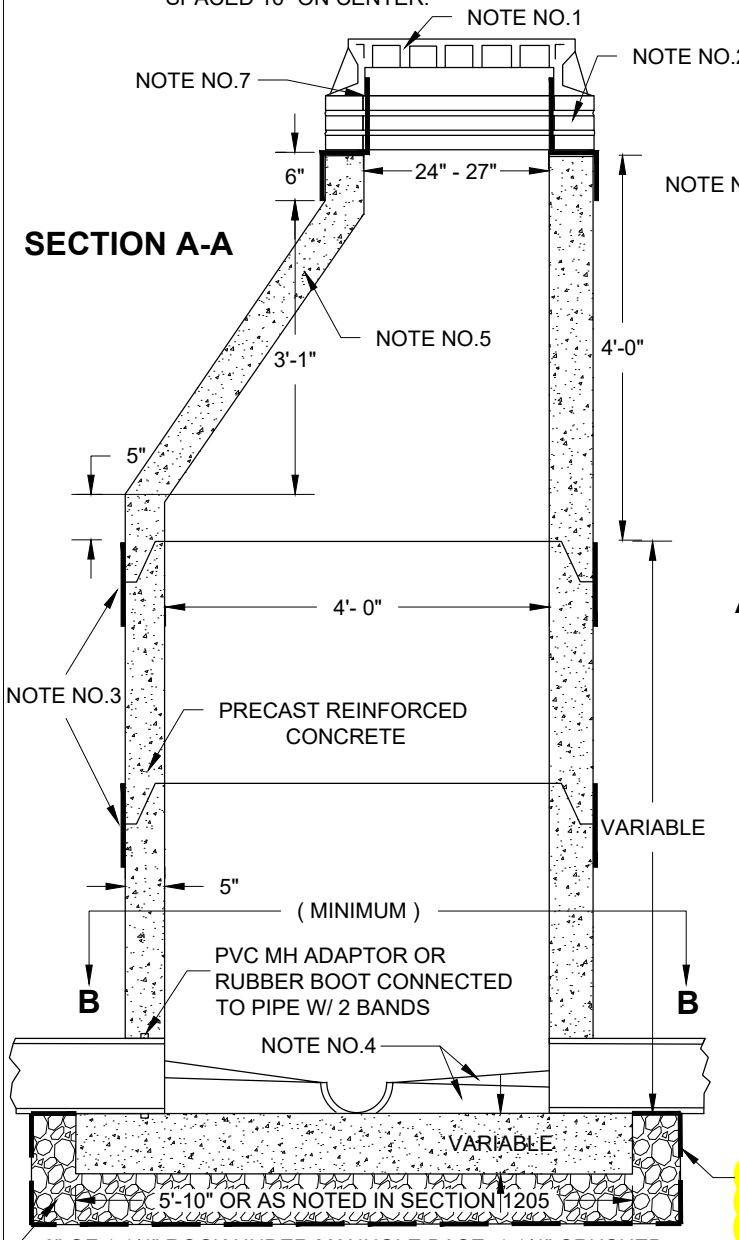
NOTE NO.4 - IF PRECAST MANHOLE BASE IS USED, FLOOR SHALL BE GROUTED AND SLOPED, AS SHOWN, FROM 1\2" THE DIAMETER OF THE PIPE. SEE SECTION 1205 FOR PRECAST MANHOLE BASE REINFORCEMENT.

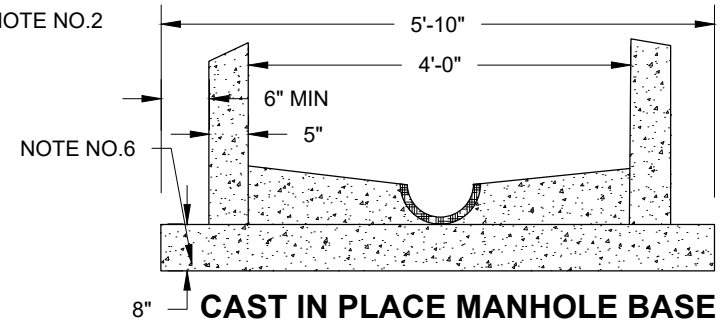
NOTE NO.5 - MANHOLE TOP SECTION SHALL BE ECCENTRIC FOR 48" MANHOLES.

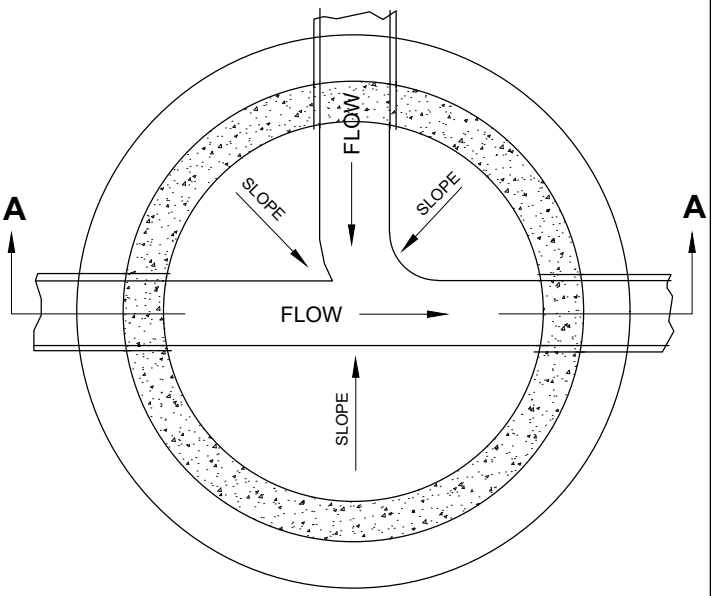
NOTE NO.6 - CAST IN PLACE MANHOLE BASES SHALL BE DIMENSIONED AS SHOWN UNLESS OTHERWISE INDICATED. BASES SHALL BE REINFORCED WITH NO.4 REBAR SPACED 15" ON CENTER BOTH WAYS.

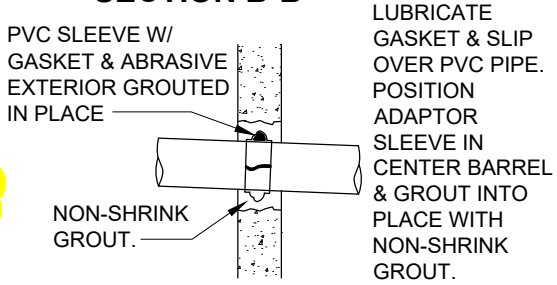
NOTE NO.7 - IF REQUIRED PER SECTION 1205, I/I BARRIER AS MANUFACTURED BY AP/M PERMAFORM OR AN APPROVED EQUAL. ADJUSTMENT RINGS SHALL NOT EXCEED THE HEIGHT OF THE I/I BARRIER.

NOTE NO.8 - STEPS SHALL NOT BE PLACED IN SANITARY SEWER, STORM SEWER, AIR RELEASE MANHOLES OR INLETS UNLESS SPECIFIED. IF SPECIFIED, THE MANHOLE STEPS SHALL BE DELTA SUREFOOT OR APPROVED EQUAL. STEPS SHALL BE SPACED 16" ON CENTER.









ROCK SHALL BE WRAPPED IN R1 FABRIC WHEN GROUND WATER IS PRESENT

Bismarck ENGINEERING	SANITARY SEWER MANHOLE	SCALE: Not to Scale DATE: 1/2024	STANDARD DETAIL NO. 1205-1

NOTE NO.1 - MANHOLE CASTING AND COVER SHALL BE AS DEFINED IN SECTION 1205, CENTER OVER FLOW LINE.

NOTE NO.2 - ALLOWANCE FOR ADJUSTMENT - 0" TO 6" - SEE STANDARD DETAIL NO. 1206-1.

NOTE NO.3 - P2 GASKETED JOINT FOR 48" MANHOLES, CX-4 JOINT FOR ALL OTHER SIZES OF MANHOLES AND EXTERIOR SEAL BY PRESS-SEAL GASKET CORP. EZ WRAP AND EZ STIK NO. 4 PRIMER, SPECIALTY PRODUCTS "MAC WRAP", INFI-SHIELD EXTERNAL GATOR WRAP, OR AN APPROVED EQUAL.

NOTE NO.4 - IF PRECAST MANHOLE BASE IS USED, FLOOR SHALL BE GROUTED AND SLOPED, AS SHOWN, FROM 1\2 THE DIAMETER OF THE PIPE. SEE SECTION 1205 FOR PRECAST MANHOLE BASE REINFORCEMENT.

NOTE NO.5 - DROP MANHOLE SHALL BE MIN 60" INSIDE DIAMETER.

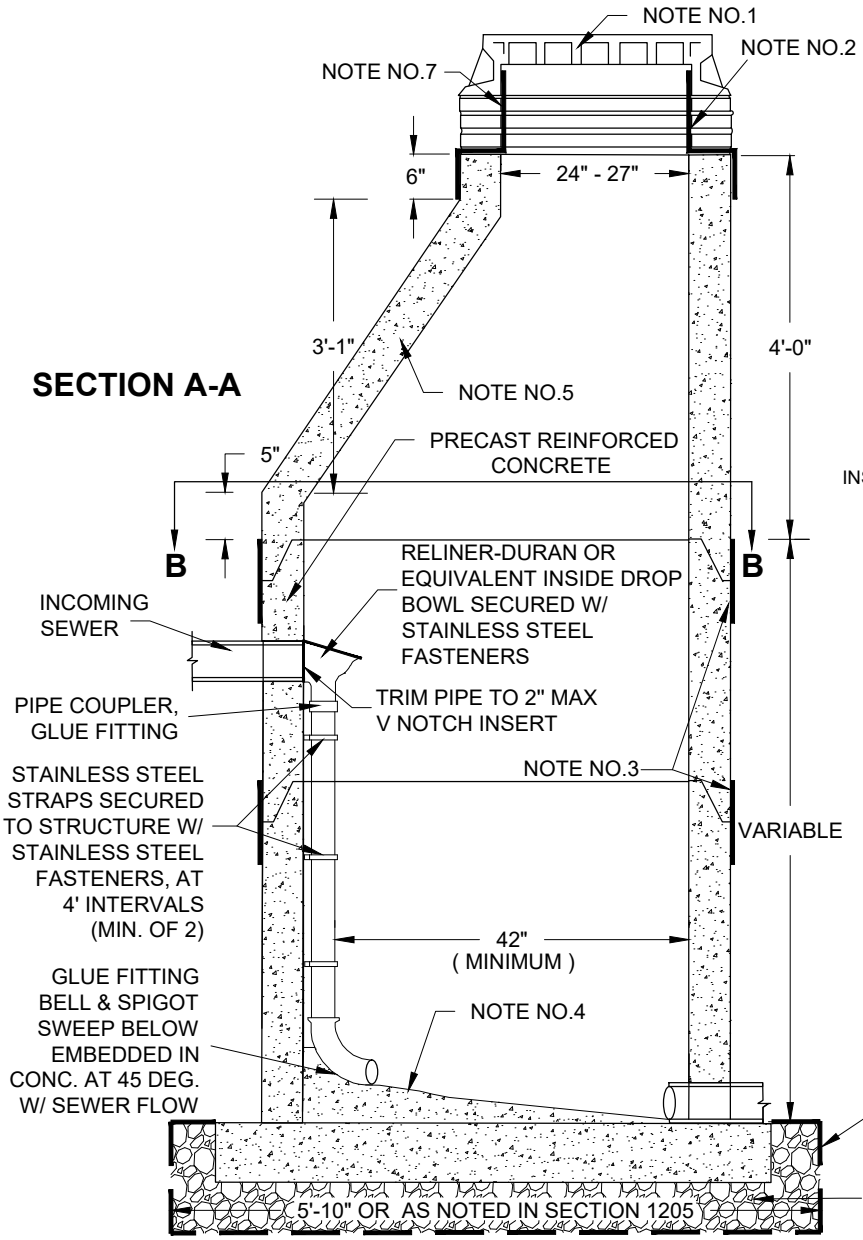
NOTE NO.6 - CAST IN PLACE MANHOLE BASES SHALL BE DIMENSIONED AS SHOWN UNLESS OTHERWISE INDICATED. BASES SHALL BE REINFORCED WITH NO.4 REBAR SPACED 15" ON CENTER BOTH WAYS.

NOTE NO.7 - IF REQUIRED PER SECTION 1205, I/I BARRIER AS MANUFACTURED BY AP/M PERMAFORM OR AN APPROVED EQUAL, IF SPECIFIED. ADJUSTMENT RINGS SHALL NOT EXCEED THE HEIGHT OF THE I/I BARRIER.

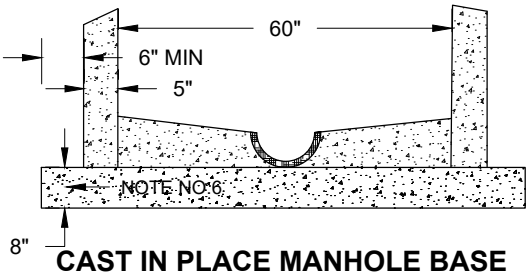
NOTE NO.8 - STEPS SHALL NOT BE PLACED IN SANITARY SEWER, STORM SEWER OR AIR RELEASE MANHOLES OR INLETS UNLESS SPECIFIED. IF SPECIFIED, THE MANHOLE STEPS SHALL BE DELTA SUREFOOT OR APPROVED EQUAL. STEPS SHALL BE SPACED 16" ON CENTER.

NOTE NO.9 - ALL STAINLESS STEEL SHALL BE GRADE 316.

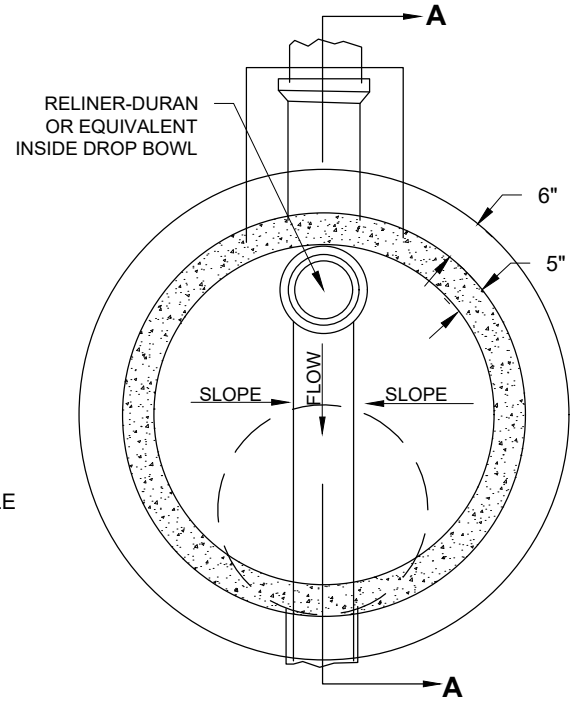
NOTE NO.10 - INTERIOR OF MANHOLE SHALL BE EPOXY COATED AS PER PLANS



SECTION A-A



CAST IN PLACE MANHOLE BASE



SECTION B-B

ROCK SHALL BE WRAPPED IN R1 FABRIC WHEN GROUND WATER IS PRESENT

6" OF 1 1/4" ROCK UNDER MANHOLE BASE. 1 1/4" CRUSHED ROCK OR BEDDING SAND TO BE USED TO SUPPORT PIPE IN OVER EXCAVATED AREA & 1/2 WAY UP PIPE.

Bismarck ENGINEERING

SANITARY SEWER DROP MANHOLE

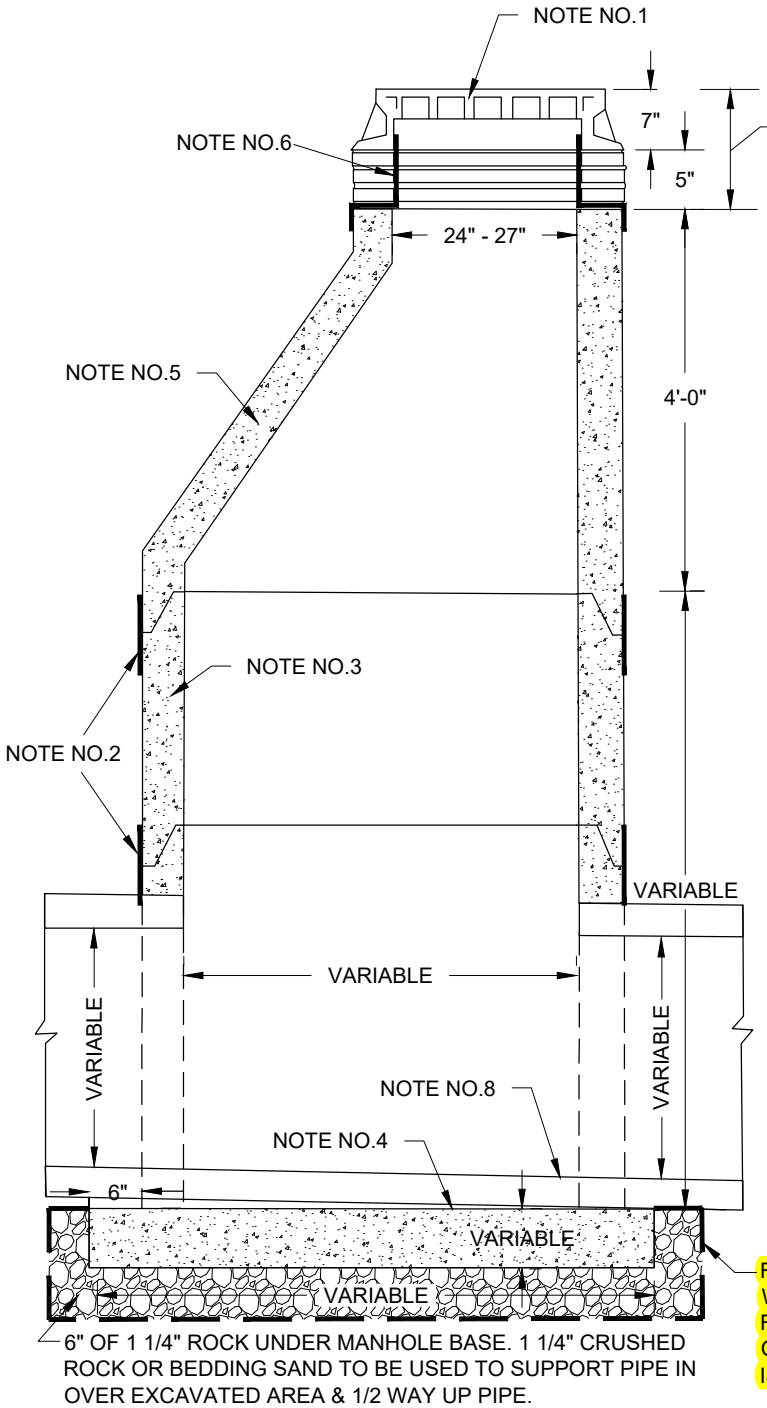
SCALE: Not to Scale

DATE: 1/2024

STANDARD DETAIL NO. **1205-2**

RECOMMENDED PRECAST SIZE REQUIREMENTS

BARREL DIAMETER	MAXIMUM SIZE RCP		
	2 @ 90	2 @ 180	RCP
48"	15"	30"	33"
54"	18"	36"	36"
60"	21"	42"	42"
66"	24"	48"	48"
72"	30"	48"	48"



NOTE NO.1 - MANHOLE CASTING SHALL BE AS DEFINED IN SECTION 1205.

NOTE NO.2 - P2 GASKETED JOINT FOR 48" MANHOLES, CX-4 JOINT FOR ALL OTHER SIZES OF MANHOLES AND EXTERIOR SEAL BY PRESS-SEAL GASKET CORP. EZ WRAP AND EZ STIK NO. 4 PRIMER, SPECIALTY PRODUCTS "MAC WRAP", INFI-SHIELD EXTERNAL GATOR WRAP, OR AN APPROVED EQUAL.

NOTE NO.3 - PRECAST CONCRETE MANHOLE SHALL CONFORM TO ASTM C478, WALL B.

NOTE NO.4 - PRECAST REINFORCED CONCRETE MANHOLE BASES SHALL CONFORM TO SECTION 1205.

NOTE NO.5 - MANHOLE TOP SECTION SHALL BE ECCENTRIC FOR 48" MANHOLES. MANHOLE TOP SECTION FOR MANHOLES OVER 48" SHALL BE FULL DIAMETER SECTION WITH FLAT TOP.

NOTE NO.6 - IF REQUIRED PER SECTION 1205, I/I BARRIER AS MANUFACTURED BY AP/M PERMAFORM OR AN APPROVED EQUAL. ADJUSTMENT RINGS SHALL NOT EXCEED THE HEIGHT OF THE I/I BARRIER.

NOTE NO.7 - STEPS SHALL NOT BE PLACED IN SANITARY SEWER, STORM SEWER, AIR RELEASE MANHOLES OR INLETS UNLESS SPECIFIED. IF SPECIFIED, THE MANHOLE STEPS SHALL BE DELTA SUREFOOT OR APPROVED EQUAL. STEPS SHALL BE SPACED 16" ON CENTER.

NOTE NO.8 - GROUT TO SLOPE FLOOR TO INVERT OF LOWEST PIPE.

Bismarck ENGINEERING

STORM SEWER MANHOLE

SCALE: Not to Scale

DATE: 1/2024

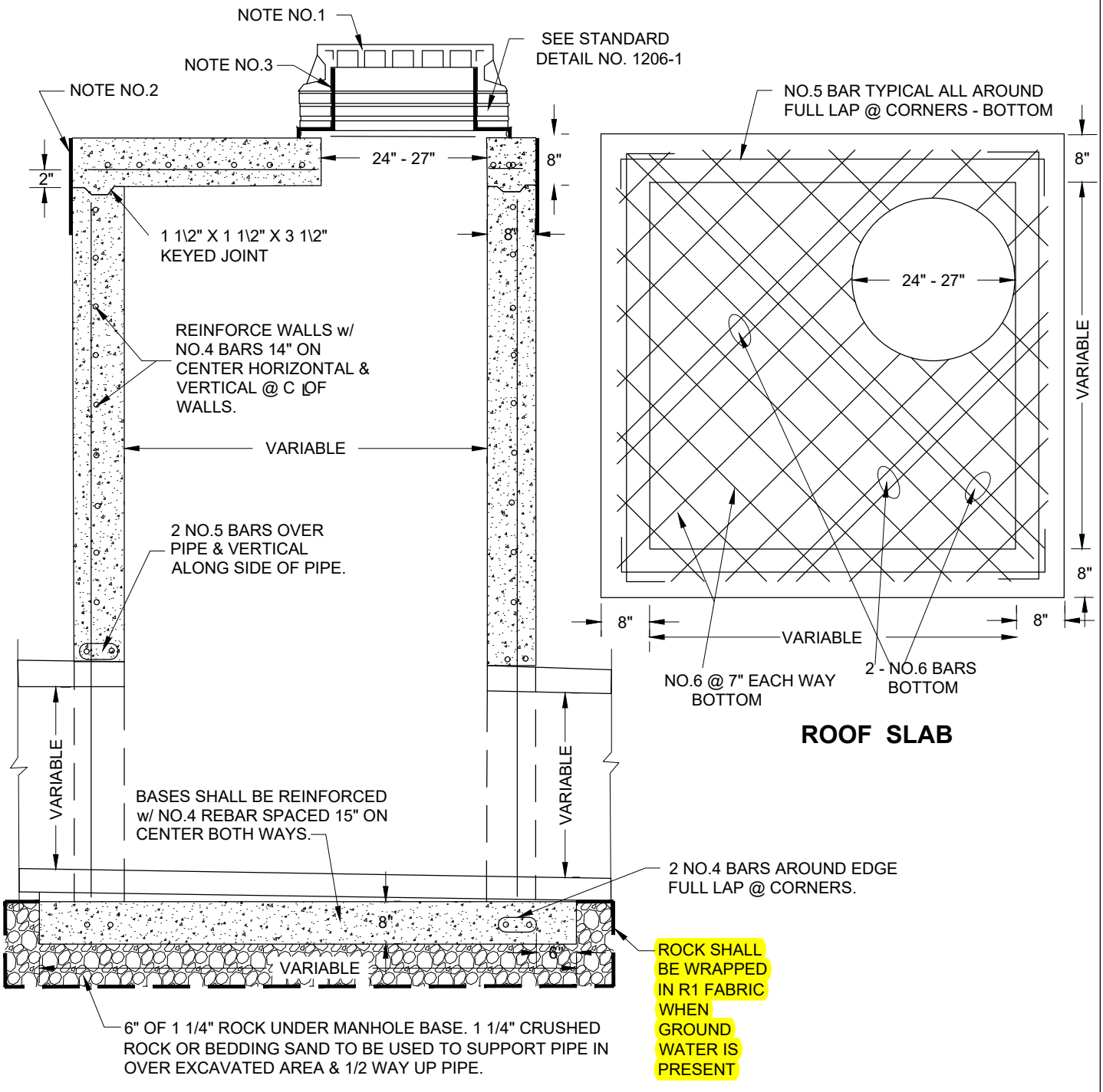
STANDARD DETAIL NO. 1205-3

NOTE NO.1 - MANHOLE CASTING AND COVER SHALL BE AS DEFINED IN SECTION 1205.

NOTE NO.2 - EXTERIOR SEAL BY PRESS-SEAL GASKET CORP. EZ WRAP AND EZ STIK NO. 4 PRIMER, SPECIALTY PRODUCTS "MAC WRAP", INFI-SHIELD EXTERNAL GATOR WRAP, OR AN APPROVED EQUAL.

NOTE NO.3 - IF REQUIRED PER SECTION 1205, I/I BARRIER AS MANUFACTURED BY AP/M PERMAFORM OR AN APPROVED EQUAL. ADJUSTMENT RINGS SHALL NOT EXCEED THE HEIGHT OF THE I/I BARRIER.

NOTE NO.4 - STEPS SHALL NOT BE PLACED IN SANITARY SEWER, STORM SEWER OR AIR RELEASE MANHOLES OR INLETS UNLESS SPECIFIED. IF SPECIFIED, THE MANHOLE STEPS SHALL BE DELTA SUREFOOT OR APPROVED EQUAL. STEPS SHALL BE SPACED 16" ON CENTER.



ROOF SLAB

Bismarck ENGINEERING	**CAST IN PLACE STORM SEWER MANHOLE**	SCALE: Not to Scale DATE: 1/2024	STANDARD DETAIL NO. **1205-4**

NOTE NO.1 - MANHOLE CASTING AND COVER SHALL BE AS DEFINED IN SECTION 1205.

NOTE NO.2 - 48" TYPE II COVER IN SECTION 1205.

NOTE NO.3 - MANHOLE CASTING ADJUSTMENT - SEE STANDARD DETAIL NO. 1206-1.

NOTE NO.4 - 2" STYROFOAM INSULATION SHALL BE AS DEFINED IN SECTION 901 INSTALLED AROUND THE OUTSIDE OF THE TOP 4 FEET OF THE MANHOLE AND HELD IN PLACE BY BANDS OR GLUE UNTIL BACKFILLED.

NOTE NO.5 - P2 GASKETED JOINT FOR 48" MANHOLES, CX-4 JOINT FOR ALL OTHER SIZES OF MANHOLES AND EXTERIOR SEAL BY PRESS-SEAL GASKET CORP. EZ WRAP AND EZ STIK NO. 4 PRIMER, SPECIALTY PRODUCTS "MAC WRAP", INFI-SHIELD EXTERNAL GATOR WRAP, OR AN APPROVED EQUAL.

NOTE NO.6 - INSTALL 2" STYROFOAM INSULATION AS DEFINED IN SECTION 901 GLUED TO THE BOTTOM OF THE PRECAST COVER.

NOTE NO.7 - SHALL BE PSX PRESS BOOT SECTION 1205.

NOTE NO.8 - AUTOMATIC AIR RELEASE VALVE - SEE SECTION 1205. Removed note for "1 -inch" valve, see 1205 for revised req's

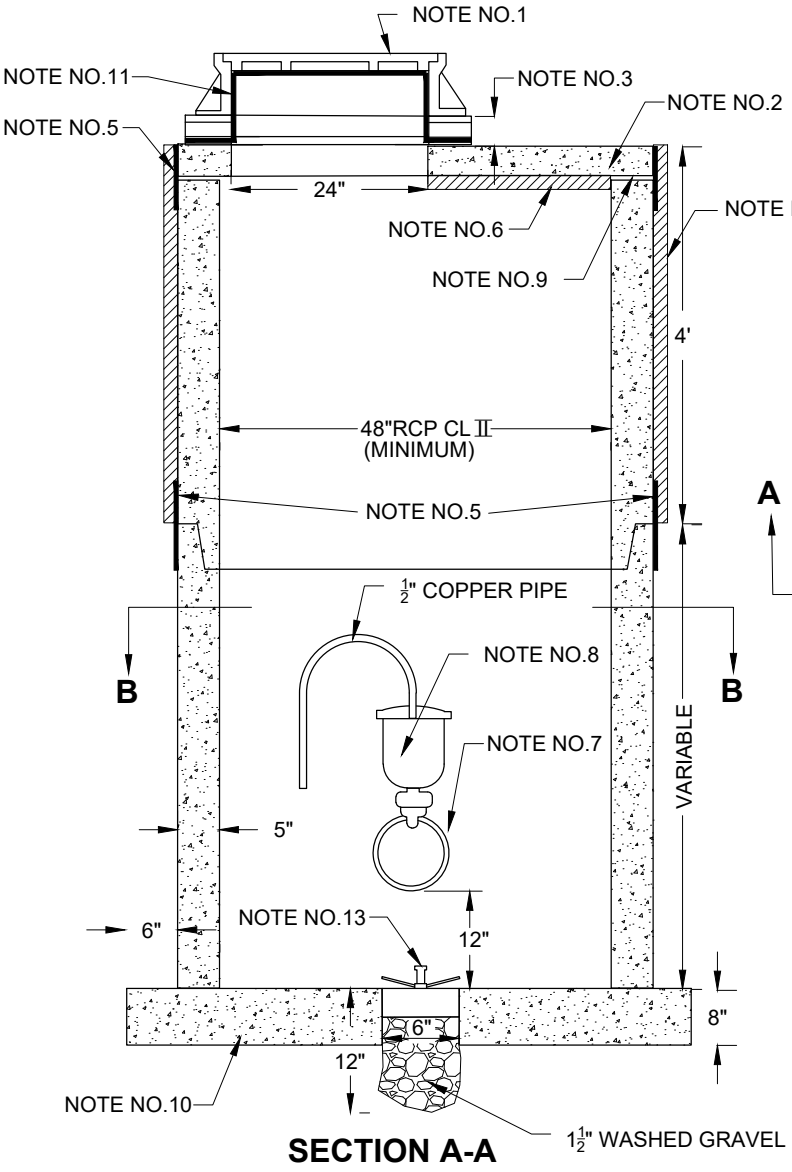
NOTE NO.9 - SEAL WITH EZ STIK PREMIUM BUTYL JOINT SEALANT OR AN APPROVED EQUAL.

NOTE NO.10 - SEE SECTION 1205 FOR PRECAST MANHOLE BASE REINFORCEMENT.

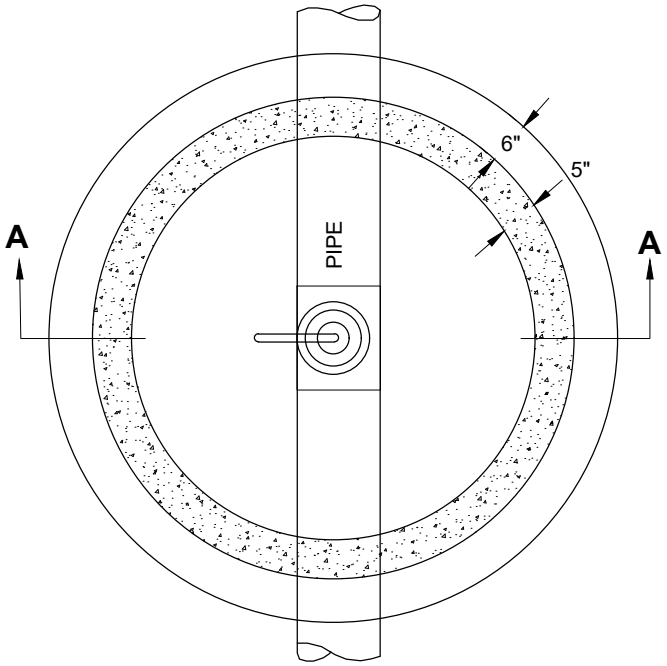
NOTE NO.11 - I/I BARRIER WITH CAP 'N SEAL AS MANUFACTURED BY AP/M PERMAFORM OR AN APPROVED EQUAL. ADJUSTMENT RINGS SHALL NOT EXCEED THE HEIGHT OF THE I/I BARRIER.

NOTE NO.12 - STEPS SHALL NOT BE PLACED IN SANITARY SEWER, STORM SEWER OR AIR RELEASE MANHOLES OR INLETS UNLESS SPECIFIED. IF SPECIFIED, THE MANHOLE STEPS SHALL BE DELTA SUREFOOT OR APPROVED EQUAL. STEPS SHALL BE SPACED 16" ON CENTER.

NOTE NO.13 - PROVIDE 6" CAST ALUMINUM EXPANSION PLUG.



SECTION A-A

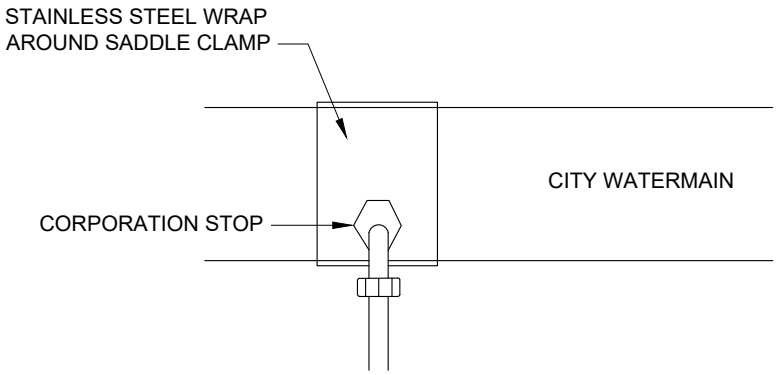


SECTION B-B

Bismarck ENGINEERING	**AIR RELEASE MANHOLE**	SCALE: Not to Scale DATE: 1/2024	STANDARD DETAIL NO. **1205-11**

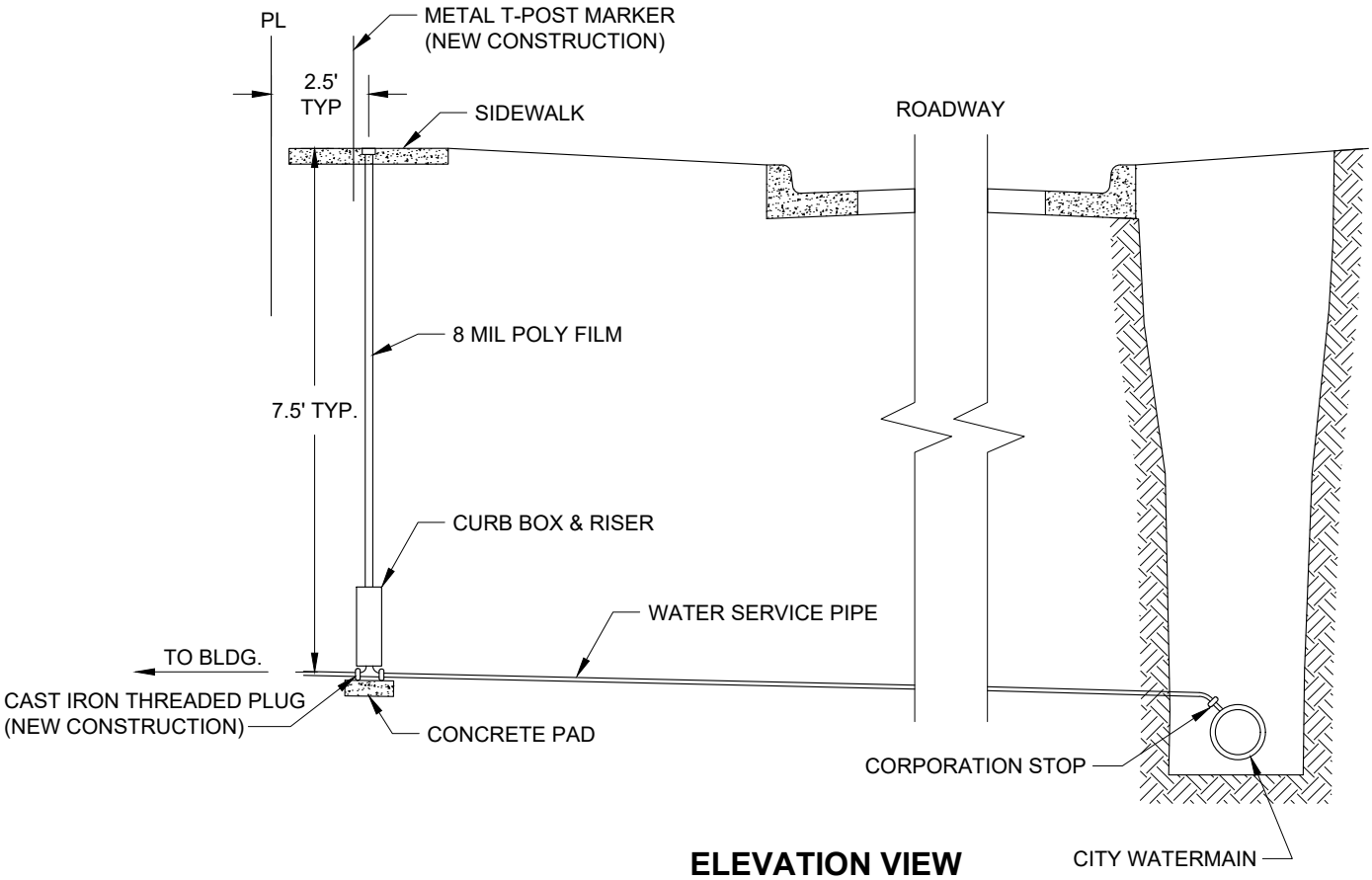
NOTES:

1. SERVICE CONNECTION FOR 1 INCH THROUGH 2 INCH PIPE, CURBSTOP & CORPORATION STOP SHALL BE AS DEFINED IN SECTION 1209.



PLAN VIEW

NEW STD DETAIL



ELEVATION VIEW

Bismarck ENGINEERING	**TYPICAL 1"-2" WATER SERVICE**	SCALE: Not to Scale DATE: 6/13/2023	STANDARD DETAIL NO. **1209-1**